CITY OF RICHMOND, VIRGINIA

DEPARTMENT OF PARKS, RECREATION AND COMMUNITY FACILITIES

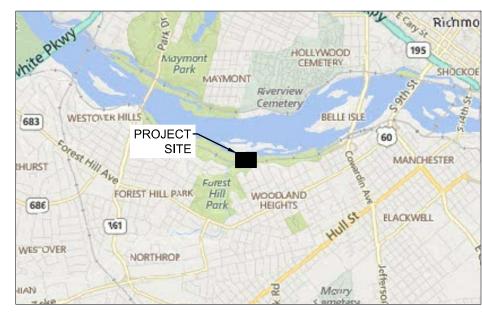


JAMES RIVER PARK SYSTEM WAREHOUSE SITE IMPROVEMENTS

INDEX OF SHEETS

SHEET TITLE SHEET NO. **COVER SHEET** C0.0 C1.0 NOTES AND DETAILS NOTES AND DETAILS C1.1 **EXISTING CONDITIONS AND DEMOLITION PLAN** C2.0 EROSION AND SEDIMENT CONTROL PHASE 1 AND 2 C3.0 **EROSION AND SEDIMENT CONTROL NOTES & DETAILS** C3.1 **EROSION AND SEDIMENT CONTROL NOTES & DETAILS** C3.2 LAYOUT AND UTILITY PLAN C4.0 C5.0 GRADING AND DRAINAGE PLAN DRAINAGE AREA MAP & PROFILES C5.1 **DRAINAGE & STORM CALCULATIONS** C5.2 C6.0 SWM - QUALITY CALCULATIONS **SWM - QUANTITY CALCULATIONS** C6.1 FLOODPLAIN CERTIFICATION AND CALCS C7.0 C7.1 FLOODPLAIN CERTIFICATION CALCULATIONS WATER QUALITY IMPACT ASSESSMENT C8.0 LANDSCAPE PLAN L1.0

3005 RIVERSIDE DRIVE, RICHMOND VA 23225



1"=2000'

MARCH 12, 2018

LANDSCAPE NOTES & DETAILS REVISIONS

LANDSCAPE NOTES & DETAILS

NO.	DATE	COMMENTS
	07/20/17	REVISED PER WR COMMENTS
	08/15/17	REVISED PER WR COMMENTS
	12/22/17	REVISED PER WR COMMENTS
	03/12/18	REVISED PER WR COMMENTS

OWNER
CITY OF RICHMOND PARKS, RECREATION &
COMMUNITY FACILITIES
4201 RIVERSIDE DRIVE
RICHMOND, VA 23225
TELEPHONE: (804) 646-5829
CONTACT: NATHAN BURRELL
EMAIL: NATHAN.BURRELL@RICHMONDGOV.COM

L2.0

L2.1

ENGINEER
TIMMONS GROUP
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RICHMOND, VA 23225
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RICHMOND, VA 23224
TELEPHONE: (804) 232-8911, EXT. 109
CONTACT: KATIE HARRIGAN
EMAIL: KHARRIGAN@3NORTH.COM

REQUIRED PERMITS:

RSMP PERMIT



TIMMO	NS G	ROUP	
Site Development	Residential	Infrastructure	Technology
,	OUR VISION ACHIEVE	D THROUGH OURS-	
VIRGINIA	NORTH CARO	LINA WEST	VIRGINIA
THIS	DRAWING PR	REPARED AT THE	E
	DOWNTOW	N OFFICE	
117 S 14th St	reet Suite 30	3 Richmond, \	/A 23219
TEL 804.521.106	5 FAX 804.5	21 1068 www.t	immons.com
	36157	.001	

DRAWING NO: 36157.001

GENERAL NOTES

1. THIS PROJECT IS PROPOSED BY:

CITY OF RICHMOND DEPARTMENT OF PARKS AND RECREATION NATHAN BURRELL DEPARTMENT OF PARKS AND RECREATION 4201 RIVERSIDE DRIVE RICHMOND, VA 23225

NUMBER OF LOTS AFFECTED BY THIS PROJECT: 1 — 4201 RIVERSIDE DRIVE, VA 23225-

- 2. EXISTING ZONING OF PROPERTY THROUGH WHICH PROJECT IS PROPOSED: R-3
- 3. ALL CONSTRUCTION AND MATERIALS SHALL CONFORM WITH THE VIRGINIA DEPARTMENT OF TRANSPORTATION \underline{ROAD} AND BRIDGE SPECIFICATIONS, DATED 2007; AND ROAD AND BRIDGE STANDARDS, DATED DECEMBER 2008; AS AMENDED BY CONTRACT PROVISIONS; THESE PLANS; CITY OF RICHMOND RIGHT-OF-WAY EXCAVATION AND RESTORATION MANUAL: CITY OF RICHMOND SANITARY SEWER SYSTEM DESIGN GUIDELINES AND STANDARD SPECIFICATIONS AND DETAILS; AND, CITY OF RICHMOND STORMWATER MANAGEMENT DESIGN AND CONSTRUCTION
- 4. LOCATE ALL EXISTING UTILITIES PRIOR TO THE START OF CONSTRUCTION. IF ANY UTILITY DIFFERS THAN WHAT IS SHOWN ON THE PLAN, CONTACT THE ENGINEER IMMEDIATELY.
- 5. IF THE ELEVATIONS SHOWN ON THESE PLANS ARE FOUND TO BE DIFFERENT THAN FIELD CONDITIONS, CONTACT THE ENGINEER IMMEDIATELY.
- 6. ACQUIRE ALL REQUIRED PERMITS PRIOR TO CONSTRUCTION. ALL FEES ASSOCIATED WITH PERMITS SHALL BE PAID BY THE CONTRACTOR UNLESS OTHERWISE SPECIFIED.
- EXISTING CONDITIONS SHOWN HEREON COMPILED FROM CITY OF RICHMOND GIS INFORMATION AND FIELD SURVEY DATA FROM H&B SURVEYING AND MAPPING DATED APRIL 29, 2016.

DRAINAGE NOTES

- I. ALL CONSTRUCTION AND MATERIALS SHALL CONFORM WITH THE VIRGINIA DEPARTMENT OF TRANSPORTATION'S ROAD AND BRIDGE SPECIFICATIONS DATED 2007, AND ROAD AND BRIDGE STANDARDS, DATED DECEMBER 2008, AS AMENDED BY CONTRACT PROVISIONS AND THESE PLANS. ALL CONSTRUCTION AND MATERIALS SHALL CONFORM WITH CITY OF RICHMOND STANDARDS AND SPECIFICATIONS, IF MORE STRINGENT.
- 2. ALL CONCRETE PIPE JOINTS ARE TO BE SEALED ACCORDING TO VDOT STANDARDS AND SPECIFICATIONS.
- 3. ALL STORM SEWERS SHALL BE ASTM, C-76, CLASS III, EXCEPT AS NOTED.
- 4. ALL STORM SEWERS AND STRUCTURES SHALL HAVE A MINIMUM OF 4" OF AGGREGATE BEDDING PLUS 1/10 DIAMETER. 11. INSTALL STORM SEWER PER SHEET C5.0. INSTALL INLET PROTECTION ON ALL INLETS PER SHEET C5.0.
- 5. ALL MANHOLE AND INLET INVERTS SHALL BE SHAPED IN ACCORDANCE WITH VDOT STANDARD IS-1, EXCEPT WHERE SPECIFIED.
- 6. IF DURING CONSTRUCTION, THE EXISTING CULVERT OR DITCH INVERT ELEVATIONS SHOWN ON THESE PLANS ARE FOUND TO DIFFER SIGNIFICANTLY FROM THE ELEVATIONS IN THE FIELD, THE CONTRACTOR MUST NOTIFY THE ENGINEER IMMEDIATELY FOR AN ADJUSTMENT IN ELEVATIONS.
- ALL PIPE LENGTHS SHOWN ON PLANS ARE FROM CENTER OF CHAMBER TO CENTER OF CHAMBER, UNLESS OTHERWISE SPECIFIED.

CONSTRUCTION ACCESS AND MAINTENANCE OF TRAFFIC NOTES

- 1. IMPLEMENT THE NOTES AND DETAILS PROVIDED IN THE VDOT "VIRGINIA WORK AREA PROTECTION MANUA STANDARDS AND GUIDELINES" AT ALL TIMES DURING CONSTRUCTION. MAINTAIN A COPY OF THIS MANUAL ONSITE AT ALL TIMES DURING CONSTRUCTION.
- ALL WORK IS SUBJECT TO INSPECTION BY DPW INSPECTOR, NOTIFY APPROPRIATE CITY OFFICIALS 72 HOURS PRIOR TO START OF WORK.
- 3. CALL "MISS UTILITY" OF CENTRAL VIRGINIA 1-800-552-7001 (TOLL FREE) PRIOR TO CONSTRUCTION. VERIFY LOCATION AND ELEVATION OF ALL UNDERGROUND UTILITIES SHOWN ON THE PLANS IN AREAS OF CONSTRUCTION PRIOR TO STARTING WORK, CONTACT ENGINEER IMMEDIATELY IF LOCATION OR ELEVATION IS DIFFERENT FROM THAT SHOWN ON THE PLAN, IF THERE APPEARS TO BE A CONFLICT, OR UPON DISCOVERY OF ANY UTILITY NOT SHOWN ON THE
- 4. TAKE ALL NECESSARY PRECAUTIONS TO PROTECT AND MAINTAIN UNINTERRUPTED UTILITY SERVICE AT ALL TIMES DURING CONSTRUCTION, ANY DAMAGE TO EXISTING STRUCTURES SHALL BE REPARED IMMEDIATELY TO THE SATISFACTION OF THE CITY UTILITY INSPECTOR, AT THE CONTRACTOR'S EXPENSE.
- 5. MAINTAIN SAFE VEHICULAR AND PEDESTRIAN ACCESS TO ALL PROPERTIES THROUGHOUT CONSTRUCTION AND PREPARE A TRAFFIC MAINTENANCE PLAN IF REQUIRED BY THE CITY. ANY DEVIATIONS FROM THIS PLAN SHALL BE APPROVED BY THE CITY TRAFFIC ENGINEER PRIOR TO IMPLEMENTATION.
- 6. STOCKPILES OF MATERIAL NOT PERMITTED IN THE TRAVELWAY.
- 7 THE COST OF ALL CONSTRUCTION SIGNS SIGN POST BARRICADES DELINEATORS CONCRETE CONSTRUCTION THE COST OF ALL CONSTRUCTION SIGNS, SIGN POST, BARRICADES, DELINEATORS, CONCRETE CONSTRUCTION BARRIERS, FLASHING AND STEADY BURN LIGHTS, AND OTHER TRAFFIC CONTROL DEVICES WHICH ARE NECESSARY FOR CONSTRUCTION SHALL BE BORNE BY THE CONTRACTOR. IN ADDITION, THE COST OF ALL TRAFFIC CONTROL WHICH ARE REQUIRED BY THE CONSTRUCTION STAGING, AND/OR EXTENSIONS OF TIME WHICH ARE REQUIRED BY THE TRAFFIC ENGINEER AND CONSTRUCTION ENGINEER AND CONSTRUCTION ENGINEER AND CONSTRUCTION ENGINEER, SHALL BE BORNE BY THE CONTRACTOR AND ARE APPROVED BY THE TRAFFIC ENGINEER AND CONSTRUCTION ENGINEER, SHALL BE BORNE BY THE CONTRACTOR AND INCLUDED IN THE BID PRICE.
- ALL SIGNS AND BARRICADES USED IN CONJUNCTION WITH THIS PROJECT SHALL CONFORM WITH THE LATES' EDITIONS OF THE "NATIONAL MANUAL ON UNIFORM TRAFFIC CONTROL DEVICES" AND "THE VIRGINIA SUPPLEMENT TO THE MANUAL ON UNIFORM TRAFFIC CONTROL DEVICES FOR STREETS AND HIGHWAYS".
- FXCAVATIONS WHICH ARE PROPOSED TO BE OPEN PAST NORMAL WORKING HOURS MUST BE APPROVED BY THE EACAVATIONS WHICH ARE PROPOSED TO BE OPEN PAST NORMAL WORKING HOURS MUST BE APPROVED BY THI TRAFFIC ENGINEER. NO OPEN TRENCHES ARE ALLOWED OVERNIGHT—EITHER TEMPORARY STEEL PLATING OR TEMPORARY BACKFILL ARE REQUIRED. THE COST OF SIGNING AND BARRICADING THESE EXCAVATIONS IS THE RESPONSIBILITY OF THE CONTRACTOR AND MUST BE INCLUDED IN THE BID PRICE.
- 10. WHEN CONSTRUCTION OCCURS PARALLEL AND/OR PERPENDICULAR TO ROADS, INCLUDE IN THE TRAFFIC MAINTENANCE PLAN AT LEAST AN 11' MINIMUM TRAVEL LANE WITH FLAGMEN TO DIRECT TRAFFIC THROUGH THE
- 11. DESIGN FEATURES RELATING TO CONSTRUCTION OR TO REGULATION AND CONTROL OF TRAFFIC MAY BE SUBJECT TO CHANGE AS DEEMED NECESSARY BY THE CITY TRAFFIC ENGINEER.
- 12. REMOVE UNSUITABLE MATERIAL IF ENCOUNTERED AND REPLACE WITH SUITABLE MATERIAL TO THE SPECIFICATION
- 13. IF TREES DESIGNATED "TO REMAIN" ARE LOCATED WITHIN 10' OF CONSTRUCTION LIMITS, CONTACT DPW URBAN FORESTRY 48 HOURS PRIOR TO BEGINNING CONSTRUCTION.
- 14. PROVIDE TEMPORARY DRAINAGE WITHIN THE PROJECT LIMITS DURING CONSTRUCTION OR TO RELIEVE AREAS THAT MAY CAUSE DAMAGE TO ROADWAYS OR IMPEDE TRAFFIC AS DIRECTED BY THE CITY OF RICHMOND INSPECTOR.
- 15. CLEAN ALL DRAINAGE PIPES AND STRUCTURES OF DEBRIS AND ERODED MATERIAL AT ALL STAGES OF CONSTRUCTION TO THE SATISFACTION OF THE CITY OF RICHMOND INSPECTOR.
- REFER TO THE LATEST VERSION OF THE VIRGINIA DEPARTMENT OF TRANSPORTATION ROAD AND BRIDGE STANDARDS AND SPECIFICATIONS.
- 17. PERFORM ALL CUTS IN THE STREET UNDER A WORK IN STREET PERMIT. WORK SHALL BE MONITORED BY THE PERMIT
- DO NOT BEGIN WORK UNTIL THE PERMIT INSPECTOR HAS BEEN NOTIFIED, A PRE-CONSTRUCTION CONFERENCE HAS BEEN HELD AND MISS UTILITY HAS MARKED ALL UTILITIES.
- 19. ASPHALT PAVEMENT CUTS SHALL BE AS CLEAN AND STRAIGHT AS POSSIBLE, WITH NO OUTLINE DIMENSIONS LESS THAN 3 FEET WITHOUT SPECIAL APPROVAL OF THE PERMIT INSPECTOR. REFER TO DETAIL ON THIS SHEET FOR PAVEMENT RESTORATION.
- 20. ALL ASPHALT PAVEMENT RESTORATION THICKNESS SHALL BE 1 1/2 TIMES THE EXISTING SECTION OR A MINIMUM OF 8-INCHES WHICHEVER IS GREATER. REFER TO THE DPW TRENCH CUT RESTORATION DETAIL ON THIS SHEET FOR THE TYPICAL CONFORMANCE STANDARDS.

- 21. THE FINAL RESTORATION ON OPEN TRENCH CUTS REQUIRES THE DISTURBED ASPHALT PAVEMENT ZONE TO BE A SQUARE POINTED OFF AND STRAIGHT LINE. THE AREA OF PAVEMENT RESTORATION IS TO BE FULLY ENVELOPED BY THE FINAL SURFACE COURSE REPAIRS. THE ADJOINING SURFACE/TOP COURSE LAYER IS TO BE OVER-MILLED A MINIMUM DEPTH OF 1.25 INCHES OR MORE, A MINIMUM DISTANCE OF ONE FOOT BEYOND EACH SIDE OF THE TRENCH WALL. THIS STEP OUT IS TO OCCUR ALONG THE ENTIRE TRENCH LINE RUN AND/OR SQUARED POINTED AREA.
- 22. FINAL ACCEPTANCE BY THE CITY SHALL NOT BE MADE UNTIL ALL WORK SHOWN ON THE APPROVED PLANS IS COMPLETE TO THE SATISFACTION OF THE CITY INSPECTOR AND PROJECT MANAGER.

CONSTRUCTION SEQUENCE GUIDELINES

- PROVIDE A DETAILED SCHEDULE AND SEQUENCE OF CONSTRUCTION TO THE OWNER AND ENGINEER PRIOR TO CONSTRUCTION. CONSTRUCTION SEQUENCE GUIDELINES HAVE BEEN PROVIDED BELOW TO PROVIDE REQUIRED OPERATIONAL PARAMETERS DURING CONSTRUCTION.
- 2. ACQUIRE ALL REQUIRED PERMITS PRIOR TO CONSTRUCTION. ALL PERMIT FEES TO BE PAID BY CONTRACTOR.
- 3. SCHEDULE A PRE-CONSTRUCTION MEETING WITH THE CITY OF RICHMOND AND TIMMONS GROUP AT LEAST 72 HOURS PRIOR TO THE START OF WORK.
- 4. CALL "MISS UTILITY" AT 1-800-552-7001 AT LEAST 48 HOURS PRIOR TO CONSTRUCTION. CONTACT THE ENGINEER
- MEDIALELT IF:

 LOCATION OR ELEVATION IS DIFFERENT FROM THAT SHOWN ON THE PLAN

 IF THERE APPEARS TO BE A CONFLICT;

 OR UPON DISCOVERY OF ANY UTILITY NOT SHOWN ON THE PLANS.
- PERFORM CONSTRUCTION SURVEY STAKEOUT FOR PROPOSED IMPROVEMENTS AND CONSTRUCTION LIMITS. ALL SURVEYING OPERATIONS MUST BE PERFORMED BY A VIRGINIA LICENSED SURVEYOR.
- MAINTAIN UNINTERRUPTED UTILITY SERVICE TO ALL ADJACENT PROPERTIES AT ALL TIMES DURING CONSTRUCTION.
- 7. NOTE, THE BUILDING MATERIALS AT AND BELOW THE BASE FLOOD ELEVATIONS (BFE) HAVE TO BE FLOOD-RESISTANT NOTE: FIRE DIFFERENCE MATERIALS AT AND BELOW THE BASE LINEAD ELEVATIONS (BF) AND THE REPORT OF THE REPORT OF SUPPLIES AND ATTEMPT OF THE REPORT OF THE REPOR
- 8. INSTALL THE PHASE I EROSION & SEDIMENT CONTROL MEASURES BEFORE COMMENCING ANY LAND DISTURBING ACTIVITIES. REFER TO SHEET C3.0 FOR PHASE I AND II EROSION & SEDIMENT CONTROL SEQUENCES.
- 9. DEMOLISH/REMOVE ITEMS INDICATED ON SHEET C2.0 ONLY AS NECESSARY FOR SITE CONSTRUCTION.
- 10. INSTALL AND MOVE TEMPORARY PUMPS AS NECESSARY TO DIVERT CLEAN WATER AROUND ACTIVE PORTION OF THE CONSTRUCTION SITE, PROVIDE TEMPORARY DRAINAGE MEASURES WITHIN THE PROJECT LIMITS AT THE END OF EACH DAY AS NECESSARY TO PREVENT FLOODING AND SEDIMENT RUNOFF INTO EXISTING STORMWATER SYSTEMS.
- 12. INSTALL UTILITIES (WATER) PER SHEET C4.0
- 13. ROUGH GRADE SITE TO APPROXIMATE ELEVATIONS OF PROPOSED GRADE MINUS PAVEMENT/PAVER SECTION DEPTH. EXCLUDING LANDSCAPE AREAS, COMPACT SITE PRIOR TO INSTALLATION OF BUILDINGS.
- 14. INSTALL BUILDINGS. REFER TO ARCHITECTURAL PLANS
- 15. INSTALL PHASE II EROSION & SEDIMENT CONTROL MEASURES AS SHOWN ON SHEET C3.0.
- 16. BRING SITE TO FINAL GRADE. REFER TO SHEET C5.0
- 17. MAINTAIN ALL EROSION & SEDIMENT CONTROL MEASURES AT ALL TIMES. NO MEASURE CAN BE REMOVED UNTIL APPROVED BY THE CITY OF RICHMOND INSPECTOR.

PERMEABLE PAVER CONSTRUCTION SEQUENCE AND MAINTENANCE NOTES

CONSTRUCTION SEQUENCE

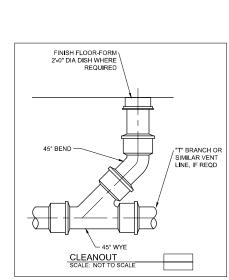
- STABILIZED, DO NOT INSTALL SYSTEM IN RAIN OR SNOW, DO NOT INSTALL FROZEN BEDDING MATERIALS
- TEMPORARY E&S MEASURES NEEDED DURING INSTALLATION TO KEEP PAVEMENT AREA SEDIMENT FREE DURING CONSTRUCTION.
- WHERE POSSIBLE, EXCAVATORS OR BACKHOES SHOULD WORK FROM THE SIDES TO EXCAVATE THE RESERVOIR LAYER TO ITS APPROPRIATE DESIGN DEPTH AND DIMENSIONS.
- THE NATIVE SOILS ALONG THE BOTTOM AND SIDES OF THE SYSTEM SHOULD BE SCARIFIED OR TILLED TO A DEPTH OF 3 TO 4 INCHES PRIOR TO THE PLACEMENT OF THE FILTER LAYER OR FILTER FABRIC.
- FILTER FABRIC SHOULD BE INSTALLED ON THE BOTTOM AND SIDES OF THE RESERVOIR LAYER. FILTER FABRIC STRIPS SHOULD OVERLAP DOWN-SLOPE BY A MINIMUM OF 2 FEET, AND BE SECURED A MINIMUM OF 4 FEET BEYOND THE EDGE OF THE EXCAVATION.
- PROVIDE A MINIMUM OF 2-INCHES OF AGGREGATE ABOVE AND BELOW THE UNDERDRAINS MOISTEN AND SPREAD 6-INCH LIFTS OF THE APPROPRIATE CLEAN, WASHED STONE AGGREGATE, PLACE AT LEAST 4-INCHES OF ADDITIONAL AGGREGATE ABOVE THE UNDERDRAIN, AND THEN COMPACT IT USING
- A VIBRATORY ROLLER IN STATIC MODE UNTIL THERE IS NO VISIBLE MOVEMENT OF THE AGGREGATE.
- INSTALL THE DESIRED DEPTH OF THE BEDDING LAYER.
 PAVING MATERIALS SHOULD BE INSTALLED IN ACCORDANCE WITH MANUFACTURER SPECIFICATIONS.

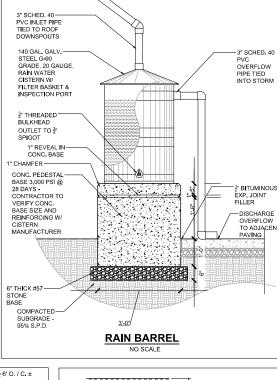
AN ANNUAL INSPECTION SHOULD BE COMPLETED EACH YEAR TO IDENTIFY ANY POTENTIAL ISSUES. COMPLETED. TO AVOID SUBSURFACE CLOGGING. ENSURE THE VACUUM SWEEPER DOES NOT USE

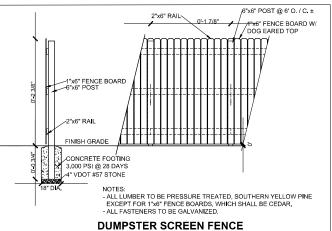
GREEN ROOF CONSTRUCTION SEQUENCE AND MAINTENANCE NOTES

- CONSTRUCT ROOF DECK WITH APPROPRIATE SLOPE AND MATERIAL
- INSTALL WATERPROOFING METHOD ACCORDING TO MANUFACTURER'S SPECIFICATIONS
- CONDUCT FLOOD TEST TO ENSURE SYSTEM IS WATER TIGHT BY PLACING 2-INCHES OF WATER OVER MEMBRANE FOR 48 HOURS TO CONFIRM INTEGRITY OF THE WATERPROOFING SYSTEM.
- ADD ADDITIONAL SYSTEM COMPONENTS, GROWING MEDIA SHOULD BE MIXED PRIOR TO DELIVERY TO THE SITE. MEDIA SHOULD BE SPREAD EVENLY OVER THE FILTER FABRIC SURFACE. COVER GROWING MEDIA UNTIL PLANTING TO PREVENT WEEDS FROM GROWING.
- VEGETATION SHOULD BE PLANTED PER THE PLANTING PLAN.

- WATER AS NEEDED TO PROMOTE PLANT GROWTH AND SURVIVAL
- INSPECT ROOF AND REPLACE ANY DEAD OR DYING VEGETATION. INSPECT WATERPROOF MEMBRANE FOR LEAKING OR CRACKS.
- ANNUAL FERTILIZATION (FIRST 5 YEARS). WEEDING TO REMOVE INVASIVE PLANTS
- INSPECT ROOF DRAINS, SCUPPERS AND GUTTERS TO ENSURE THEY ARE NOT OVERGROWN. REMOVE ANY ACCUMULATED ORGANIC MATTER OR DEBRIS.







NO SCALE

STONE OPEN-GRADED

/11 3" - 2" THICK #8

JOINTS

STONE AROUND

PERMEABLE PAVER

NO SCALE

AGGREGATE BEDDING

-COMPACTED

SUBGRADE -

95% S.P.D.

COURSE & SWEPT

TYPICAL PAVING PATTERN

GRASS GROWN IN-

SOIL MIX WITHIN IN

PAVER VOIDS

B" WIDE FLUSI

VDOT #2 STONE

SUBBASE

NONWOVEN

GEOTEXTILE @ BOTTOM & SIDES

OF SUBBASE PERFORATE

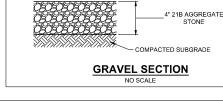
UNDERDRAIN REFER TO SHEETS C5.0 AND C5.1

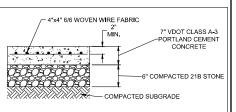
AND INVER

FOR PIPE SIZE, MATERIAL

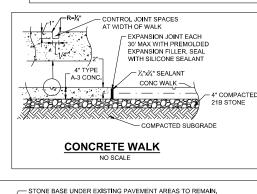
CLASS A-3 CONC

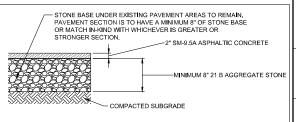
EDGE RESTRAINT





HEAVY DUTY CONCRETE PAVEMENT/CONCRETE PAD





ASPHALT PAVEMENT SECTION





DRAWN BY: K. ATKINSON CHECKED BY: A. WEHUNT REVISIONS RICHMOND REVISED PER WR'S 07/20/1 AS BUILT **TIMMONS GROUP** YOUR VISION ACHIEVED THROUGH OURS.

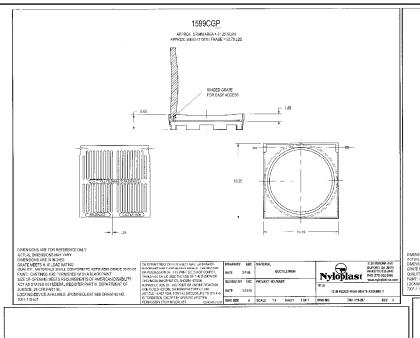
> VIRGINIA | NORTH CAROLINA | WEST VIRGINIA THIS DRAWING PREPARED AT THE DOWNTOWN OFFICE
>
> 117 S 14th Street Suite 303 | Richmond, VA 23219
> TEL 804.521.1065 FAX 804.521.1068 www.timmons.c 36157.001

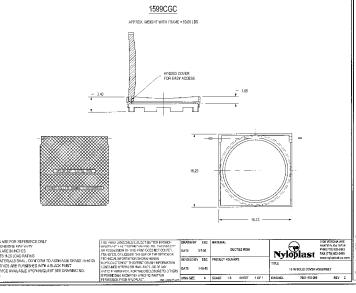
JAMES RIVER PARK SYSTEM WAREHOUSE SITE **IMPROVEMENTS**

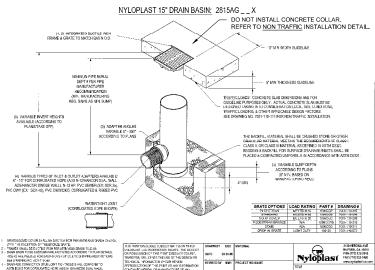
> NOTES AND DETAILS

> > SHEET NO.

DEPARTMENT OF PARKS AND RECREATION







SECTION VIEW OF DISSIMILAR MATERIALS CONNECTION Nyloplast FAX (779) 932-2443 Nyloplast FAX (779) 932-2443 DATE 04-18-07 REVISED BY CCA PROJ

Engineered Surface Drainage Products

GENERALPVC surface drainage inlets shall include the drain basin type as indicated on the contract drawing and referenced within the contract specifications. The dudble inon grates for each of these fittings are to be considered an integral part of the surface drainage inlet and shall be furnished by the same manufacturer. The surface drainage inlets shall be as manufactured by Nyloplast a division of Advanced Drainage Systems, Inc., or prior approved equal.

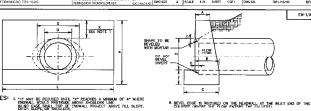
sins required for this contract shall be main furthered from PVC pipe stock, utilizing a thermoforming process to reform the cine stock to the specified.

The distinge pipe connection subsistant be mainufactured from PVC pipe stock and formed to provide a vicatelight connection with the specified this positive shall conform to ASTM 592°1 big prints for what makes the past past positive stock and the provided provided the stock of the stock of

The grates and trames furnished for all surface drainage inlets shall be ducttle iron for sizes 8°, 10°, 12°, 15°, 18°, 24° and 30° and shall be made specifically for each basin so as to provide a round bottom flarge that closely matches the diameter of the surface drainage inlet. Grates for drain basins shall be capable of supporting various wheel pages as specimed by Wegoliss. Iz and 15° square grates will be integrable the firmer using parts. Dualitie from used in the manufacture of the castings shall conform to <u>ASTM ASSB grade 70-50-05</u>. Grates and covers shall be provided painted black.

INSTALLATION
The specified PVC surface drainage injet shall be installed using conventional flexible pipe boothit materials and procedures. The baselfit materials shall be crushed stone or other granular material meeting the requirements of class 1 or class 2 relations us defined in ACTIM ED221. Bearing and baselfit for surface control of the contro

SIGNOSES SUBJECT MATTER IN HARCH	DRAWN 83	r ÇIA	MATERIAL		3. 36 AEVOUV VAE
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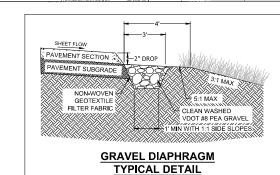
7. HEADWALL AT THE OUTLET END OF THE CULVERT MAY BE EITHER SQUARE EDGE OR BEVEL EDGE.

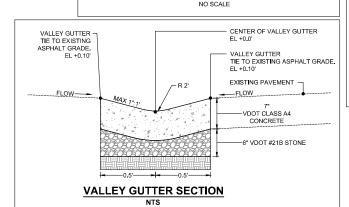
8. ON SHALLOW FILLS, WHERE ENDWALLS ARE 1 OR LESS BELOW SHOULDER LINE, THE TOP OF THE ENDWALL SHALL BE CONSTRUCTED PARALLEL TO THE GRADE OF THE ROAD. 4. THIS STANDARD TO BE USED WITH STRAIGHT CROSSINGS AND ALL SIXEWS00*TO 45*).

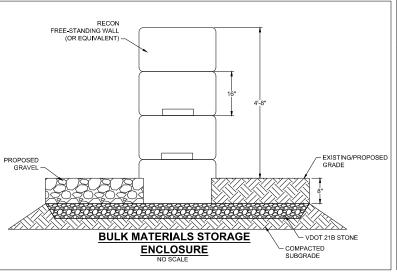
5. HEADWALL TO BE BEVELED IN ALL AREAS EXCEPT WHERE A CONFLICT WITH INVERT OR WINGWALLS OCCUR

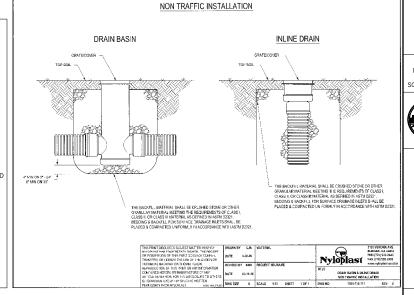
						EW-1	1									EW-1A
	END	WALL	FOR C	CIRCULA	R PIPE					END	WALL F	OR ELL	IPTICAL	PIPE		
		DIAMET	ER DF P	IPE CULVER	r		ı			SIZE OF	ELLIPTICAL	PIPE CULV	ERT (SPAN	x RISE)		
	12"	15"	18"	21" DR 24"	27" OR 30"	33" OR 36"	ı		23"x14"	30"x10"	344x22*	38"x24"	42"x27"	45"x29"	49"x32"	53"x34"
Α	0"-8"	08	0,-8,,	0'-11"	1'-0"	1'-0"	ı	A	0'-B"	0'-9"	0'-10"	D'-11"	0'-15'	1"-0"	1"-0"	1'-0"
В	0'-11"	7-1"	1'-3'	1'-6"	1-9"	2'-0"	1	В	1'-2"	1'-5"	7-6"	1'-8"	1'-9"	1"-10"	1'-11"	F-1F'
C	7-4"	1'-7"	7-9"	2"-2"	2'+6"	2*-9*	1	c	1'-8"	P-10°	2'-1"	2'-4"	2*-5"	2'-7"	2'-8"	2'-9"
D	7-0"	1'-3"	1'-6"	21-0"	20-60	3°-0"	[D	1'-2"	7-7"	17-10"	2'-0"	2'-3"	2'-5"	2'-8"	2'-10"
F	0'-6"	0,-80	0'-8"	68	084	0,-9,,	l	F	01-81	0"-8"	0'-9"	0'-9"	0'-9"	C'-9"	0'-9"	0'-9"
н	2'-3"	2"-11"	3'-2"	3'-9"	4'-3"	4'-9"	ı	Н	2'-10"	3'-3"	3'-7"	3'-9"	4'-0"	4'-2"	4'-5"	4'-7"
L	4'-0"	5'-0"	6°-0°	81-071	10'-0"	12'-0"	ı	L	5"-5"	7'-2"	8*-6"	9'-2"	10'-2"	101-11 ¹⁴	12'-1"	12"-17"
a	0'-1/4"	0'-1%"	0'-2"	0'-2 /2°	0*-31/4*	0'-3%"	ı	s	17-117	2"-6"	2'-10"	3'-2"	3'-6"	3'-9"	41-91	4'-5"
b	0'-1"	0"-1/4"	0'-1/2"	01-211	0'-21/2"	0'-3"	ı	0	0'-21/2"	0.3/4	0'-3//2"	0'-4"	0"-41/2"	0'-434"	0'-5"	0*-51/2*
		CUBIC	YARDS 0	F CONCRET			ı	ь	0'-2"	0'-21/2"	0'-274"	0'-3"	0'-3½"	0'-3¾"	0'-4"	0'-41/2"
NG. PIPE	0.241	0.492	0.697	1.319	2.087	2.947	ı	İ			CUBIC Y	ARDS OF C	ONCRETE			
.M. PIPE	0.257	0.521	0.739	1.398	2.198	3:145	ı	CONC.PIPE	0.502	0.855	1.235	1.500	1.611	2.101	2.512	2.801
PECIFICAT REFERENC	ION E		ST	ANDA	RD E	NDWAL	Ĺ	FOR	PIPE	CUL	VERT	s		ROAD AND	/DOT	ANDARDS

12" - 36" CIRCULAR AND 23" x 14" - 53" x 34" ELLIPTICAL PIPES











SYMBOLS

STORM MANHOLE TELEPHONE MANHOLE

TREE

■ UTILITY PEDESTAL Ø UTILITY POLE

BOLLARD BUSH CAP Ð ELECTRIC BOX ELECTRIC MANHOLE UTILITY BOX

FIRE HYDRANT GAS METER

○ LIGHT POLE POWER POLE

WATER METER WATER VALVE YARD LIGHT

_____ EXISTING GROUND EAST SIDE EXISTING GROUND CENTERLINE EXISTING GROUND WEST SIDE PROPERTY LINE

SEWER EASEMENT OVERHEAD ELECTRIC OVERHEAD CABLE TV UNDERGROUND ELECTRIC UNDERGROUND TELEPHONE

DATE: MARCH 12, 2018 DRAWN BY: K. ATKINSON





VIRGINIA | NORTH CAROLINA | WEST VIRGINIA THIS DRAWING PREPARED AT THE DOWNTOWN OFFICE

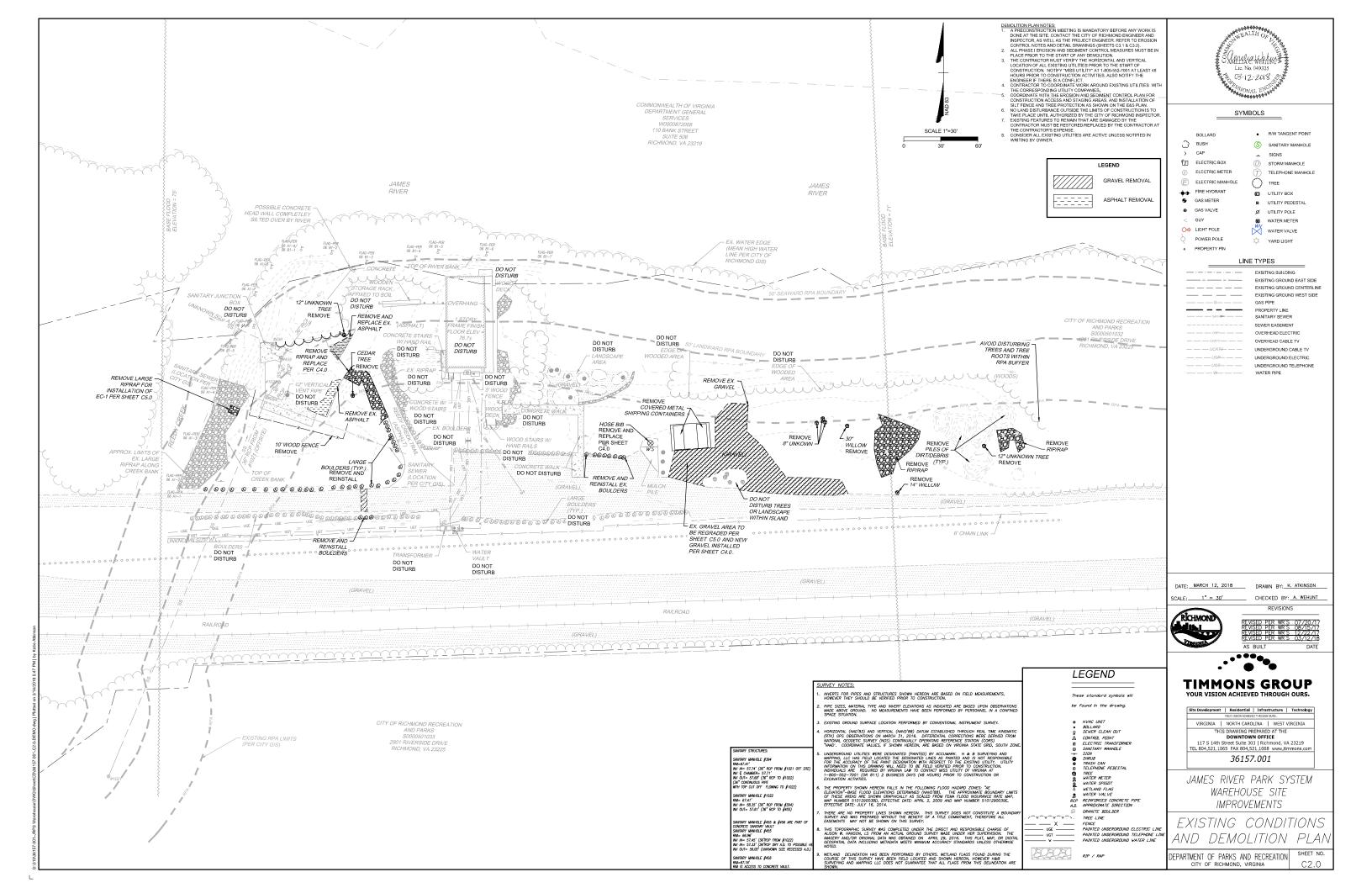
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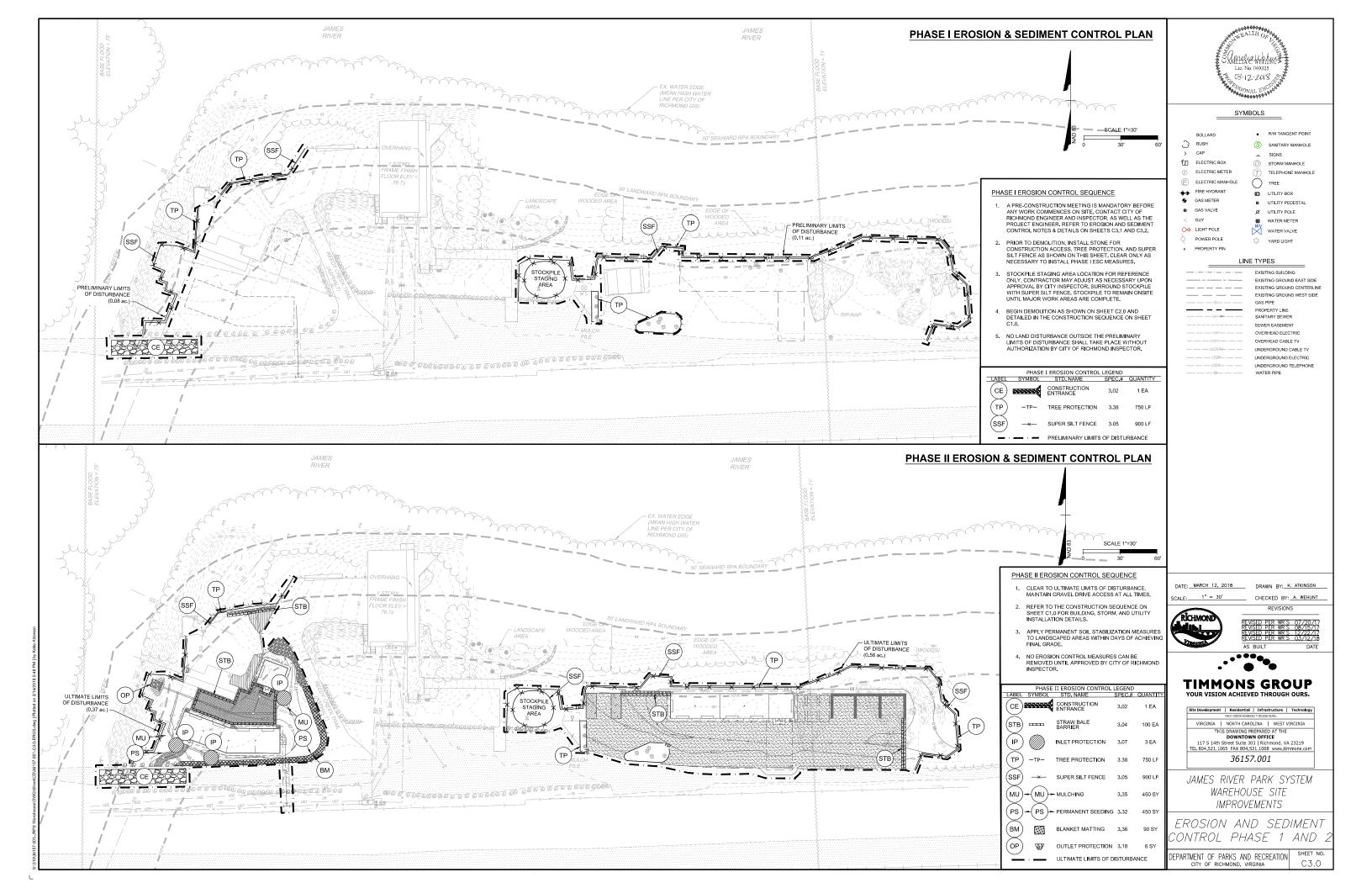
JAMES RIVER PARK SYSTEM WAREHOUSE SITE *IMPROVEMENTS*

> NOTES AND DETAILS

> > SHEET NO.

DEPARTMENT OF PARKS AND RECREATION CITY OF RICHMOND, VIRGINIA





- MAINTENANCE BULLDING: TRACTORS, SMALL FUEL CONTAINERS, MAINTENANCE TOOLS, MOWERS, BLOWERS, EDGERS, AND OTHER SIMILAR MAINTENANCE EQUIPMENT.

THE TOTAL DISTURBED AREA WITHIN THIS PROJECT IS 0.93 ACRES.

EXISTING SITE CONDITIONS

EXISTING STILE CONDITIONS
THE SITE LIES ADJACENT TO THE JAMES RIVER AND SERVES AS THE JAMES RIVER PARK SYSTEM
HEADQUARTERS. THE SITE CONSISTS OF ONE PARCEL AND CONTAINS A ONE-STORY BUILDING USED
AS THE JRPS OFFICE SPACE. THE SITE IS FILLED WITH SEVERAL STORAGE CONTAINERS, MATERIAL
STOCKPIELS, MAINTENANCE EQUIPMENT, AND PILES OF DEBRIS, ZONING AREAS PLACE THE
HEADQUARTERS WITHIN A SINGLE-FAMILY RESIDENTIAL DISTRICT.

THE MAJORITY OF THE SITE IS IMPERVIOUS CONTAINING ASPHALT AND GRAVEL ACCESS ROADWAYS AND PARKING AREAS, AND MINIMAL LANDSCAPED AREAS.

THERE IS NO EXISTING DRAINAGE INFRASTRUCTURE. THE SITE SHEET FLOWS DIRECTLY INTO THE JAMES RIVER AND IS LOCATED WITHIN THE 100YEAR FLOODPLAIN

THE EXISTING SITE TOPOGRAPHY SHOWS DRAINAGE FROM THE SITE TOWARDS THE JAMES RIVER AT APPROXIMATELY A 10% SLOPE. THE REMAINDER OF THE SITE RANGES FROM A 1% TO 2% SLOPE

THE MAJORITY OF THE EXISTING VEGETATION IS WITHIN THE SEAWARD 50' OF THE RPA AND PART OF A FOREST STAND CONTAINING SYCAMORE, OAK, MAPLE, CHERRY, HICKORY, AND OTHER HARDWOOD SPECIES TYPICAL OF THIS REGION, THE MAJORITY OF THIS VEGETATION HAS BEEN OVERTAKEN BY IVY AND OTHER INVASIVE SPECIES AND IS IN POOR CONDITION. BEYOND THE FOREST STAND, VEGETATION INCLUDES SOME SHRUB/SCRUB CLUSTERS AS WELL AS SEVERAL SMALL DECIDIOUS AND EVERGREEN TREES THAT HAVE BEEN NOTED ON THE EXISTING CONDITIONS PLAN SHEET ON C2.0.

ADJACENT SITE

THE SITE IS BOUND TO THE NORTH BY THE JAMES RIVER; TO THE EAST BY REEDY CREEK WALKING TRAIL; TO THE WEST BY REEDY CREEK AND FOREST HILL NEIGHBORHOOD; AND TO THE SOUTH BY WOODLAND HEIGHTS NEIGHBORHOOD AND FOREST HILL PARK.

OFF-SITE AREAS OFFSITE BORROW WILL NOT BE REQUIRED FOR THE CONSTRUCTION OF THIS PROJECT.

 \underline{SOILS} THE ENTIRE SITE BOUNDARY IS LOCATED WITHIN SOIL TYPE 40 (UDORTHENTS-DUMPS COMPLEX) .

THE UDORTHENTS-DUMPS COMPLEX, WHICH IS 50% UDORTHENTS AND 50% DUMPS COMPRISES 100% OF THE SITE MAP. THIS SOIL IS NOT PRIME FARMLAND, DEPTH TO RESTRICTIVE FEATURE IS MORE THAN 80 INCHES. DEPTH TO WATER TABLE IS ALSO MORE THAN 80 INCHES. DES, SHRINK-SWELL POTENTIAL IS LOW. THERE IS NO ZONE OF WATER SATURATION WITHIN A DEPTH OF 72 INCHES. THERE IS NO FREQUENCY OF FLOODING OR PONDING.

CRITICAL AREAS
THE PROJECT LIMITS LIE WITHIN AN RPA BOUNDARY.

STORMWATER RUN-OFF CONSIDERATIONS

EXISTING DRAINAGE OUTFALL PATTERNS WILL BE MAINTAINED. STORMWATER WILL BE CONVEYED THROUGH SHALLOW SWALES AND A PROPOSED STORM SYSTEM THAT DISCHARGE INTO GRAVEL DIAPHRAGMS AND SHEETFLOW INTO THE 100 YEAR FLOOD PLAIN OF REEDY CREEK AND THE JAMES

RSMP/VSMP CONSIDERATIONS
THE SITE DISTURBANCE FOR THIS PROJECT IS 0.93 AC. THE PROJECT IS LOCATED WITHIN THE
CHESAPEAKE BAY AREA. AN RSMP PERMIT WILL BE REQUIRED, REFER TO SHEET C6.0 FOR
STORMWATER MANAGEMENT CALCULATIONS.

CHESAPEAKE BAY NARRATIVE

THIS PROJECT IS DESIGNED TO BE COMPLETED WITH LIMITED LAND DISTURBANCE AND MINIMAL IMPACT ON EXISTING VEGETATION. EXISTING CONDITIONS WITHIN THE PROJECT LIMITS ARE AN EXISTING BUILDING, GRAVEL PARKING, STORAGE CONTAINERS, MAINTENANCE EQUIPMENT ALL SURROUNDED BY MINIMAL GRASSY AREAS. THE TOTAL LAND DISTURBANCE IS 0.93ac (35,719 sf). ALL NEW IMPERVIOUS AREA WILL DRAIN THROUGH SHALLOW SWALES AND A PROPOSED STORM SYSTEM THAT DISCHARGE INTO GRAVEL DIAPHRAGMS AND SHEETFLOW INTO THE FLOODPLAIN AT REEDY CREEK AND THE JAMES RIVER SILT FENCE INLET PROTECTION AND OUTLET PROTECTION MILL BE INSTALLED THROUGHOUT CONSTRUCTION TO PROHEID TS SEDIMENT RUNOF FROM LEAVING THE PROJECTION TO PROHEID SEDIMENT RUNOF FROM LEAVING THE PROJECT AREA, ALL DISTURBED GRASS AREAS WILL BE STABILIZED WITH TOPSOIL, SEED, AND STRAW, ALL STEEP GRASS AREAS WILL BE STABILIZED WITH VDOT EC-3 MATTING. REFER TO SHEET C3.0 FOR PHASED EROSION CONTROL PLANS.

PERMANENT STABILIZATION
ALL NON-PAVED AREAS DISTURBED BY CONSTRUCTION SHALL BE STABILIZED WITH PERMANENT SEEDING IMMEDIATELY FOLLOWING FINISHED GRADE. TOPSOIL SHALL BE PLACED TO A DEPTH OF 6" & SEEDING SHALL BE IN ACCORDANCE WITH STD. & SPEC. 3.32. PERMANENT SEEDING, IMPORTED TOPSOIL SHALL BE OBTAINED FROM A SITE WITH AN APPROVED ESC PLAN. SEED TYPE SHALL BE AS SPECIFIED FOR "MINIMUM CARE LAWNS" AND "GENERAL SLOPES" IN THE HANDBOOK, MULCH (STRAW OR FIBER) IN ACCORDANCE WITH STD. & SPEC. 3.35. SHALL BE USED ON ALL SEEDED SURFACES. EROSION CONTROL BLANKETS WILL BE INSTALLED OVER FILL SLOPES WHICH HAV BEEN BROUGHT TO FINAL GRADE AND HAVE BEEN SEEDED TO PROTECT THE SLOPES WIND HAVE BEEN SEEDED TO PROTECT THE SLOPES FROM RILL & GULLY EROSION AND TO ALLOW THE SEED TO GERMINATE PROPERLY. IN ALL SEEDING DEPARTIONS OF THE SOLL TEST PRIOR RECOMMENDATIONS OF THE SOLL TEST PRIOR

EROSION AND SEDIMENT CONTROL MEASURES
UNLESS OTHERWISE INDICATED, ALL VEGETATIVE AND STRUCTURAL EROSION AND SEDIMENT
CONTROL PRACTICES SHALL BE CONSTRUCTED AND MAINTAINED ACCORDING TO MINIMUM
STANDARDS AND SPECIFICATIONS OF THE VIRGINIA EROSION AND SEDIMENT CONTROL HANDBOOK.
THE MINIMUM STANDARDS OF THE VESCH SHALL BE ADHERED TO UNLESS OTHERWISE WAIVED OR
APPROVED BY A VARIANCE BY LOCAL AUTHORNIES HAVING JURISDICTION.

STRUCTURAL PRACTICES

 CONSTRUCTION ENTRANCE - 3.02
 A STONE PAD. LOCATED AT POINTS OF VEHICULAR INGRESS AND EGRESS ON A CONSTRUCTION SITE, TO REDUCE THE SOIL TRANSPORTED ONTO PUBLIC ROADS AND

2. STRAW BALE BARRIER - 3.04
A TEMPORARY SEDMENT BARRIER COMPOSED OF A ROW OF ENTRENCHED AND
ANCHORED STRAW BALES. APPLICABLE WHERE SHEET AND RILL EROSION MAY BE A PROBLEM IN DRAINAGE AREAS OF LIMITED SIZE, STRAW BALE BARRIERS ARE PLACED ACROSS OR AT THE TOE OF A SLOPE TO INTERCEPT AND DETAIN SEDIMENT AND DECREASE SHEET FLOW VELOCITIES. MAXIMUM EFFECTIVE LIFE IS 3 MONTHS.

 SUPER SILT FENCE BARRIER - 3.05
 A TEMPORARY SEDIMENT BARRIER CONSISTING OF A SYNTHETIC FILTER FABRIC STRETCHED ACROSS AND ATTACHED TO SUPPORTING POSTS AND ENTRENCHED

DROP INLETS OR CURB INLET STRUCTURES PRIOR TO PERMANENT STABILIZATION OF THE DISTURBED AREA; LIMITED TO DRAINAGE AREAS NOT EXCEEDING ONE ACRE, AND NOT INTENDED TO CONTROL LARGE, CONCENTRATED STORMWATER FLOWS.

STORM DRAIN OUTLETS TO REDUCE EROSION AND UNDER-CUTTING FROM SCOURING AT OUTLETS AND TO REDUCE FLOW VELOCITIES BEFORE STORMWATER ENTERS RECEIVING CHANNELS BELOW THESE OUTLETS.

 SOIL STABILIZATION BLANKETS AND MATTING - 3.36
 THE INSTALLATION OF A PROTECTIVE BLANKET (TREATMENT 1) OR A SOIL STABILIZATION MAT (TREATMENT 2) ON A PREPARED PLANTING OF A STEEP SLOPE, CHANNEL OF

7. TREE PRESERVATION AND PROTECTION - 3.38
PROTECTING EXISTING TREES FROM MECHANICAL AND OTHER INJURY DURING LAND-DISTURBING AND CONSTRUCTION ACTIVITY TO ENSURE THE SURVIVAL OF DESIRABLE TREES WHERE THEY WILL BE EFFECTIVE FOR EROSION AND SEDIMENT CONTROL AND PROVIDE OTHER ENVIRONMENTAL AND AESTHETIC BENEFITS.

5. PERMANENT SEEDING - 3.32
ESTABLISHMENT OF PERENNIAL VEGETATIVE COVER BY PLANTING SEED ON ESTABLISHMENT OF FERENMENT USE OF THE COVER BY FERMING SEED ON MORE OR WHERE PERMANENT, LONG-LIVED VEGETATIVE COVER IS NEEDED ON

 MULCHING - 3.35
 APPLICATION OF PLANT RESIDUES OR OTHER SUITABLE MATERIALS TO DISTURBED
 TO SUITABLE MATERIALS TO DISTURBED BY THE PLANT OF THE PLANT SURFACES TO PREVENT EROSION AND REDUCE OVERLAND FLOW VELOCITIES. FOSTERS PLANT GROWTH BY INCREASING AVAILABLE MOISTURE AND PROVIDING INSULATION AGAINST EXTREME HEAT OR COLD, SHOULD BE APPLIED TO ALL SEEDING OPERATIONS OTHER PLANT MATERIALS WHICH DO NOT PROVIDE ADEQUATE SOIL PROTECTION BY THEMSELVES AND BARE AREAS WHICH CANNOT BE SEEDED DUE TO THE SEASON BUT WHICH STILL NEED PROTECTION TO PREVENT SOIL LOSS.

ESC CONTROL MEASURE MAINTENANCE

(REFER TO "MINIMUM STANDARDS" FOR ADD. INFORMATION).

- CONSTRUCTION ENTRANCE 3.02
 7.1. THE ENTRANCE SHALL BE MAINTAINED IN A CONDITION WHICH WILL PREVENT
 TRACKING OR FLOW OF MUD ONTO PUBLIC RIGHTS-OF-WAY THIS MAY REQUIRE PERIODIC REPAIR AND/OR CLEANOUT OF ANY STRUCTURES USED TO TRAP
- S.E.JIMEN.I.
 7.2. ALL MATERIALS SPILLED, DROPPED, WASHED, OR TRACKED FROM VEHICLES ONTO ROADWAYS OR INTO STORM DRAINS MUST BE REMOVED IMMEDIATELY.
 7.3. THE USE OF WATER TRUCKS TO REMOVE MATERIALS DROPPED, WASHED, OR TRACKED ONTO ROADWAYS WILL NOT BE PERMITTED UNDER ANY CIRCUMSTANCES.

- STRAW BALE BARRIER 3.04
 8.1. STRAW BALE BARRIERS SHALL BE INSPECTED IMMEDIATELY AFTER EACH RAINFALL
- AND AT LEAST DAILY DURING PROLONGED RAINFALL.

 8.2. CLOSE ATTENTION SHALL BE PAID TO THE REPAIR OF DAMAGED BALES, END RUNS,
- 8.2. CLOSE AT IERH ION SHALL BE PAID TO THE REPAIR OF DAMAGED BALES, EN AND UNDERCUTTING BENEATH BALES.
 8.3. NECESSARY REPAIRS TO BARRIERS OR REPLACEMENT OF BALES SHALL BE ACCOMPLISHED PROMPTLY.
- REMOVED WHEN THE LEVEL OF DEPOSITION REACHES ONE-HALF THE HEIGHT OF THE
- 8.5. ANY SEDIMENT DEPOSITS REMAINING IN PLACE AFTER THE STRAW BALE BARRIER IS NO LONGER REQUIRED SHALL BE DRESSED TO CONFORM TO THE EXISTING GRADE, PREPARED, AND SEEDED.

- SUPER SILT FENCE BARRIER 3.05
 9.1. SILT FENCES SHALL BE INSPECTED IMMEDIATELY AFTER EACH RAINFALL AND AT LEAST DAILY DURNING PROLONGED RAINFALL. ANY REQUIRED REPAIRS SHALL BE MADE IMMEDIATELY.

 9.2. CLOSE ATTENTION SHALL BE PAID TO THE REPAIR OF DAMAGED SILT FENCE
- RESULTING FROM END RUNS AND UNDERCUTTING.
 9.3. SHOULD THE FABRIC ON A SILT FENCE DECOMPOSE OR BECOME INEFFECTIVE PRIOR TO THE END OF THE EXPECTED USABLE LIFE AND THE BARRIER STILL BE NECESSARY, THE FARRIC SHALL BE REPLACED PROMPTLY
- 9.4 SEDIMENT DEPOSITS SHOULD BE REMOVED AFTER EACH STORM EVENT. THEY MUST
- 9.5. ANY SEDIMENT DEPOSITS REMAINING IN PLACE AFTER THE SILT FENCE IS NO LONGER REQUIRED SHALL BE DRESSED TO CONFORM WITH THE EXISTING GRADE, PREPARED

- 10. STORM DRAIN INLET PROTECTION 3.07
 10.1.THE STRUCTURE SHALL BE INSPECTED AFTER EACH RAIN AND REPAIRS MADE AS
- 10.2 SEDIMENT SHALL BE REMOVED AND THE TRAP RESTORED TO ITS ORIGINAL 10.2.SEDIMENT SHALL BE REMOVED AND THE TRAP RESTORED TO ITS ORIGINAL DIMENSIONS WHEN THE SEDIMENT HAS ACCUMULATED TO ONE HALF THE DESIGN DEPTH OF THE TRAP. REMOVED SEDIMENT SHALL BE DEPOSITED IN A SUITABLE AREA AND IN SUCH A MANNER THAT IT WILL NOT ERODE.
 10.3.STRUCTURES SHALL BE REMOVED AND THE AREA STABILIZED WHEN THE REMAINING DRAINAGE AREA HAS BEEN PROPERLY STABILIZED.

- 11. OUTLET PROTECTION 3.18
 11.1.OUTLET PROTECTION SHOULD BE CHECKED REGULARLY FOR SEDIMENT ACCUMULATION OR VEGETATION GROWTH WITHIN THE STONE. SEDIMENT AND VEGETATION SHOULD BE REMOVED WHEN EITHER IS BLOCKING OR REDIRECTING
- 11.2 EROSION CAUSED BY HIGH FLOWS AROUND THE EDGES OF THE RIPRAP SHOULD ALSO BE CORRECTED IMMEDIATELY.

12. SOIL STABILIZATION BLANKETS AND MATTING - 3.36
12.1.ALL SOIL STABILIZATION BLANKETS AND MATTING SHOULD BE INSPECTED PERIODICALLY FOLLOWING INSTALLATION, PARTICULARLY AFTER RAINSTORMS TO CHECK FOR EROSION AND UNDERMINING, ANY DISLOCATION OR FAILURE SHOULD BE REPAIRED IMMEDIATELY, IF WASHOUTS OR BREAKAGE OCCURS, REINSTALL THE MATERIAL AFTER REPAIRING DAMAGE TO THE SLOPE OR DITCH. CONTINUE TO MONITOR THESE AREAS LINTIL WHICH TIME THEY BECOME PERMANENTLY STABILIZED

- 13. TREE PROTECTION -3.38
 13.1.TREE PROTECTION SHOULD BE INSPECTED PERIODICALLY FOLLOWING INSTALLATION, PARTICULARLY AFTER RAINSTORMS TO CHECK FOR AREAS OF DAMAGE. ANY DISLOCATION, FAILURE, OR BREAKAGE THAT OCCURS SHOULD BE
- 13.2.IF. IN SPITE OF ALL PRECAUTIONS, DAMAGE TO PROTECTED TREES OCCURS. FOLLOW THE MAINTENANCE GUIDELINES PER THE VIRGINIA 1992 EROSION AND SEDIMENT CONTROL HANDBOOK STANDARD AND SPECIFICATION 3.38.

GENERAL EROSION AND SEDIMENT CONTROL NOTES

- ES-1: UNLESS OTHERWISE INDICATED, CONSTRUCT AND MAINTAIN ALL VEGETATIVE AND STRUCTURAL EROSION AND SEDIMENT CONTROL PRACTICES ACCORDING TO MINIMUM STANDARDS AND SPECIFICATIONS OF THE VIRGINIA EROSION AND SEDIMENT CONTROL HANDBOOK AND VIRGINIA REGULATIONS 9 VAC 25-840-40
- ES-2: NOTIFY THE DEPARTMENT OF PUBLIC UTILITIES ONE WEEK PRIOR TO THE PRE-CONSTRUCTION CONFERENCE. ONE WEEK PRIOR TO THE COMMENCEMENT OF LAND DISTURBING ACTIVITY, AND ONE WEEK PRIOR TO THE FINAL INSPECTION.
- ES-3: PLACE ALL EROSION AND SEDIMENT CONTROL MEASURES PRIOR TO OR AS THE FIRST STEP IN CLEARING, GRADING, OR LAND DISTURBANCE. ES-4: MAINTAIN A COPY OF THE APPROVED EROSION AND SEDIMENT CONTROL PLAN ON THE SITE AT ALL TIMES.
- ES-5: PRIOR TO COMMENCING LAND-DISTURBING ACTIVITIES IN AREAS OTHER THAN INDICATED ON THESE PLANS (INCLUDING, BUT NOT LIMITED TO, OFFSITE BORROW OR WASTE AREA), THE CONTRACTOR SHALL SUBMIT A SUPPLEMENTARY EROSION CONTROL PLAN TO THE

ARCHITECT/ENGINEER FOR REVIEW AND APPROVAL BY THE PLAN APPROVING AUTHORITY

- ES-6: PROVIDE ADDITIONAL EROSION CONTROL MEASURES NECESSARY TO PREVENT EROSION AND SEDIMENTATION AS DETERMINED BY THE DPU INSPECTOR.
- ES-7: ALL DISTURBED AREAS ARE TO DRAIN TO APPROVED SEDIMENT CONTROL MEASURES AT ALL TIMES DURING LAND-DISTURBING ACTIVITIES AND DURING SITE DEVELOPMENT UNTIL FINAL STABILIZATION IS ACHIEVED.
- ES-8: DURING DEWATERING OPERATIONS, PUMP WATER INTO AN APPROVED FILTERING DEVICE
- ES-9: INSPECT ALL EROSION CONTROL MEASURES DAILY AND AFTER EACH RUNOFF-PRODUCING RAINFALL EVENT. MAKE ANY NECESSARY REPAIRS OR CLEANUP TO MAINTAIN THE EFFECTIVENESS OF THE EROSION CONTROL DEVICES IMMEDIATELY.

CITY OF RICHMOND STANDARD E&S NOTES

- 5. PERMANENT OR TEMPORARY SOIL STABILIZATION SHALL BE APPLIED TO DENUDED AREAS WITHIN SEVEN DAYS AFTER FINAL GRADE IS REACHED ON ANY PORTION OF THE SITE.

 THE STREET STABILIZATION SHALL BE APPLIED WITHIN SEVEN DAYS TO DENUDED ARRAS THAT MAY NOT BE AT FINAL GRADE BUT WILL REMAIN, DORMANT (UNDISTURBED) FOR LONGER THAN 14 DAYS, PERMANENT STABILIZATION SHALL BE APPLIED TO AREAS THAT ARE TO BE LEFT DORMANT FOR MORE THAN ONE YEAR
- THAT ARE TO BE LEFT DORMANT FOR MORE THAN ONE YEAR.

 EXCESS EXCAVATION DISPOSED OF OFF THE SITE SHALL BE DISPOSED OF IN

 ACCORDANCE WITH THE VIRGINIA EROSION AND SEDIMENT CONTROL HANDBOOK.

 EROSION AND SEDIMENT CONTROLS SHALL BE INSTALLED IN ACCORDANCE WITH

 VIRGINIA EROSION AND SEDIMENT CONTROL HANDBOOK AND SHALL BE PLACED PRIOR TO

 OR AS THE FIRST STEP OF THE LAND DISTURBING ACTIVITY.

 EDOSION AND SEDIMENT CONTROLS SHALL BE MANTANIES OF THAT THE SEDIMENT
- EROSION AND SEDIMENT CONTROLS SHALL BE MAINTAINED SO THAT THE SEDIMENT CARRYING RUNOFF FROM THE SITE WILL NOT ENTER STORM DRAINAGE FACILITIES
- 9. EROSION AND SEDIMENT CONTROLS SHALL BE MAINTAINED UNTIL THE DISTURBED AREA IS STABILIZED. 10. PROPERTIES ADJOINING THE SITE SHALL BE KEPT CLEAN OF MUD OR SILT CARRIED FROM
- PROPERTIES ADJOINING THE SITE SHALL BE KEPT CLEAN OF MUD OR SILT CARRIED FROM
 THE SITE BY VEHICULAR TRAFFIC OR RUNOFF.
 THE DISPOSAL OF WASTE MATERIALS REMOVED FROM EROSION AND SEDIMENT CONTROL
 FACILITIES AND THE DISPOSAL OF THESE FACILITIES SHALL BE IN ACCORDANCE WITH THE
 VIRIGINIA EROSION AND SEDIMENT CONTROL HANDBOOK.
 STABILIZATION MEASURES SHALL BE APPLIED TO EARTHEN STRUCTURES SUCH AS DAMS,
- DIKES AND DIVERSIONS IMMEDIATELY AFTER INSTALLATION
- 13. DURING CONSTRUCTION OF THE PROJECT, SOIL STOCKPILES SHALL BE STABILIZED OR PROTECTED WITH SEDIMENT TRAPPING MEASURES. THE APPLICANT IS RESPONSIBLE EROSKONE & SEDEDIANE NOTOCONOTROD MAINIMENT STANDARDS ALL SOIL
- STAPIC PROCESSION IN PROCESSION TO THE STAP AREAS FOR THE STAPE STAPIC PROCESSION OF THE STAP AREAS FOR THE BUT WILL REMAIN DORMANT (UNDISTURBED) FOR LONGER THAN 14 DAYS, APPLY PERMANENT STABILIZATION TO AREAS THAT ARE TO BE LEFT DORMANT FOR MORE THAN ONE YEAR.
- MS-2: STABILIZE OR PROTECT TEMPORARY SOIL STOCKPILES WITH SEDIMENT TRAPPING MEASURES, PROVIDE TEMPORARY PROTECTION AND PERMANENT STABILIZATION OF ALL SOIL STOCKPILES ON SITE AS WELL AS SOIL TRANSPORTED FROM THE PROJECT SITE.
- MS-3: ESTABLISH A PERMANENT VEGETATIVE COVER ON DENLIDED AREAS NOT OTHERWISE PERMANENTLY STABILIZED. PERMANENT VEGETATION SHALL NOT BE CONSIDERED ESTABLISHED LINTIL. A GROUND COVER IS ACHIEVED THAT IN THE OPINION OF THE
- MS-4: CONSTRUCT SEDIMENT BASINS AND TRAPS, PERIMETER DIKES, SEDIMENT BARRIERS AND OTHER MEASURES INTENDED TO TRAP SEDIMENT AS A FIRST STEP IN ANY LAND-DISTURBING ACTIVITY AND MAKE THESE MEASURES FUNCTIONAL BEFORE UP-SLOPE LAND DISTURBANCE
- MS-5: APPLY STABILIZATION MEASURES TO EARTHEN STRUCTURES SUCH AS DAMS, DIKES AND DIVERSIONS IMMEDIATELY AFTER INSTALLATION.
- MS-6 DESIGN AND CONSTRUCT SEDIMENT TRAPS AND SEDIMENT BASINS BASED UPON THE TOTAL
 - a. DESIGN THE MINIMUM STORAGE CAPACITY OF THE SEDIMENT TRAP TO BE 134 CUBIC YARDS PER ACRE OF DRAINAGE AREA AND THE MAXIMUM DRAINAGE AREA TO THE TRAP BE LESS THAN THREE ACRES.
 - b. DESIGN THE SEDIMENT BASIN TO CONTROL SURFACE RUNOFF FROM DISTURBED AREAS COMPRISED OF FLOW FROM DRAINAGE AREAS GREATER THAN OR EQUAL TO THREE ACRES. ENSURE THE MINIMUM STORAGE CAPACITY OF THE SEDIMENT BASIN TO BE 134 CUBIC YARDS PER ACRE OF DRAINAGE AREA CONSTRUCT THE OUTFALL SYSTEM TO AT MINIM IM MAINTAIN THE STRUCTURAL INTEGRITY OF THE BASIN DURING A 25-YEAR STORM OF 24-HOUR DURATION, WHEN COMPLETING RUNOFF CALCULATIONS, APPLY
- MS-7: CONSTRUCT CUT AND FILL SLOPES IN A MANNER THAT WILL MINIMIZE EROSION. PROVIDE SLOPES THAT ARE FOUND TO BE ERODING EXCESSIVELY WITHIN ONE YEAR OF PERMANENT STABILIZATION WITH ADDITIONAL SLOPE STABILIZING MEASURES UNTIL THE PROBLEM IS
- MS-8: CONCENTRATED RUNOFF SHALL NOT FLOW DOWN CUT OR FILL SLOPES UNLESS CONTAINED WITHIN AN ADEQUATE TEMPORARY OR PERMANENT CHANNEL, FLUME OR SLOPE DRAIN
- MS-9: WHENEVER WATER SEEPS FROM A SLOPE FACE, PROVIDE ADEQUATE DRAINAGE OR OTHER
- MS-10: PROTECT ALL STORM SEWER INLETS THAT ARE MADE OPERABLE DURING CONSTRUCTION SO THAT SEDIMENT-LADEN WATER CANNOT ENTER THE CONVEYANCE SYSTEM WITHOUT FIRST BEING FILTERED OR OTHERWISE TREATED TO REMOVE SEDIMENT
- MS-11: BEFORE STORMWATER CONVEYANCE CHANNELS ARE MADE OPERATIONAL, PROVIDE ADEQUATE OUTLET PROTECTION AND ANY REQUIRED TEMPORARY OR PERMANENT CHANNEL LINING IN BOTH THE CONVEYANCE CHANNEL AND THE RECEIVING CHANNEL.
- MS-12: WHEN WORK IN A LIVE WATERCOURSE IS PERFORMED. TAKE PRECAUTIONS TO MINIMIZE ENCROACHMENT, CONTROL SEDMENT TRANSPORT AND STABILIZE THE WORK AREA TO THE GREATEST EXTENT POSSIBLE DURING CONSTRUCTION, PROVIDE NON-ERODABLE MATERIAL FOR THE CONSTRUCTION OF CAUSEWAYS AND COFFERDAMS. EARTHEN FILL MAY BE USED
- MS-13: WHEN A LIVE WATERCOURSE MUST BE CROSSED BY CONSTRUCTION VEHICLES MORE THAN TWICE IN ANY SIX-MONTH PERIOD, PROVIDE A TEMPORARY STREAM CROSSING CONSTRUCTED OF NON-ERODABLE MATERIAL.
- MS-14: MEET ALL APPLICABLE FEDERAL, STATE AND LOCAL REGULATIONS PERTAINING TO WORKING
- MS-15:STABILIZE THE BED AND BANKS OF A WATERCOURSE IMMEDIATELY FOLLOWING AFTER WORK IN THE WATERCOURSE IS COMPLETED.
- MS-16: UNDERGROUND UTILITY LINES SHALL BE INSTALLED IN ACCORDANCE WITH THESE STANDARDS IN ADDITION TO OTHER APPLICABLE CRITERIA
 - a. OPEN NO MORE THAN 500 LINEAR FEET OF TRENCH AT ONE TIME. b. PLACE EXCAVATED MATERIAL ON THE UPHILL SIDE OF TRENCHES.
- c. FILTER EFFLUENT FROM DEWATERING OPERATIONS OR PASS THROUGH AN APPROVED SEDIMENT TRAPPING DEVICE, OR BOTH, AND DISCHARGE IN A MANNER THAT DOES NOT

ADVERSELY AFFECT FLOWING STREAMS OR OFFSITE PROPERTY.

- d. PROPERLY COMPACT MATERIAL USED FOR BACKFILLING TRENCHES IN ORDER TO MINIMIZE EROSION AND PROMOTE STABILIZATION.
- e. ACCOMPLISH RESTABILIZATION IN ACCORDANCE WITH THIS CHAPTER
- f. COMPLY WITH ALL APPLICABLE SAFETY REGULATIONS.

EROSION & SEDIMENT CONTROL MINIMUM STANDARDS (CONT'D)

MS-17: WHERE CONSTRUCTION VEHICLE ACCESS ROUTES INTERSECT PAVED PUBLIC ROADS, MAK PROVISIONS TO MINIMIZE THE TRANSPORT OF SEDIMENT BY VEHICULAR TRACKING ONTO THE PAVED SURFACE. WHERE SEDIMENT IS TRANSPORTED ONTO A PUBLIC ROAD SURFACE, CLEAN THE ROAD THOROUGHLY AT THE END OF EACH DAY, REMOVE SEDIMENT FROM THE ROADS BY SHOVELING OR SWEEPING AND TRANSPORT TO A SEDIMENT CONTROL DISPOSAL AREA. STREET WASHING SHALL BE ALLOWED ONLY AFTER SEDIMENT IS REMOVED IN THIS

: REMOVE ALL TEMPORARY EROSION AND SEDIMENT CONTROL MEASURES WITHIN 30 DAYS AFTER FINAL SITE STABILIZATION OR AFTER THE TEMPORARY MEASURES ARE NO LONGER NEEDED, UNLESS OTHERWISE AUTHORIZED BY THE LOCAL AUTHORITY HAVING JURISDICTION. PERMANENTLY STABILIZE TRAPPED SEDIMENT AND THE DISTURBED SOIL AREAS RESULTING FROM THE DISPOSITION OF TEMPORARY MEASURES TO PREVENT FURTHER EROSION AND SEDIMENTATION.

MS-18: REMOVE ALL TEMPORARY EROSION AND SEDIMENT CONTROL MEASURES. WITHIN 30 DAYS

- MS-19: PROPERTIES AND WATERWAYS DOWNSTREAM FROM DEVELOPMENT SITES SHALL BE PROTECTIES AND WATERWAYS DEVINED REASON AND DAMAGE DUE TO INCREASES IN VOLUME, VELOCITY AND PEAK FLOW RATE OF STORMWATER RUNOFF FOR THE STATED FREQUENCY STORM OF 24-HOUR DURATION IN ACCORDANCE WITH THE FOLLOWING STANDARDS AND CRITERIA:
 - a CONCENTRATED STORMWATER RUNOFF LEAVING A DEVELOPMENT SITE SHALL BE CONCENTRAL LED STOKMWALER KONOFF LEAVING A DEVELOPMENT STIF SPALL BE DISCHARGED DIRECTLY INTO AN ADEQUATE NATURAL OR MAN-MADE RECEIVING CHANNEL, PIPE OR STORM SEWER SYSTEM. FOR THOSE SITES WHERE RUNOFF IS DISCHARGED INTO A PIPE OR PIPE SYSTEM, DOWNSTREAM STABILITY ANALYSES AT THE OUTFALL OF THE PIPE OR PIPE SYSTEM SHALL BE PERFORMED.
 - b. ADEQUACY OF ALL CHANNELS AND PIPES SHALL BE VERIFIED IN THE FOLLOWING
 - b.1. THE APPLICANT SHALL DEMONSTRATE THAT THE TOTAL DRAINAGE AREA TO THE POINT OF ANALYSIS WITHIN THE CHANNEL IS ONE HUNDRED TIMES GREATER THAN THE CONTRIBUTING DRAINAGE AREA OF THE PROJECT IN QUESTION; OR
 - b.2. NATURAL CHANNELS SHALL BE ANALYZED BY THE USE OF A TWO-YEAR FREQUENCY STORM TO VERIFY THAT STORMWATER WILL NOT OVERTOP CHANNEL BANKS NOR CAUSE EROSION OF CHANNEL BED OR BANKS. b.3. ALL PREVIOUSLY CONSTRUCTED MAN-MADE CHANNELS SHALL BE ANALYZED BY THE
 - ALL PREVIOUSE, CONSTRUCT DE MANAGEMENT OF CHANNELS STALL BE ANALTZED BY IT USE OF A TEN-YEAR FREQUENCY STORM TO VERIFY THAT STORMWATER WILL NOT OVERTOP ITS BANKS AND BY THE USE OF A TWO-YEAR STORM TO DEMONSTRATE THAT STORMWATER WILL NOT CAUSE EROSION OF CHANNEL BED OR BANKS; AND b.4. PIPES AND STORM SEWER SYSTEMS SHALL BE ANALYZED BY THE USE OF A TEN-YEAR FREQUENCY STORM TO VERIFY THAT STORMWATER WILL BE CONTAINED WITHIN THE
 - c. IF EXISTING NATURAL RECEIVING CHANNELS OR PREVIOUSLY CONSTRUCTED MAN-MADE CHANNELS OR PIPES ARE NOT ADEQUATE. THE APPLICANT SHALL
 - c.1. IMPROVE THE CHANNELS TO A CONDITION WHERE A TEN-YEAR FREQUENCY STORM WILL NOT OVERTOP THE BANKS AND A TWO-YEAR FREQUENCY STORM WILL NOT CAUSE EROSION TO THE CHANNEL, THE BED, OR BANKS; OR c.2. IMPROVE THE PIPE OR PIPE SYSTEM TO A CONDITION WHERE THE TEN-YEAR
 - FREQUENCY STORM IS CONTAINED WITHIN THE APPURTENANCES
 - c.3. DEVELOP A SITE DESIGN THAT WILL NOT CAUSE THE PRE-DEVELOPMENT PEAK RUNOFF RATE FROM A TWO-YEAR STORM TO INCREASE WHEN RUNOFF OUTFALLS INTO A NATURAL CHANNEL OR WILL NOT CAUSE THE PRE-DEVELOPMENT PEAK RUNOFF RATE FROM A TEN-YEAR STORM TO INCREASE WHEN RUNOFF OUTFALLS INTO A MAN-MADE CHANNEL: OR
 - c.4. PROVIDE A COMBINATION OF CHANNEL IMPROVEMENT, STORMWATER DETENTION/RETENTION OR OTHER MEASURES WHICH IS SATISFACTORY TO THE VESCP AUTHORITY TO PREVENT DOWNSTREAM EROSION
 - d. THE APPLICANT SHALL PROVIDE EVIDENCE OF PERMISSION TO MAKE THE IMPROVEMENTS. e. ALL HYDROLOGIC ANALYSES SHALL BE BASED ON THE EXISTING WATERSHED CHARACTERISTICS AND THE ULTIMATE DEVELOPMENT CONDITION OF THE SUBJECT
 - PROJECT. f. IF THE APPLICANT CHOOSES AN OPTION THAT INCLUDES STORMWATER DETENTION, HE SHALL OBTAIN APPROVAL FROM THE VESCP OF A PLAN FOR MAINTENANCE OF THE DETENTION FACILITIES. THE PLAN SHALL SET FORTH THE MAINTENANCE REQUIREMENTS
 - OF THE FACILITY AND THE PERSON RESPONSIBLE FOR PERFORMING THE MAINTENANCE. g. OUTFALL FROM A DETENTION FACILITY SHALL BE DISCHARGED TO A RECEIVING CHANNEL, AND ENERGY DISSIPATERS SHALL BE PLACED AT THE OUTFALL OF ALL DETENTION FACILITIES AS NECESSARY TO PROVIDE A STABILIZED TRANSITION FROM THE FACILITY TO
 - THE RECEIVING CHANNEL. . ALL ON-SITE CHANNELS MUST BE VERIFIED TO BE ADEQUATE
 - INCREASED VOLUMES OF SHEET FLOWS THAT MAY CAUSE EROSION OR SEDIMENTATION ON ADJACENT PROPERTY SHALL BE DIVERTED TO A STABLE OUTLET, ADEQUATE CHANNEL, PIPE OR PIPE SYSTEM, OR TO A DETENTION FACILITY.
- IN APPLYING THESE STORMWATER MANAGEMENT CRITERIA INDIVIDUAL LOTS IN A IN APPLYING THESE STORMWALER MANAGEMENT CRITERIA, INDIVIDUAL LOTS IN A
 RESIDENTIAL SUBDIVISION DEVELOPMENT SHALL NOT BE CONSIDERED TO BE SEPARATE
 DEVELOPMENT PROJECTS. INSTEAD, THE RESIDENTIAL SUBDIVISION DEVELOPMENT, AS A
 WHOLE, SHALL BE CONSIDERED TO BE A SINGLE DEVELOPMENT PROJECT. HYDROLOGIC PARAMETERS THAT REFLECT THE ULTIMATE SUBDIVISION DEVELOPMENT SHALL BE USED IN ALL ENGINEERING CALCULATIONS.
- k. ALL MEASURES USED TO PROTECT PROPERTIES AND WATERWAYS SHALL BE EMPLOYED IN A MANNER WHICH MINIMIZES IMPACTS ON THE PHYSICAL, CHEMICAL AND BIOLOGICAL INTEGRITY OF RIVERS, STREAMS AND OTHER WATERS OF THE STATE.
- ANY PLAN APPROVED PRIOR TO JULY 1, 2014, THAT PROVIDES FOR STORMWATER MANAGEMENT THAT ADDRESSES ANY FLOW RATE CAPACITY AND VELOCITY REQUIREMENTS FOR NATURAL OR MAN-MADE CHANNELS SHALL SATISFY THE FLOW RATE CAPACITY AND VELOCITY REQUIREMENTS FOR NATURAL OR MAN-MADE CHANNELS IF THE PRACTICES ARE DESIGNED TO (I) DETAIN THE WATER QUALITY VOLUME AND TO RELEASE IT OVER 48 HOURS: (II) DETAIN AND RELEASE OVER A 24-HOUR PERIOD THE EXPECTED RAINFALL RESULTING FROM THE ONE YEAR, 24-HOUR STORM: AND (III) REDUCE THE RAINFALL RESULTING FROM THE ONE YEAR, 24-HOUR STORM; AND (III) REDUCE THE
 ALLOWABLE PEAK FLOW RATE RESULTING FROM THE 1.5, 2, AND 10-YEAR, 24-HOUR
 STORMS TO A LEVEL THAT IS LESS THAN OR EQUAL TO THE PEAK FLOW RATE FROM THE
 SITE ASSUMING IT WAS IN A GOOD FORESTED CONDITION, ACHIEVED THROUGH
 MULTIPLICATION OF THE FORESTED PEAK FLOW RATE BY A REDUCTION FACTOR THAT IS
 COUAL TO THE RUNOFF VOLUME FROM THE SITE WHEN IT WAS IN A GOOD FORESTED
 CONDITION DIVIDED BY THE RUNOFF VOLUME FROM THE SITE IN ITS PROPOSED
 CONDITION AND SHALL BE EVEMPT FROM ANY FLOW RATE CAPACITY AND VEH OF THE CONDITION, AND SHALL BE EXEMPT FROM ANY FLOW RATE CAPACITY AND VELOCITY REQUIREMENTS FOR NATURAL OR MAN-MADE CHANNELS AS DEFINED IN AN REGULATIONS PROMULGATED PURSUANT TO § 62.1-44.15:54 OR 62.1-44.15:65 OF THE ACT.
- m. FOR PLANS APPROVED ON AND AFTER JULY 1, 2014, THE FLOW RATE CAPACITY AND VELOCITY REQUIREMENTS OF \S 62.1-44.15:52 A OF THE ACT AND THIS SUBSECTION SHALL BE SATISFIED BY COMPLIANCE WITH WATER QUANTITY REQUIREMENTS IN THE STORMWATER MANAGEMENT ACT (§ <u>62.1-44.15:24</u> ET SEQ. OF THE CODE OF VIRGINIA) AND ATTENDANT REGULATIONS, UNLESS SUCH LAND-DISTURBING ACTIVITIES ARE IN ACCORDANCE WITH PROVISIONS FOR THE TIME LIMITS ON APPLICABILITY OF APPROVED DESIGN CRITERIA IN 97AC25-870-47 OR GRADPATHERING IN 97AC25-870-48 OF THE VIRGINIA STORMWATER MANAGEMENT PROGRAM (VSMP) REGULATION, IN WHICH CASE THE FLOW RATE CAPACITY AND VELOCITY REQUIREMENTS OF § 82.1-44.15:34 OF THE ACT SHALL APPLY, OR ARE EXEMPT PURSUANT TO § 62.1-44.15:34 C7 OF THE ACT.
- DECEMBER OF THE WATER QUANTITY MINIMUM STANDARDS SET OUT IN 9VAC25-870-66 OF THE VIRGINIA STORMWATER MANAGEMENT PROGRAM (VSMP)
 REGULATIONS SHALL BE DEEMED TO SATISFY THE REQUIREMENTS OF SUBDIVISION 19 OF



SYMBOLS

(S) SIGNS ELECTRIC BOX

STORM MANHOLE

TREE

TELEPHONE MANHOLE

ELECTRIC METER (F) ELECTRIC MANHOLE FIRE HYDRANT UTILITY BOX GAS METER

BOLLARD

BUSH

CAP

GAS VALVE

○ LIGHT POLE

POWER POLE

PROPERTY PIN

■ UTILITY PEDESTAL Ø UTILITY POLE M WATER METER

WATER VALVE YARD LIGHT

LINE TYPES _____

EXISTING GROUND EAST SIDE EXISTING GROUND CENTERLINE EXISTING GROUND WEST SIDE GAS PIPE PROPERTY LINE SEWER EASEMENT OVERHEAD ELECTRIC

OVERHEAD CABLE TV _____ UCATY_____ UNDERGROUND ELECTRIC UNDERGROUND TELEPHONE

DATE: MARCH 12, 2018 DRAWN BY: K. ATKINSON

RICHMOND

CHECKED BY: A. WEHUNT REVISIONS REVISED PER WR'S 07/20/17



AS BUILT

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JAMES RIVER PARK SYSTEM WAREHOUSE SITE

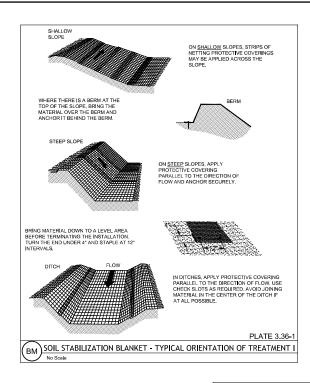
SEDIMENT CONTROL

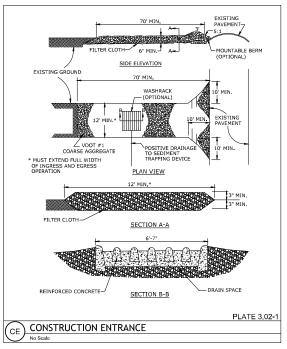
DEPARTMENT OF PARKS AND RECREATION

36157.001

IMPROVEMENTS EROSION AND

SHEET NO.





10' MAXIMUM CENTER TO CENTER

SPACING

33" MIN

36" MIN

2 3" DIAMETER

GALVANIZED ALUMINIUM POSTS

33" MIN

FLOW

FILTER CLOTH

CHAIN LINK FENCE

FILTER CLOTH

NOTES:

1. FASTEN CHAIN LINK FENCE SECURELY TO FENCE POSTS WITH

WRE TIES OR STAPLES.
2. FASTEN FILTER CLOTH SECURELY TO CHAIN LINK FENCE WITH

GROUND.

4. OVERLAP AND FOLD ADJOINING SECTIONS OF FILTER CLOTH.

SUPER SILT FENCE DETAIL NO SCALE

TIES SPACED A MINIMUM OF 24" ALONG POST.

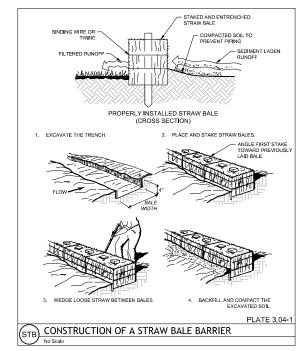
3. EMBED FILTER CLOTH A MINIMUM OF 6" INTO EXISTING

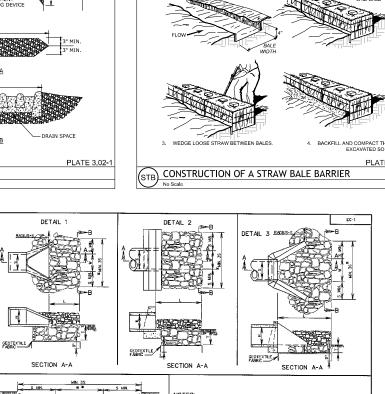
EMBESSED A MIN. OF 6" INTO EX, GROUND

GALVANIZED

ALUMINIUM POSTS

EX. GROUND -





SECTION B-B

(FOR DESIGN STORM) MINIMUM "T' UNCHES)

CULVERT OUTLET PROTECTION

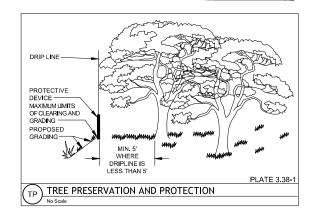
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a tps

TYPE OF OUTLET PROTECTION MATERIAL MAXIMUM OUTLET VELOCITY

CLASS I CLASS I DRY RIPRAP

OP



I. FOR MULTIPLE LINE INSTALLATIONS, DIMENSION S IS TO GOVERN THE PROTECTION CUTSIDE THE CHANNEL WIDTH (W).

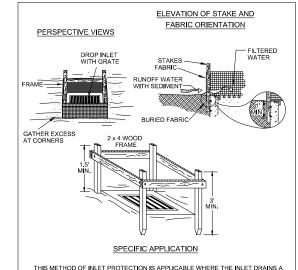
 ON ANY INSTALLATION REQUIRING CULVERT OUTLET PROTECTION WHERE NO ENDWALL OR ENDSECTION IS SPECIFIED ON THE PLANS, CONSTRUCTION IS TO BE IN ACCORDANCE WITH DETAIL 2 SHOWN ABOVE. CENTEYTRE FARRIC TO BE INSTALLED UNDER CLASS ALT, AND I MATERIALS IN ACCORDANCE WITH THE SPECIFICATIONS.

S - DIAMETER OF CIRCULAR CULVERT OR SPAN FOR BOX, ELLIPTICAL OR ARCH CULVER H - DIAMETER OF CIRCULAR CULVERT OR RISE/HEIGHT FOR BOX, ELLIPTICAL, OR ARCH CILLVERT.

USE TYPICAL SECTION SHOWN ON PLANS FOR SIDE SLOPE, BOTTOM WIDTH AND DEPTH OF CHANNEL OR MATCH EXISTING DITCH DR NATURAL GROUND.

OUTLET PROTECTION MINUMUM LENGTH (L)

REVISION DATE SHEET 1 OF 1



THIS METHOD OF INLET PROTECTION IS APPLICABLE WHERE THE INLET DRAINS A RELATIVELY FLAT AREA (SLOPES NO GREATER THAN 5%) WHERE INLET SHEET OR OVERLAND FLOWS (NOT EXCEEDING 1 CFS) ARE TYPICAL. THE METHOD SHALL NOT APPLY TO INLETS RECEIVING CONCENTRATED FLOWS, SUCH AS IN STREET

SILT FENCE DROP INLET PROTECTION

TABLE 3.31-B (REVISED June 2003) TEMPORARY SEEDING SPECIFICATIONS

APPLICATION DATES	SPECIES	APPLICATION RATES
	50/50 Mix of Annual Ryegrass (Iolium multi- florum) & Cereal (Winter) Rye (Secale cereale)	50 -100 (lbs/acre)
Feb. 16 - Apr. 30	Annual Ryegrass (Iolium multi-florum)	60 - 100 (lbs/acre)
May 1 - Aug. 31	German Millet	50 (lbs/acre)

FERTILIZER & LIME

- Apply 10-10-10 fertilizer at a rate of 450 lbs / acre (or 10lbs. / 1,000 sq. ft.)

- NO 1c:

 1 A soil test is necessary to determine the actual amount of lime required to adjust the soil pH of site,
 2 Incorporate the lime and fertilizer into the top 4 6 inches of the soil by disking or by other means.
 3 When applying Slowly Available Nitrogen, use rates available in Ergosion & Sediment Control Technical Bulletin
 #4, 2003 Nutrient Management for Development Sites at http://www.dcr.state.va.us/sw/e&s.htm#pubs

TABLE 3.32-D (REVISED JUNE 2003) PERMANENT SEEDING SPECIFICATIONS FOR PIEDMONT AREA

MINIMUM CARE LAWN (COMMERCIAL OR RESIDENTIAL) HIGH-MAINTENANCE LAWN GENERAL SLOPE (3:1 OR LESS) TALL FESCUE' TOTAL TALL FESCUE' RED TOP GRASS OR CREEPING RED FESCUE SEASONAL NURSE CROP' TALL FESCUE' RED TOP GRASS OR CREEPING RED FESCUE SEASONAL NURSE CROP' TALL FESCUE' RED TOP GRASS OR CREEPING RED FESCUE SEASONAL NURSE CROP' TALL FESCUE' TOTAL TOT		SEED¹	
MINIMUM CARE LAWN PERENNIAL RY EGRASS COMMERCIAL OR RESIDENTIAL KENTUCKY BLUEGRASS TOTAL	LAND USE	SPECIES	APPLICATION PER ACRE
GENERAL SLOPE (3:1 OR LESS) TALL FESCUE¹ RED TOP GRASS OR CREEPING RED FESCUE SEASONAL NURSE CROP² T	JM CARE LAWN PE	ERENNIAL RYEGRASS	95-100% 0-5% 0-5% TOTAL: 175-200 LBS.
GENERAL SLOPE (3:1 OR LESS) RED TOP GRASS OR CREEPING RED FESCUE SEASONAL NURSE CROP ³ T	MAINTENANCE LAWN TA	ALL FESCUE ¹	TOTAL: 200-250 LBS.
LOW-MAINTENANCE SLOPE TALL FESCUE1	RAL SLOPE (3:1 OR LESS) RI	ED TOP GRASS OR CREEPING RED FESCUE	128 LBS. 2 LBS. 20LBS. TOTAL: 150 LBS.
(STEEPER THAN 3:1) RED TOP GRASS OR CREEPING RED FESCUE SEASONAL NURSE CROP2 CROWNVETCH	PER THAN 3:1) RI	ED TOP GRASS OR CREEPING RED FESCUE EASONAL NURSE CROP ²	108 LBS 2 LBS 20 LBS 20 LBS TOTAL: 150 LBS

1. WHEN SELECTING VARIETIES OF TURFORASS, USE THE VIRGINIA CROP IMPROVEMENT ASSOCIATION (VCIA) RECOMMENDED TURFORASS VARIETY UST, OULLITY SEED WILL BEAR A LABEL INDICATING THAT THEY ARE APPROVIED BY VCIA, A CURRENT TURFORASS VARIETY LIST IS AVAILABLE AT THE LOCAL COUNTY EXTENSION OFFICE OR THROUGH VCIA AT 804-746-4884 OR AT HTTP://SUDAN.CSES.VT.EDUHTMLTURF/TURF/FUBLICATIONS/PUBLICATIONS2.HTML

- USE SEASONAL NURSE CROP IN ACCORDANCE WITH SEEDING DATES AS STATED BELOW:

MAINTENANCE MIXTURE DURING WARMER SEEDING PERIODS, INCREASE TO 30-40

FERTILIZER & LIME

- APPLY 10-20-10 FERTILIZER AT A RATE OF 500 LBS. / ACRE (OR 12 LBS. / 1,000 SQ. FT.)
 APPLY PULVERIZED AGRICULTURAL LIMESTONE AT A RATE OF 2 TONS/ACRE (90 LBS. / 1,000 SQ. FT.)
- NOTE:

 A SOIL TEST IS NECESSARY TO DETERMINE THE ACTUAL AMOUNT OF LIME REQUIRED TO ADJUST THE SOIL PH OF SITE.

 INCORPORATE THE LIME AND FERTILIZER INTO THE TOP 4 6 INCHES OF THE SOIL BY DISKING OR BY OTHER MEANS.

 WHEN APPLYING SLOWLY AVAILABLE NITROGEN, USE RATES AVAILABLE IN EROSION & SEDIMENT CONTROL TECHNICAL BULLETIN M4, 2003 NUTRIENT MANAGEMENT FOR DEVELOPMENT SITES AT HTTP://WWW.DCR.STATE.VAJUS/SW/E&S.HTM#PUBS

03-12-2018

SYMBOLS

BOLLARD BUSH CAP SIGNS ELECTRIC BOX STORM MANHOLE ELECTRIC METER TELEPHONE MANHOLE (F) ELECTRIC MANHOLE TREE FIRE HYDRANT UTILITY BOX GAS METER ■ UTILITY PEDESTAL Ø UTILITY POLE WATER METER ○ LIGHT POLE WATER VALVE POWER POLE YARD LIGHT

_____ EXISTING GROUND EAST SIDE EXISTING GROUND CENTERLINE EXISTING GROUND WEST SIDE PROPERTY LINE SEWER EASEMENT OVERHEAD ELECTRIC OVERHEAD CABLE TV UNDERGROUND ELECTRIC UNDERGROUND TELEPHONE

DATE: MARCH 12, 2018 DRAWN BY: K. ATKINSON



REVISIONS AS BUILT



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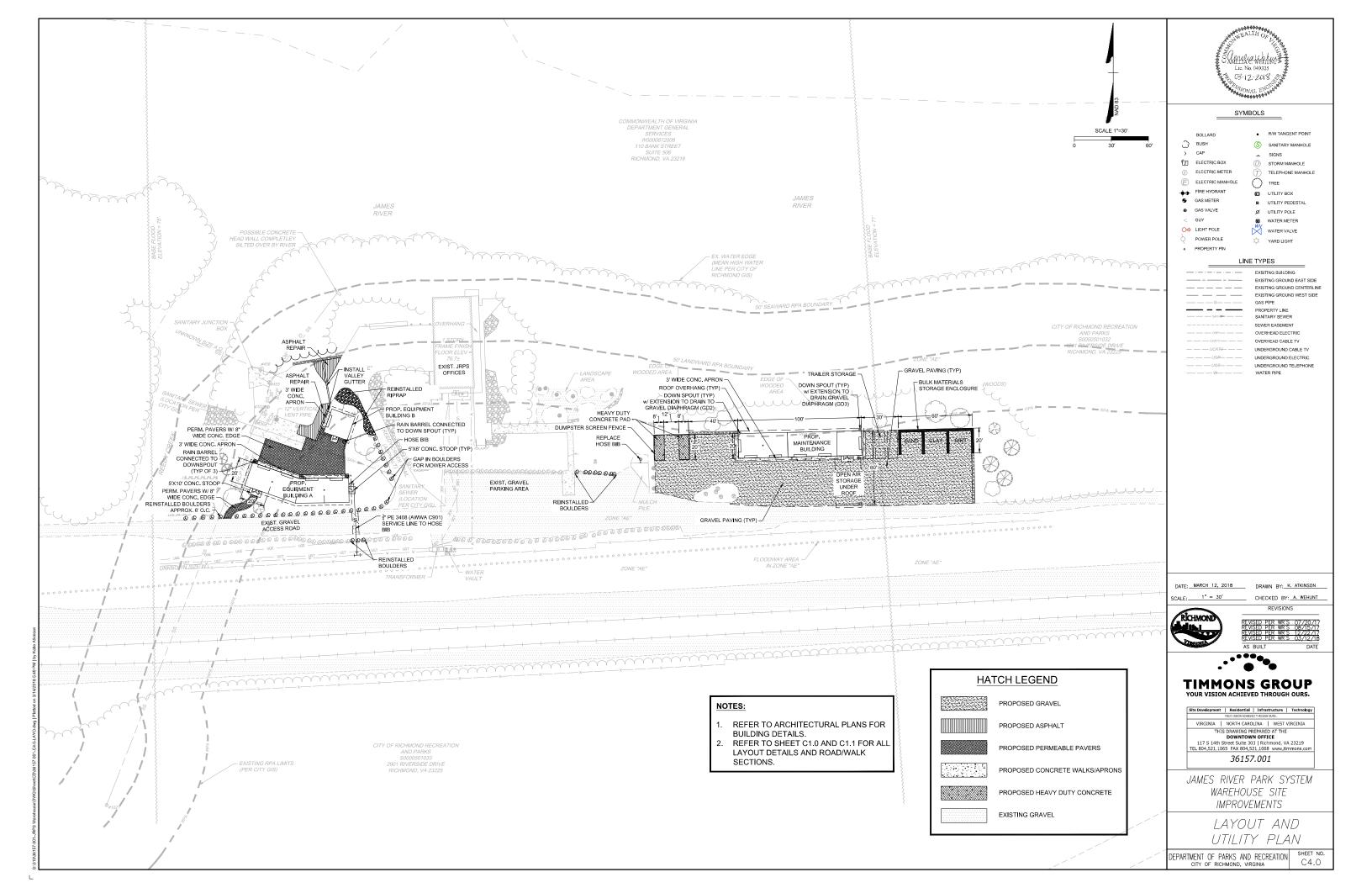
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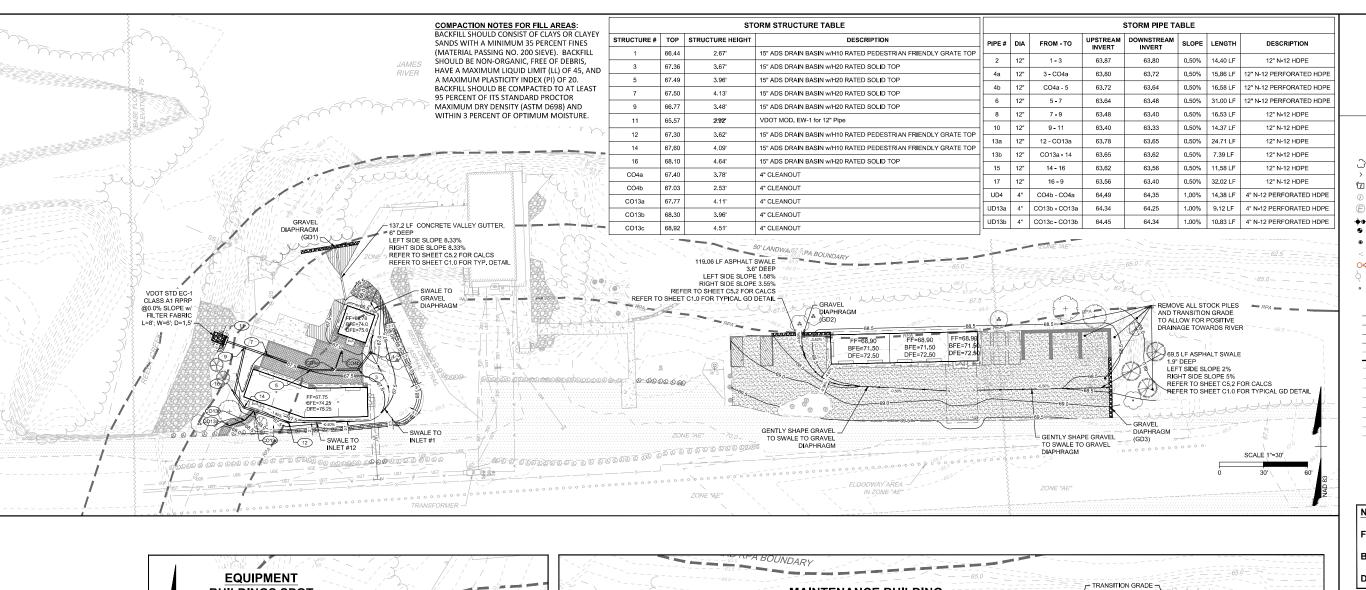
JAMES RIVER PARK SYSTEM WAREHOUSE SITE **IMPROVEMENTS** EROSION AND SEDIMENT CONTROL NOTES & DETAILS

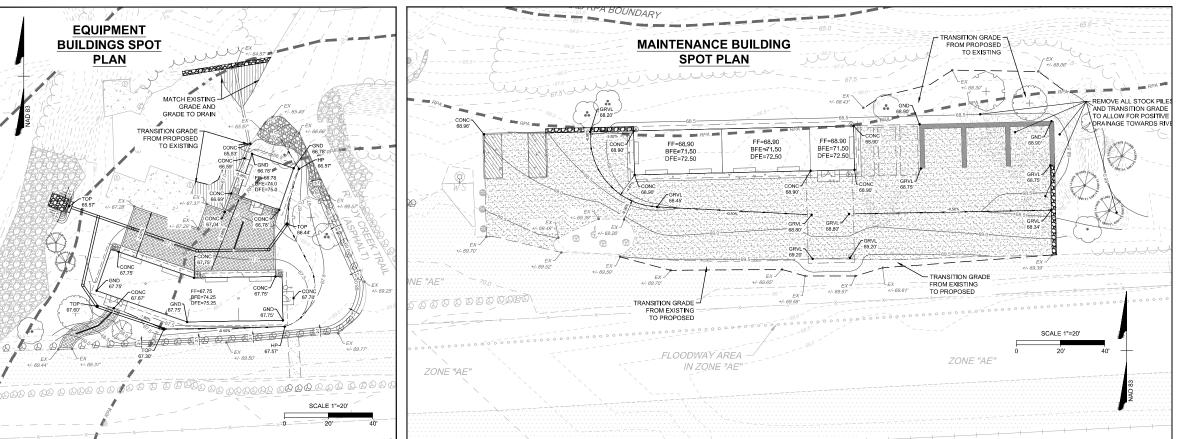
DEPARTMENT OF PARKS AND RECREATION

SHEET NO.













BOLLARD

BUSH

CAP

ELECTRIC BOX

ELECTRIC MANHOLE

TREE

FIRE HYDRANT UTILITY BOX
GAS METER UTILITY PEDESTAL
GAS VALVE Ø UTILITY POLE

WATER METER

WATER VALVE

WATER VALVE

VARD LIGHT

POWER POLE
PROPERTY PIN

EXISTING GROUND CENTERLI

EXISTING GROUND CENTERLI

EXISTING GROUND WEST SIDE

EXISTING GROUND CENTERLIN
EXISTING GROUND WEST SIDE
GAS PIPE
PROPERTY LINE
SANITARY SEWER
SEWER EASEMENT
OVERHEAD ELECTRIC

OVERHEAD CABLE TV
UNDERGROUND CABLE TV
UNDERGROUND ELECTRIC
UNDERGROUND TELEPHONE
WATER PIPE

NOTES:

FF = FINISHED FLOOR BFE = BASE FLOOD ELEVATION

DFE = DESIGN FLOOD ELEVATION

DATE: MARCH 12, 2018

AS SHOWN CHECKED BY: A. WEHUN



REVISED PER WR'S 07/20/17
REVISED PER WR'S 08/15/17
REVISED PER WR'S 12/72/17
REVISED PER WR'S 12/72/17
REVISED PER WR'S 03/12/18
AS BUILT DATE

TIMMONS GROUP

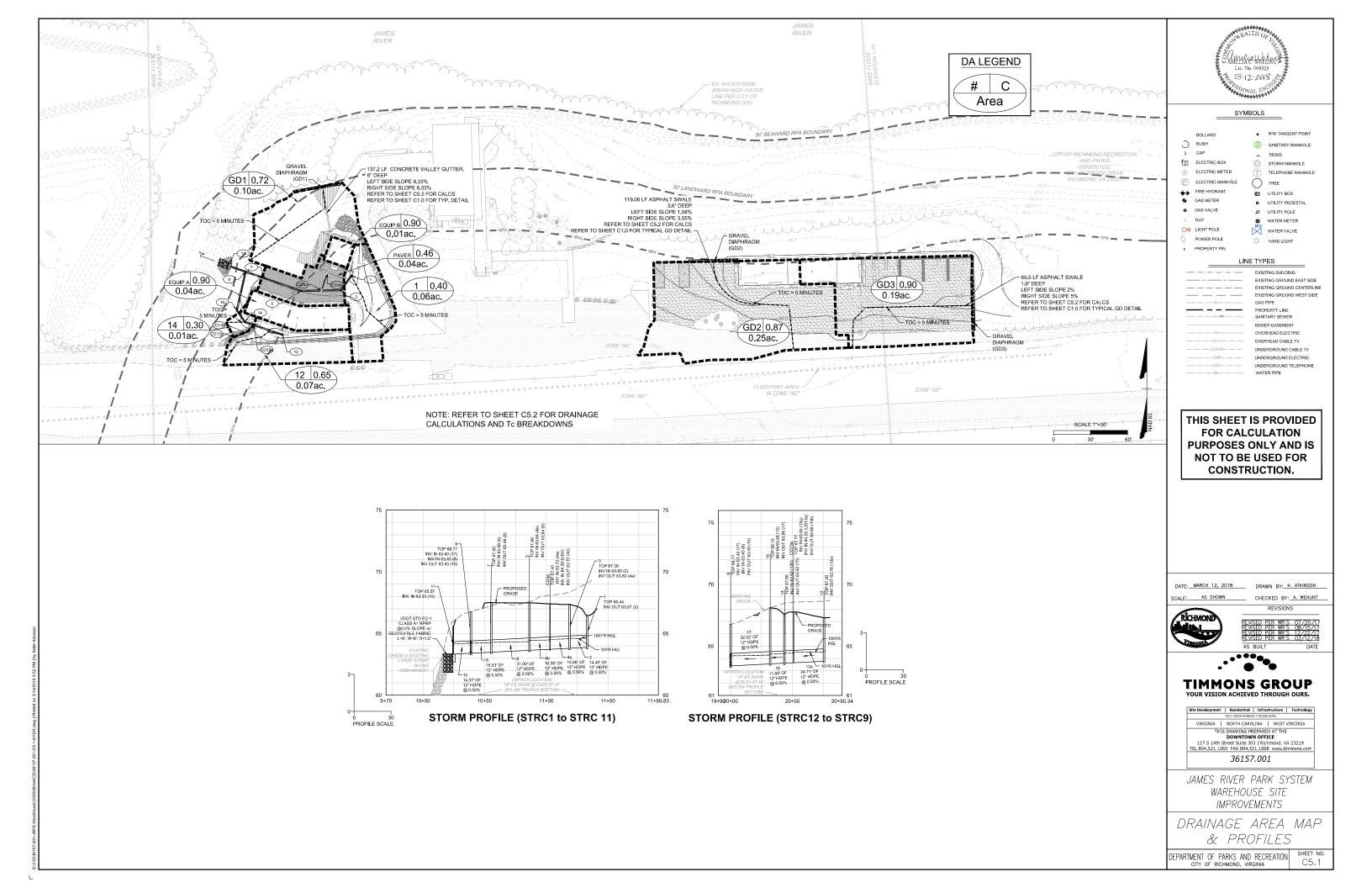
| Site Development | Residential | Infrastructure | Technology | Techn

JAMES RIVER PARK SYSTEM WAREHOUSE SITE IMPROVEMENTS

GRADING AND DRAINAGE PLAN

DEPARTMENT OF PARKS AND RECREATION CITY OF RICHMOND, VIRGINIA

SHEET NO. C5.0



GRAVEL DIAPHRAGM CAPACITY CALCULATIONS

	Worksheet for GD1		Worksheet for GD2		Worksheet for GD3
Volect Descriptor	Considera della regiona della della regiona della considera della regiona della regiona della regiona della re	Project Descriptions		Project Description	na da la companya da mana da m
iction Method	Manning Formula	Priction Method	Manning Permuta	Friction Method	Mensico Formula
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AT Skin Skins	833 %	Left Side Siops	1.09 %	Left Sixta Sistem	200 %
gM Sirie Slope	830 %	Rotal Side Stops	3.65 %	Right Side Store	5.00 %
ischarge	0.53 m/s	Discharge	1.00 EW	Discharge	1.23 m/s
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armal Depth	9.63 e	Normal Depth	0.16 ft	Normal Conth	0.15 e
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tical Slape	0.09958 Neg	Critical Stope Valouity	0.00007 AN 1.45 NA	Critical Slope	0.03594 6/8
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iniosy maso ipedfic Energy	128 M	Specific Energy	0.19 #	Specific Emergy	9.00 a 9.19 a
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our Type	Superation	Flow Type	Subcritical	Flow Type	Subcritical
rvii Indul Data	TOTAL CONTROL OF THE STATE OF T	GVF linput Data	建设路接受的电影。1995年的特别的	GIVE Input Date	Commence Administration of the Commence of the
owretwern Depth	0.00 ±	Downstream Depits	0.00 1	Downstream Death	0.00 ft
ingth	0.00 *	Longth	9.00 ft	Length	0.60 n
ember Of Sleps	0	Number Of Steps	0	Number Of Steps	0
VE OUTSTOOM	ENTLIE GOVERNMEN UND GESTIMMEN -	GVF Output Date		GVP Output Data	では複雑雑組を表しませんできましたものです。 女
potream Dapth	0.00 g	Upetream Depth	0:00 n	Upskeem Depth	0.00 g
rotie Decorption		Profile Description		Profile Description	
roffia Hoselicas	0.00 R	Profile Headless	0.00 A	Profile Headloss	0.00 a
ovnetnem Velocity	Infinity: Ale	Cownetreurs Velocity	infinity rija	Downstowers Velocity	Indinity 19th
stream Vallocity	Infinity (Ida	Upstream Velocity	infinity this	Upstream Velocity	Infinity 409
ormal Depth	0.13 m	Normal Depth Critical Depth	0.15 m	Namai Depth	0.15 1
riscal Depth	0.10 n	Charrie Stope	0.00000 644	Critical Depth	0.18 m
harrel Slope ritical Gloca	0.16600 NR 0.06600 Am	Grilliani Skope	9,000F 84t	Charmel Slope	0.00630 mm
acts probe	C.COCCC 25H	Distances (Mary 1)	170	Critical Slope	0.00004 6/8

	STRUCTURE	1				STRUCTURE	12				STRUCTURE	14				STRUCTURE	Equip A				
		C-factor /	Area (C*A			C-factor A	rea	C*A		ll	C-factor A	rea	C*A			C-factor .	Area	C*A		
	Lawn	0.30	0.05	0.02		Lawn	0.30	0.03	0.01		Lawn	0.30	0.01	0.00		Impervious	0.90	0.04	0.04		
	Impervious	0.90	0.01	0.01		Impervious	0.90	0.04	0.03												
						II .															
		,	krea T	otal C-facto	r		Д	rea	Total C-facto	r		Д	rea	Total C-factor				Area	Total C-fac	tor	
			0.06	0.40				0.07	0.65		l		0.01	0.30				0.04	0.90		
	Tc:					Tc:					Tc:					Tc:					
		Overland:					Overland:				l	Overland:					Overland:				
			enath =	24 ft				ength =	21 f		l		ength =	15 ft				Lenath =	0	ft	
			Slope =	1.2 %	,			lope =	0.5 %		l		lope =	5.0 %				Slope =		%	
			-factor =	0.90				-factor =	0.90		l		-factor =	0.30				c-factor =	0.90		
		7		tc =	2.2 minutes		-		tc =	2.5 minutes	ll			tc =	4.1 minutes			- 144141	tc =		0.0 minutes
					L.L IIIIIGO					2.0 minutos	1				4.1 minutou	1					o.u minatou
		Concentrate	d:				Concentrate	d:			l	Concentrate	d:				Concentrati	orl:			
			.ength =	31 ff				ength =	39 1		l		ength =	O ff				Length =	0	ff	
			H=	1 6			-	H=	2 f		ll	_	H=	1 8				H=	1	A	
					0.5 minutes			- 11	- '	0.5 minutes	l		- 11		0.0 minutes			- 11-		II.	0.0 minutes
				Tc =	5.0 minutes	1			Tc = -	5.0 minutes	1			Tc = -	5.0 minutes	1			Tc =		5.0 minutes
			5.38			l		5.38					5.38				1 -	5.38			
		l ₂ =		Q ₂ =	0.14 cfs		l ₂ =		Q ₂ =	0.23 cfs		I ₂ =		Q ₂ =	0.01 cfs		12 =		Q ₂ =		.19 cfs
		I ₁₀ =	7.05	$Q_{10} =$	0.18 cfs		I ₁₀ =	7.05	$Q_{10} =$	0.30 cfs		I ₁₀ =	7.05	Q ₁₀ =	0.02 cfs	ll l	I ₁₀ =	7.05	Q ₁₀ =		.25 cfs
		I ₂₅ =	7.80	$Q_{25} =$	0.20 cfs		I ₂₅ =	7.80	$Q_{25} =$	0.33 cfs		I ₂₅ =	7.80	$Q_{25} =$	0.02 cfs		I ₂₅ =	7.80	Q ₂₅ =	0	.28 cfs
		I ₅₀ =	10.09	$Q_{50} =$	0.25 cfs		I ₅₀ =	10.09	$Q_{50} =$	0.43 cfs		I ₅₀ =	10.09	$Q_{50} =$	0.02 cfs	[[I ₅₀ =	10.09	$Q_{50} =$	0	.36 cfs
		I ₁₀₀ =	11.38	$Q_{100} =$	0.29 cfs		I ₁₀₀ =	11.38	Q ₁₀₀ =	0.49 cfs		I ₁₀₀ =	11.38	Q ₁₀₀ =	0.03 cfs	[[I ₁₀₀ =	11.38	Q ₁₀₀ =	0	.41 cfs
٦	STRUCTURE	Dougre				STOLICTUDE	GD1				ETDUCTUDE	GD2				STRUCTURE	GD3				

GD1

AREA OF CHANNEL = 3 sqft
AREA LESS 40% STONE VOID = 1.8 sqft
AREA LESS 40% STONE VOID = 1.8 sqft
AREA OF NORMAL FLOW THROUGH CHANNEL = 0.22 sqft
PERCENT NORMAL FLOW THROUGH CHANNEL = 12.2% CAPACITY
GD2
AREA OF CHANNEL = 4.13 sqft
AREA OF NORMAL FLOW THROUGH CHANNEL = 1.07 sqft
PERCENT NORMAL FLOW THROUGH CHANNEL = 25.9% CAPACITY
GD3
AREA OF CHANNEL = 0.90 sqft
AREA OF CHANNEL = 0.90 sqft
AREA OF NORMAL FLOW THROUGH CHANNEL = 0.84 sqft
PERCENT NORMAL FLOW THROUGH CHANNEL = 0.84 sqft
PERCENT NORMAL FLOW THROUGH CHANNEL = 0.84 sqft

-g-13-lj	150 - 10:00 Q50 - 0:25 CIS	150 - 10:00 Q50 - 0:40 CIS	150 - 10.00 Q50 - 0.02 CIS	150 - 10.00 Q50 - 0.50 CIS
	I ₁₀₀ = 11.38 Q ₁₀₀ = 0.29 cfs	I ₁₀₀ = 11.38 Q ₁₀₀ = 0.49 cfs	I ₁₀₀ = 11.38 Q ₁₀₀ = 0.03 cfs	I ₁₀₀ = 11.38 Q ₁₀₀ = 0.41 cfs
STRUCTURE Equip B ST	STRUCTURE Pavers	STRUCTURE GD1	STRUCTURE GD2	STRUCTURE GD3
C-factor Area C*A	C-factor Area C*A	C-factor Area C*A	C-factor Area C*A	C-factor Area C*A
Impervious 0.90 0.01 0.01 Lav	_awn 0.40 0.04 0.02	Lawn 0.30 0.03 0.01		Lawn 0.30 0.00 0.00
Imp	mpervious 0.90 0.01 0.00	Impervious 0.90 0.07 0.06	Impervious 0.90 0.24 0.22	Impervious 0.90 0.19 0.17
Area Total C-factor 0.01 0.90 Tc: Overland: Length = 0 ft Slope = 1.0 % c-factor = 0.90 to = 0.00 minutes	Area Total C-factor 0.04 0.45 Coverland: Length = 0 ft Slope = 1.0 % c-factor = 0.45 tc = 0.0 minutes	Area Total C-factor 0.10 0.72 To: Overland: Length = 75 ft Stope = 2.8 % c-factor = 0.90 tc = 3.0 minutes	Area Total C-factor 0.25 0.87 Tc: Overland: Length = 36 ft Stope = 2.7 % c-factor = 0.90 tc = 2.2 minutes	Area Total C-factor 0.19 0.90 To: Overland: Length = 37 ft Slope = 1.0 % c-factor = 0.90 to = 2.7 minutes
10 - <u>0.0 minutes</u>	10 - 0.0 minutes	tc - 3.0 minutes	- 2.2 minutes	10 - 2.7 minutes
Concentrated:	Concentrated:	Concentrated:	Concentrated:	Concentrated:
Length = 0 ft	Length = 0 ft	Length = 16 ft	Length = 82 ft	Length = 86 ft
H= 1 ft	H = 1 ft	H= 1 ft	H = 1 ft	H= 1 ft
0.0 minutes	0.0 minutes	0.2 minutes	1.8 minutes	1.6 minutes
Tc = 5.0 minutes	Tc = 5.0 minutes	Tc = 5.0 minutes	Tc = 5.0 minutes	Tc = 5.0 minutes
I ₂ = 5.38 Q ₂ = 0.05 cfs	$I_2 = 5.38$ $Q_2 = 0.11$ cfs	l ₂ = 5.38 Q ₂ = 0.40 cfs	l ₂ = 5.38 Q ₂ = 1.18 cfs	l ₂ = 5.38 Q ₂ = 0.94 cfs
I ₁₀ = 7.05 Q ₁₀ = 0.06 cfs	I ₁₀ = 7.05 Q ₁₀ = 0.14 cfs	I ₁₀ = 7.05 Q ₁₀ = 0.53 cfs	I ₁₀ = 7.05 Q ₁₀ = 1.55 cfs	I ₁₀ = 7.05 Q ₁₀ = 1.23 cfs
I ₂₅ = 7.80 Q ₂₅ = 0.07 cfs	I ₂₅ = 7.80 Q ₂₅ = 0.16 cfs	I ₂₅ = 7.80 Q ₂₅ = 0.58 cfs	I ₂₅ = 7.80 Q ₂₅ = 1.71 cfs	I ₂₅ = 7.80 Q ₂₅ = 1.36 cfs
I ₅₀ = 10.09 Q ₅₀ = 0.09 cfs	$I_{50} = 10.09$ $Q_{50} = 0.20$ cfs	I ₅₀ = 10.09 Q ₅₀ = 0.75 cfs	I ₅₀ = 10.09 Q ₅₀ = 2.21 cfs	I ₅₀ = 10.09 Q ₅₀ = 1.76 cfs
I ₁₀₀ = 11.38 Q ₁₀₀ = 0.10 cfs	I ₁₀₀ = 11.38 Q ₁₀₀ = 0.23 cfs	I ₁₀₀ = 11.38 Q ₁₀₀ = 0.85 cfs	I ₁₀₀ = 11.38 Q ₁₀₀ = 2.49 cfs	I ₁₀₀ = 11.38 Q ₁₀₀ = 1.98 cfs

10 YR STORM SEWER CALCULATIONS

			Storm Im		ents					PROJE		James Rive	-	em				Design	ed by:	L. Coffr	nan		
	SEWER D		COMPUTA	ATIONS						LOCATIO	·N:	City of Rich	mond					Checke	ed by:	L. Coffr	nan		
O TORWIT	TEGOE NOT	IVIK	1																UNITS	ENGI	_ISH		
					DRAIN.	RUNOFF		CA		TOTAL	RAIN	RUNOFF	INVERT EL	EVATIONS	LENGTH	SLOPE.	SIZE	SHAPE	Capacity	Friction	VEU	FLOW	
PIPE	FROM P	OINT	ТО РО	INT	AREA	COEFF.	INCRE-	ACCUM-	ADOTL	INLET	FALL	Q	UPPER	LOWER	of Pipe		(Dia. Or			Slope	Vn	тім є	l
NO	REFERENCE	STA.	REFERENCE	STA.	"A"	*C*	MENT	ULATED	CA	TIME			END	END			Span/Rise)						REMARKS
					Acre					Minutes	In/Hr	CFS			Ft.	Ft/Ft.	In.		CFS	Ft./Ft.	F1/Sec	Sec.	ı
	(1)		(2)		(3)	(4)	(5)	(6)		(7)	(8)	(9)	(10)	(11)	(12)	(13)	(14)		(15)		(16)	(17)	(18)
2	1		3		0.06	0.40	0.02	0.02	0.00	5.00	7.07	0.17	63.87	63.80	14.40	0.005	12	Circular	2.73	0.000	1.94	0.12	
4a	3		CO4a		0.00	0.00	0.00	0.06	0.04	5.12	7.04	0.43	63.80	63.72	15.86	0.005	12	Circular	2.73	0.000	2.53	0.10	
UD4	CO4b		CO4a		0.00	0.00	0.00	0.00	0.00	5.00	7.07	0.00	64.49	64.35	14.38	0.010	4	Circular	0.21	0.000	0.00	0.00	
4b	CO4a		5		0.00	0.00	0.00	0.06	0.00	5 23	7.00	0.42	63 72	63 64	16.58	0.005	12	Circular	2 73	0.000	2.53	0.11	
6	5		7		0.00	0.00	0.00	0.07	0.01	5.34	6.98	0.49	63.64	63.48	31.00	0.005	12	Circular	2.73	0.000	2.63	0.20	
8	7		9		0.00	0.00	0.00	0.08	0.01	5.53	6.90	0.54	63.48	63.40	16.53	0.005	12	Circular	2.73	0.000	2.71	0.10	
UD136	CO13e		CO13b		0.00	0.00	0.00	0.00	0.00	5.00	7.07	0.00	64.45	64.34	10.83	0.010	4	Circular	0.21	0.000	0.00	0.00	
UD13a	CO13b		CO13a		0.00	0.00	0,00	0.00	0,00	5 00	7,07	0,00	64 34	64 25	912	0,010	4	Circular	0.21	0,000	0.00	0,00	1
13a	12		CO13a		0.07	0.65	0.05	0.05	0.00	5.00	7.07	0.32	63.78	63.65	24.71	0.005	12	Circular	2.73	0.000	2.34	0.18	
13b	CO13a		14		0.00	0.00	0.00	0.05	0.00	5.18	7.04	0.32	63.65	63.62	7.39	0.005	12	Circular	2.73	0.000	2.33	0.05	
15	14		16		0.01	0.30	0.00	0.05	0.00	5 23	7,00	0.34	63 62	63 56	11,58	0.005	12	Circular	2 73	0,000	2 37	0.08	
17	16		9		0.00	0.00	0.00	0.05	0.00	5.31	6.98	0.34	63.56	63 40	32.02	0.005	12	Circular	2 73	0.000	2 37	0.23	
10	9		11		0.00	0.00	0.00	0.13	0.00	5.64	6.87	0.87	63.40	63.33	14.37	0.005	12	Circular	2.73	0.001	3.10	0.08	

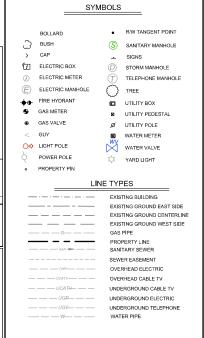
	ER PARK SY C GRADE LIN			PROVEMI	ENTS		PROJEC*	Г:	JAMES I	RIVER PA	ARK SYS	TEM								DESIGN	ED BY:	L. COFFMA	N
	PROBABILIT		15	10	YR															Checked	l:	L. COFFMA	N.
		OUTLET	DIA.	DESIGN	LENGTH	FRICTION	FRICTION					JL	INCTION LO	68							Inlet		T
NLET	INLET	WATER	PIPE	DISCH.	PIPE	SLOPE, Sfo	LOSS											1.3	0.5	FINAL	Water	Rim	Comment
OR	STATION	SURFACE	Do	Qo	Lo		Hr	Vo	Ho	Qi	VI	QIVI	Vi*2/2g	H	Angle	на	Ht	Hit	Ht	н	Surface	Bev	
JUNCTION		ELEV.	(In)	(CFS)	(Ft)	(FT/FT)	(Ft)		(Ft)											(Ft)	⊟evation		
9		64.13	12	0.87	14.37	0.05%	0.01	3.1	0.04	0.54	2.71	1.47	0.11	0.04	83.74	0.06	0.13	0	TRUE	0.07	64.2	66.77	OK -2.
7		64.2	12	0.54	16.53	0.02%	0	2.71	0.03	0.49	2.63	1.28	0.11	0.04	0	0	0.07	0	TRUE	0.04	64.24	67.5	OK -3.
5		64.28	12	0.49	31	0.02%	0	2.63	0.03	0.42	2.53	1.07	0.1	0.03	20	0.02	0.09	0	TRUE	0.05	64.33	67.49	OK -3.
CO4a		64.44	12	0.42	16.58	0.01%	0	2.53	0.02	0.43	2.53	1.08	0.1	0.03	0	0	0.06	0	TRUE	0.03	64.47	67.4	OK -2.
3		64.52	12	0.43	15.86	0.01%	0	2.53	0.02	0.17	1.94	0.33	0.06	0.02	53.18	0.03	0.07	0	TRUE	0.04	64.56	67.36	OK -2
1		64.6	12	0.17	14.4	0.00%	0	1.94	0.01	0	0	0	0	0	0	0	0.01	0.02	TRUE	0.01	64.61	66.44	OK -1.
16		64.2	12	0.34	32.02	0.01%	0 1	2.37	0.02	0.34	2.37	0.81	0.09	0.03	47.1	0.04	0.1	0	TRUE	0.05	64.25	68.1	OK -3.
14		64.36	12	0.34	11.58	0.01%	0	2.37	0.02	0.32	2.33	0.75	0.08	0.03	36.58	0.04	0.09	0	TRUE	0.04	64.4	67.6	OK -3
CQ13a		64.42	12	0.32	7.39	0.01%	0	2.33	0.02	0.32	2.34	0.75	0.08	0.03	0.54	0	0.05	0	TRUE	0.03	64.44	67.77	QK -3.
CO13b		64.52	4	0	9.12	0.00%	0	0	0	0	0	0	0	0	0	0	0	0	TRUE	0	64.52	68.3	OK -3.
CO13c		64.61	4	0	10.83	0.00%	0	0	0	0	0	0	0	0	0	0	0	0	TRUE	0	64.61	68.92	OK -4.
12		64.45	12	0.32	24.71	0.01%	0	2.34	0.02	0	0	0	0	0	0	0	0.02	0.03	TRUE	0.02	64.47	67.3	OK -2.
		T										_		_			_	_		_			T
CO4b		64.62	4	0	14.38	0.00%	0	0	0	0	0	0	0	0	0	0	0	0	TRUE	0	64.62	67.03	OK -2

100 YR STORM SEWER CALCULATIONS

			n Storm Im		ents					PROJE			r Park Syste	em				Design	ed by:	L. Coffr	nan		
			COMPUTA	ATIONS						LOCATIO	N:	City of Rich	mond					Checke	d by:	L. Coffr	man		
NORM F	REQUENCY	100YR	_																UNITS	ENG	LIŞH		
					DRAIN.	RUNOFF		CA		TOTAL	RAIN	RUNOFF	INVERT EL	EVATIONS	LENGTH	SLOPE	SIZE	SHAPE	Capacity	Friction	VEL	FLOW	
PIPE	FROM I	OINT	TO PO	TXT	AREA	COEFF.	INCRE-	ACCUM-	ADDTL	INLET	FALL	Q	UPPER	LOWER	of Pipe		(Dia. Or			Slope	Vn	TIME	
NO	REFERENCE	STA	REFERENCE	STA.	"A"	*C"	MINT	ULATED	CA	TIME			END	END			Span/Rise)						REMARKS
					Acre					Mmutes	ln/Hr	CFS			Ft.	Ft./Ft.	ln.		CFS	Ft/Ft.	Ft/Sec	Sec.	
	(1)		(2)		(3)	(4)	(5)	(6)		(7)	(8)	(9)	(10)	(11)	(12)	(13)	(14)		(15)		(16)	(17)	(18)
2	1		3		0.06	0.40	0.02	0.02	0.00	5.00	9,07	0.22	63.87	63 80	14,40	0.005	12	Circular	2.73	0.000	2.08	0.12	
4a	3		CO4a		0.00	0.00	0.00	0.06	0.04	5.12	9.02	0.55	63.80	63.72	15.86	0.005	12	Circular	2.73	0.000	2.72	0.10	
UD4	CO4b		CO4a		0.00	0.00	0.00	0.00	0.00	5.00	9.07	0.00	64.49	64.35	14.38	0.010	4	Circular	0.21	0.000	0.00	0.00	
4b	CO4a		5		0,00	0.00	0.00	0.06	0 OD	5,23	8,98	0.54	63,72	63 64	16,58	0.005	12	Circular	2,73	0,000	2,72	0,10	
6	5		7		0.00	0.00	0.00	0.07	0.01	5.34	8.93	0.62	63.64	63.48	31.00	0.005	12	Circular	2.73	0.000	2.82	0.18	
8	7		9		0.00	0.00	0.00	0.08	0.01	5.53	8.85	0.70	63.48	63.40	16.53	0.005	12	Circular	2.73	0.000	2.91	0.09	
UD13b	CO13c		CO13b		0.00	0.00	0.00	0.00	0.00	5.00	9.07	0.00	04.45	04.34	10.83	0.010	4	Circular	0.21	0.000	0.00	0.00	
UD13a	СО13ь		CO13a		0.00	0.00	0.00	0.00	0.00	5.00	9.07	0.00	64.34	64.25	9.12	0.010	4	Circular	0.21	0.000	0.00	0.00	
13a	12		CO13u		0.07	0.65	0.05	0.05	0.00	5.00	9.07	0.41	63.78	63.65	21.71	0.005	12	Circular	2.73	0.000	2.51	0.16	
13h	CO13a		14		0.00	0.00	0.00	0.05	0.00	5.18	9.00	0.41	63.65	63.62	7.39	0.005	12	Circular	2.73	0.000	2.50	0.05	
15	14		16		0.01	0.30	0.00	0.05	0.00	5.23	8.98	0.44	63.62	63.56	11.58	0.005	12	Circular	2.73	0.000	2.55	0.08	
17	16		9		0.00	0.00	0.00	0.05	0.00	5.31	8.94	0.43	63.56	63.40	32.02	0.005	12	Circular	2.73	0.000	2.55	0.21	
10	9		- 11		0,00	0.00	0.00	0.13	0.00	5,64	8,81	1.12	63,40	63 33	14,37	0.005	12	Circular	2,73	0,001	3,31	0.07	

17	16		9			1.00 0.0	0.00	0.05	0.00	5.31	8.9	4 0.4	3	63.56	63,40	32.02	0.005	12	Circular	2.73	0.000	2.55 0.21	
10	9		- 11			0.00	0.00	0.13	0.00	5,64	8,8	1 11	2	63,40	63 33	14,37	0.005	12	Circular	2,73	0,001	3,31 0,07	
	ER PARK SYS			PROVEME	NTS		PROJEC	Γ;	JAMES F	RIVER PA	ARK SYS	тем								DESIGN		L. COFFMA	
CIDENCE	PROBABILIT	Υ		100	YR															Checked	i;	L. COFFMA	N.
		OUTLET	DIA.	DESIGN	LENGTH	FRICTION	FRICTION					JL	INCTION LC	SS							Inlet		
NLET	INLET	WATER	PIPE	DISCH.	PIPE	SLOPE, Sfo	LOSS											1.3	0.5	FINAL	Water	Rim	Comment
OR	STATION	SURF/CE	Do .	Qo	Lo		HF	Vo	Ho	Qi	VI	QIVI	V#2/2g	н	Angle	HA	H	Ht	Ht	н	Surface	Blev	
JUNCTION		BLEV.	(In)	(CFS)	(Ft)	(FT/FT)	(Ft)		(Ft)											(Ft)	⊟evation		
9		64.13	12	1.12	14.37	0.08%	0.01	3.31	0.04	0.7	2.91	2.02	0.13	0.05	83.74	0.07	0.15	0	TRUE	0.09	64.22	66.77	OK -2.5
7		64.22	12	0.7	16.53	0.03%	0.01	2.91	0.03	0.62	2.82	1.75	0.12	0.04	0	0	0.08	0	TRUE	0.04	64.26	67.5	OK -3.2
5		64.28	12	0.62	31	0.03%	0.01	2.82	0.03	0.54	2.72	1.48	0.11	0.04	20	0.03	0.1	0	TRUE	0.06	64.34	67.49	OK -3.1
CO4a		64.44	12	0.54	16.58	0.02%	0	2.72	0.03	0.55	2.72	1.49	0.11	0.04	٥	0	0.07	0	TRUE	0.04	64.47	67.4	OK -2.0
3		64.52	12	0.55	15.86	0.02%	0	2.72	0.03	0.22	2.08	0.45	0.07	0.02	53.18	0.03	0.09	0	TRUE	0.05	64.56	67.36	OK -2.
1		64.6	12	0.22	14.4	0.00%	0	2.08	0.02	0	0	0	0	0	0	0	0.02	0.02	TRUE	0.01	64.61	66.44	OK -1.8
					•								•	•	•								
16		64.22	12	0.43	32.02	0.01%	0	2.55	0.03	0.44	2.55	1.11	0.1	0.04	47.1	0.05	0.11	0	TRUE	0.06	64.28	68.1	OK -3.8
14		64.36	12	0.44	11.58	0.01%	0	2.55	0.03	0.41	2.5	1.03	0.1	0.03	36.58	0.04	0.1	0	TRUE	0.05	64.41	67.6	OK 3.1
CO13a		64.42	12	0.41	7.39	0.01%	0	2.5	0.02	0.41	2.51	1.04	0.1	0.03	0.54	0	0.06	0	TRUE	0.03	64.45	67.77	OK -3.3
CO13b		64.52	4	0	9.12	0.00%	0	0	0	0	0	0	0	0	0	0	0	0	TRUE	0	64.52	68.3	OK -3.7
CO13c		64.61	4	0	10.83	0.00%	0	0	0	0	0	0	D	0	0	0	0	0	TRUE	0	64.61	68.92	OK -4.3
12		64.45	12	0.41	24.71	0.01%	О	2.51	0.02	0	0	0	0	0	0	0	0.02	0.03	TRUE	0.02	64.47	67.3	OK -2.8
CO4b		64 62	4	0	14.38	0.00%	0	0	0	0	0	0	0	0	0	0	0	0	TRUE	0	64.62	67.03	OK -2.4





THIS SHEET IS PROVIDED FOR CALCULATION PURPOSES ONLY AND IS NOT TO BE USED FOR CONSTRUCTION.

DATE: MARCH 12, 2018 DRAWN BY: K. ATKINSON

SCALE: AS SHOWN CHECKED BY: A. WEHUNT

REVISIONS

REVISED PER WR'S 07/20/17

REVISED PER WR'S 08/15/17

PEWSED PER WR'S 08/15/17



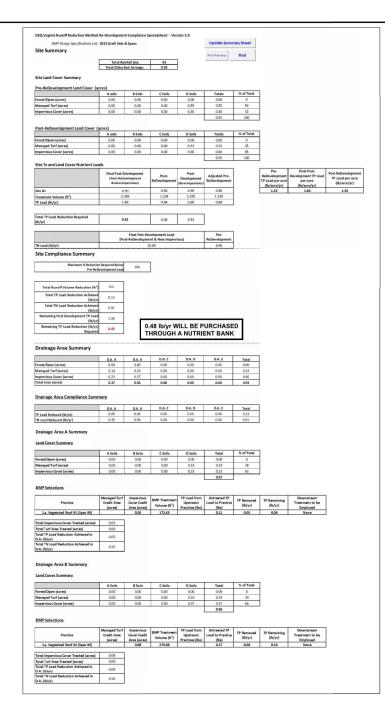


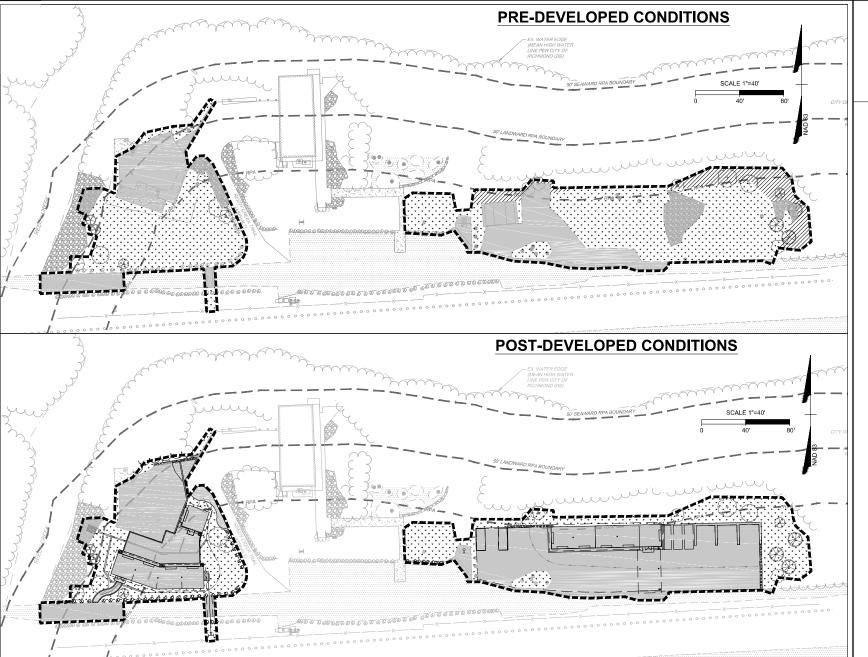
JAMES RIVER PARK SYSTEM WAREHOUSE SITE IMPROVEMENTS

DRAINAGE & STORM CALCULATIONS

DEPARTMENT OF PARKS AND RECREATION CITY OF RICHMOND, VIRGINIA

C5.2





STORMWATER MANAGEMENT NARRATIVE - QUALITY

THE TOTAL WATER QUALITY COMPLIANCE AREA FOR THE PROJECT IS APPROXIMATELY 0.93 ACRES WHICH EQUALS THE LIMITS OF DISTURBANCE. THIS AREA INCLUDES THE INSTALLATION OF TWO EQUIPMENT BUILDINGS. A MAINTENANCE BUILDING, A MATERIAL STORAGE AREA AND STORM SEWER SYSTEM.

PER 9VAC25-870-63 AND 9VAC25-870-65 OF THE VIRGINIA STORMWATER REGULATIONS, WATER QUALITY REQUIREMENTS FOR THE PROJECT WERE DETERMINED USING THE VIRGINIA RUNOFF REDUCTION METHOD (VRRM) AND THE "REDEVELOPMENT" DESIGN CRITERIA. THE SOIL/AREA DATA WAS CALCULATED USING NRCS TR-55 METHOD AND INPUT INTO THE VRRM REDEVELOPMENT SPREADSHEET (v3.0) TO OBTAIN THE PHOSPHORUS REDUCTION REQUIREMENT. REFER TO THE VRRM SUMMARY BELOW.

WATER QUALITY REQUIREMENTS WILL BE MET THROUGH THE USE OF VEGETATIVE ROOFS (Spec#5), AND THE PURCHASE OF WATER QUALITY CREDITS PER 9VAC25-870-69, PERMEABLE PAVERS (Spec#7) AND RAIN BARRELS (Spec#6) ARE INCORPORATED TO PROVIDE NET WATER QUALITY BENEFITS, HOWEVER ARE NOT ACCOUNTED FOR IN THE STORMWATER REGULATORY COMPLIANCE CALCULATIONS. THE BMPS HAVE BEEN DESIGNED AND WILL BE CONSTRUCTED PER DEQ SPECIFICATIONS.





SYMBOLS



EXISTING GROUND EAST SIDE

EXISTING GROUND CENTERLINE

EXISTING GROUND WEST SIDE

GAS PIPE

PROPERTY LINE

SEWER EASEMENT

OVERHEAD ELECTRIC

OCATY

UNDERGROUND CABLE TY

UNDERGROUND TELEPHONE

WATER PIPE

WATER PIPE

THIS SHEET IS PROVIDED FOR CALCULATION PURPOSES ONLY AND IS NOT TO BE USED FOR CONSTRUCTION.

ATE: MARCH 12, 2018

S SHOWN CHECKED BY: A. WEHU

RÎCHMOND B. I

| REVISIONS | | REVISIONS | | REVISED PER WR'S 07/20/1 | REVISED PER WR'S 08/15/1 | REVISED PER WR'S 12/22/1 | REVISED PER WR'S 03/12/11 | AS BUILT DATE

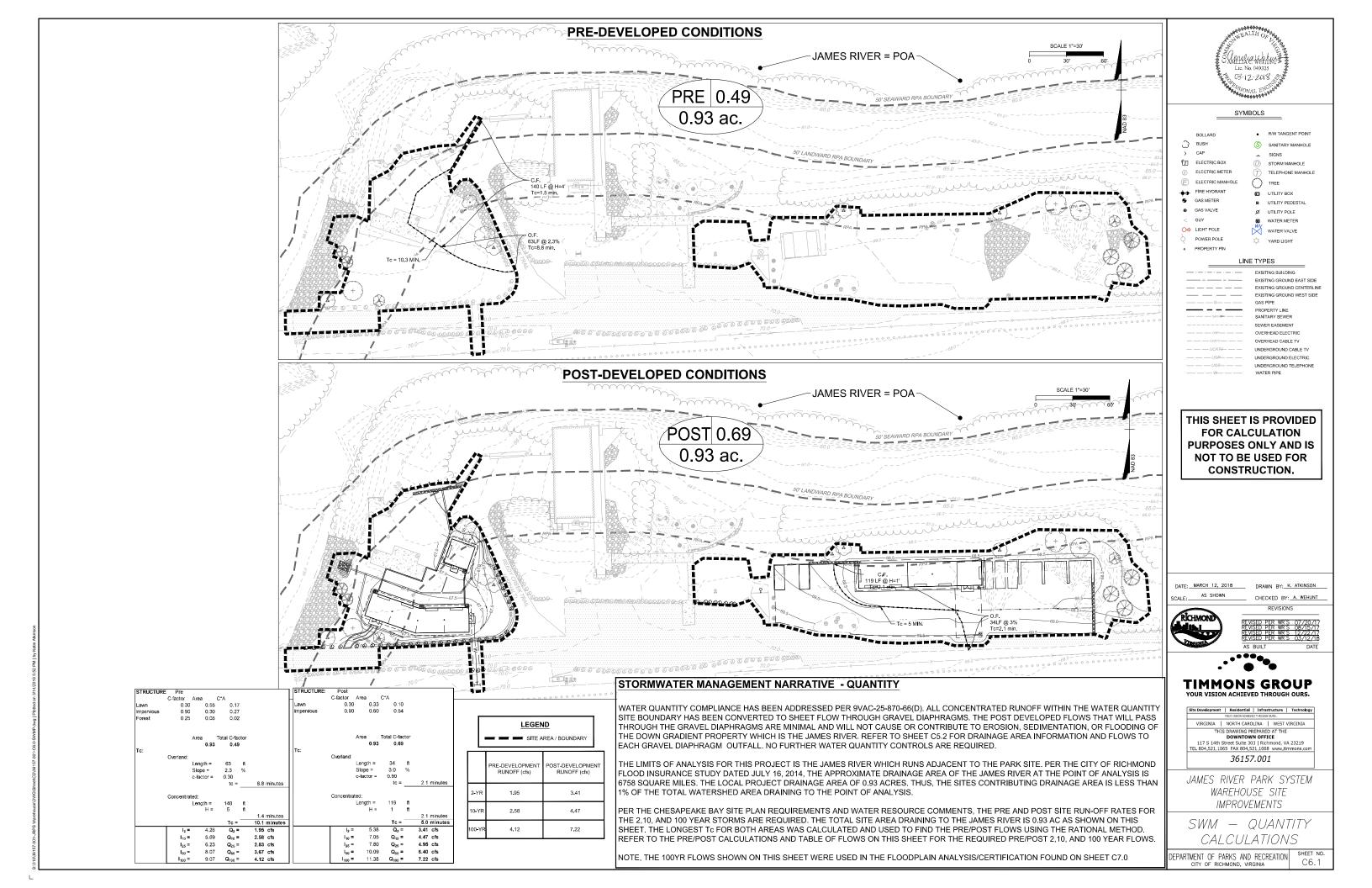


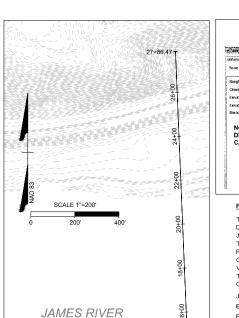
JAMES RIVER PARK SYSTEM WAREHOUSE SITE IMPROVEMENTS

SWM — QUALITY CALCULATIONS

DEPARTMENT OF PARKS AND RECREATION CITY OF RICHMOND, VIRGINIA

C6.0





R Workstein Grovestive 1975 12 Uniform Flow : Graduativ Varied Flow 2 Aerosone So,ve For. Normal Depth 9.00500 71.80 NOTE: REFER TO C7.1 FOR DETAILED FLOWMASTER CALCS.

				er carrieren vie baterial at Statistist	TAX - MANUAL TRUE
Monte can value and					Come of
inform Flow Gradually Vari					
Solve For. Normal Dept	<u> </u>	8	Friction Method.	Manning Formula	
Roughness Coefficient	0.000		FlowArtes	27334.91	Its
Channel Stope:	0.00500	NIII	Welled Penmeier	2378.18	п
Elevation:	71 80	я	Hydraulic Radius.	11,49	n
Elevation Stange:	60.00 to 119.10 R		Top Width:	2072 75	
Discharge:	290037.22	R*98	Normal Depth:	15.80	
			Critical Depth:	11.67	
NOTE: REFE	R TO C7.1 F	OR	Critical Stope	0.01841	D/R
DETAILED FL	OWMASTE	R	Velocity:	1961	F/5
CALCS.			Velocity Head	1.75	*
			Specific Energy:	17.55	a
			Foude Number		

TIMMONS GROUP

James River 100-yr flow per CoR FIS Study = 290,000 cfs

Ex. 100-yr flow from JRPS Site = 4.12 cfs Prop. 100-yr flow from JRPS Site = 7.22 cfs Ex. 100-yr flow input into Flowmaster = 290,004.12 cfs Prop. 100-yr flow input into Flowmaster = 290,007.22 cfs

Prevost-White, Jonet M. - DPU

suggest an operating plan be in place to empty the buildings when a flood that will get in

James Piver Parts would like to build a few sheds near their existing admin building. The area where they plan to build is floodeay. They are proposing the otherchros (comprosed of a series of units) that will be for storage only. They understand electric spatient, for judy is work have to be elected and they are proposing one wall on the suchian would be and up of boost and heavy equipment in and out off). They ready can't be developed. I booked through 44CPR 603 but mostly apple

Dear Ms. Prevost-White,

FLOODPLAIN NARRATIVE:

THE CITY OF RICHMOND DEPARTMENT OF UTILITIES WATER RESOURCE DIVISION REQUESTED THAT A CROSS SECTION OF THE JAMES RIVER BE ANALYZED JUST DOWNSTREAM OF THE PROPOSED IMPACTS USING THE PRE AND POST-DEVELOPED 100-YR FLOWS AT THE POINT OF ANALYSIS. THE OVERALL FLOW WITHIN THE JAMES RIVER WAS FOUND USING THE CITY OF RICHMOND'S FLOOD INSURANCE STUDY (FIS) DATED JULY 16, 2014, PER TABLE 1-SUMMARY OF DISCHARGES (PG 7) PROPOSED 100-YR FLOW AT THE DOWNSTREAM CORPORATE LIMITS WAS 290,000 CFS. THIS FLOW AS THE BASE FLOW AND ADDED THE EXISTING AND PROPOSED 100-YR FLOWS FROM OUR SITE TO THE OVERALL FLOW. AN IRREGULAR CROSS SECTION WAS THEN DEFINED IN BENTLEY'S FLOWMASTER USING CITY GIS TOPOGRAPHY AND STATION/ELEVATION POINTS ALONG THE CROSS SECTION, GOOGLE EARTH AERIAL IMAGERY WAS USED TO DEFINE THE MANNING'S N VALUES. THE CALCULATED FLOWS WERE THEN INPUT INTO THE CROSS SECTION, ALONG WITH THE CHANNEL SLOPE FROM THE CITY'S GIS TOPOGRAPHY, TO FIND THE RESULTING WATER SURFACE ELEVATION (WSE) FOR THE PRE AND POST DEVELOPED 100-YR FLOWS, REFER TO C7.1 FOR ADDITIONAL FLOWMASTER CALCULATIONS. THE FOLLOWING DATA WAS USED FOR THE ANALYSIS:

JAMES RIVER 100-YR FLOW PER CoR FIS STUDY = 290,000 CFS

EX. 100-YR FLOW FROM JRPS SITE = 4.12 CFS

PROP 100-YR FLOW FROM JRPS SITE = 7 22 CFS

EX. 100-YR FLOW INPUT INTO FLOWMASTER = 290,004.12 CFS

EX. WSE RESULT = 71.80'

PROP. 100-YR FLOW INPUT INTO FLOWMASTER = 290,007.22 CFS

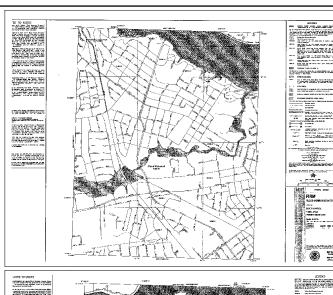
PROP. WSE RESULT = 71.80'

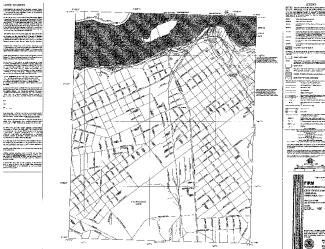
PER THE CROSS-SECTION ANALYSIS, THE PROPOSED WORK WILL NOT RESULT IN ANY INCREASE IN FLOOD LEVELS DURING OCCURRENCE OF THE BASE FLOOD.

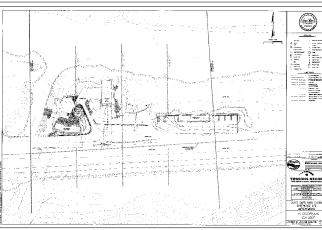
	Peak Discharges (cubic feet per second)									
Flooding Source and Location	Drainage Area (square miles)	10-Percent- Annual-Chance	2-Percent- Annual-Chance	1-Percent- Annual-Chance	0.2-Percent- Annual-Chan					
BROAD ROCK CREEK										
At mouth	6.02	2,900	5.300	6,400	9,600					
Central Avenue	3.71	1,900	3,200	3,800	6,700					
Jefferson Davis Highway Seahoard Coastline Railread	2.79	900	1,200	1,300	4,000					
Crossing	2.25 1.40	1,400	2,500	3,000	4,700					
Hopkins Road	1.40	800	1,450	1,800	2,800					
CHEROKEE CRITICK										
At mouth	1.03	270	1,706	1.100	2,300					
Garden Street	0.31	110	270	600	1.150					
Hobbybill Road	0.24	70	210	480	1,100					
GILLIES CREEK										
Jennie Scher Road	14.23	3,200	5,800	7,100	11,000					
GOODES CREEK										
At mouth	1.82	600	1,300	1,800	4,200					
Royall Avenue	1.16	0,	400	800	2,900					
GRINDALL CREEK										
At corporate limits	2.57	1,000	1.400	1.600	2,000					
Castlewood Road Upper Scaboard Coastline Rastroad	1.97	520	620	660	750					
Crossing	1.66	1,100	2,100	2,600	3,900					
Hopkins Road	0.85	600	1,200	1,600	2,300					
JAMES RIVER										
At downstream corporate limits	6,758,00	131,000	232,000	290,000	475,000					
PITTAWAY CREEK										
At mouth	0.59	340	670	830	1,200					
POCOSHAM CREEK										
At mouth	5.97	2,100	4.000	5,000	9,500					
Above West Branch Pocosham		-,	1,000	2,000	/y.nn					
Creek	4,07	1,600	3,100	3.800	6,000					
Chippenham Parkway	1.07	600	1,100	1,400	2,200					
POWIETE CREEK										
At mouth	12.09	2.200								
	12.09	2,200	4,200	5,200	8,200					
RATTLESNAKE CREEK.										
At mouth	2.72	1,150	2,200	2,700	4,000					
Chippenham Parkway	1.80	670	1,300	1,600	2,400					
STONY POINT CREEK										
At mouth	1.03	560	1.100	1.350	2.000					

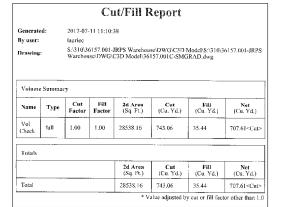
	PRE/P	ALCUI			- 17	-
STRUCTUR	E Pre					
Lawn	C-factor A	vrea	C'A			
Impervious	0.30	0.55	0.17			
Forest	0.30	0.08	0.02			
. 0.00.	0.20	4.00	0.04			
	,	Area '	Total C fo	atar		
	,	0.93	0.49			
To:		0.50	0.40			
	Overland:					
		ength =		ft		
	٤	Slope =	2.3	%		
	c	-factor =				
			lc =		8.8	minutes
	Concentrate					
	L	ength = H =	140	ft		
		H =	5	ft		
			To-	_	10.1	minutes minutes
	l ₂ =	1.28			1.95	
	l ₁₀ =		Q ₁₆ ≃		2.58	
	100	6.23			2.83	
	lea =	8.07			3.67	
	I ₁₀₀ m	9.07	Q ₁₀₀ =	_	4.12	ole i
STRUCTUR		0101	- 140	-	71112	0.0
	G-factor A	vea (C*A			
Lawn	0.30	0.33	0.10			
Impervious	0.90	0.60	0.54			
	A		Total C-fa			
Fe:		0.93	0.69			
ıç.	Overland:					
		ongth =	34	ff		
		lope -	3.0	%		
		-factor =	0.90			
			1c =	_	2.1	minutes
	Concentrate	et:				
		ength =	119	ft		
		H so	1	ft		
			_	_	2.1	minutes minutes
		F 00	Tc=		5.0	minutes
		5.38			3.41 4.47	
	12 =					
	110 =		Q10 =			
	110 = 125 =	7.05 7.80	Q10 = Q25 = Q50 =		4.95	cfs
	110 =	7.05 7.80 10.09	Q10 = Q25 = Q50 = Q100 =	_		cfs cfs

ES TO MICHIE P 804.200.6500 F 804.560.1016 w. The state of the s ...immer Rhey Park System is the existing headquarters for the City Department of Pouls, Riccreation and Community Facilities (Department). This situates oserous as the Department's maintenance and consisting for their equipment, vehicloids, and market alsonge, As an initiative to clean up the situal not compared to the programment of the programment proposes to based three small alonge was rehouses mark after exist administration belonging, a convent estimate growing a result and simple type. The washfourse will be consistented to control or control force, pressure treated word with familie, colorized control or contro Telephone . minimum. Charlie Banks, with Virginie Department of Conservation and Recreation's Dem Safety and Floodplain Management Division, further interprets this section of the Ordinance with the totlowing directive: Determine how high up the building the water will be during the 1% chance float (BFE). Using the square looting of the building, contains the cubic float volume of the 1% change flooting will display will displayed. All least the around of volume rooted to be consorted to the flooting openivation receipt to steep the building that reading an increase in the BPE. Propely done and documented, this can take the photo of the compiles float study. CORE TO USERS the family of the Charles and the second second part for the format of a country part of the country that is the format of the format A Michigan Andrew Company of the Com The second section is a laboratory of the second section in the second section in the second section in the second section in the second section is a section in the second section in the section is a section in the section in the section in the section in the section is a section in the section in the section in the section is a section in the section iloserene Significa intermedical. CENTRAL er the cross-section analysis, the proposed work will not result in any increase in flood levels during currence of the base flood. Interpretation terms of the control This letter is to certify that I am a qualified professional engineer licensed to praction in the Commonwealth Virginia. It is to further contrib, in my opinion, that the design data included within this letter supports the fact he proposed development will not significantly impact the 100-year fixed elevation, Boodway elevation or floodway widthe on the James River in the variety of the proposed development.











 BUSH > CAP SIGNS ELECTRIC BOX STORM MANHOLE ELECTRIC METER TELEPHONE MANHOLE ELECTRIC MANHOLE TREE → FIRE HYDRANT UTILITY BOX GAS METER ■ UTILITY PEDESTAL Ø UTILITY POLE

○ LIGHT POLE

POWER POLE PROPERTY PIN

WATER VALVE ☼ YARD LIGHT

WATER METER

EXISTING GROUND EAST SIDE EXISTING GROUND CENTERLIN EXISTING GROUND WEST SIDE PROPERTY I INF SEWER EASEMENT OVERHEAD ELECTRIC

OVERHEAD CABLE TV UNDERGROUND ELECTRIC UNDERGROUND TELEPHONE

THIS SHEET IS PROVIDED FOR CALCULATION **PURPOSES ONLY AND IS** NOT TO BE USED FOR CONSTRUCTION.

DATE: MARCH 12, 2018 DRAWN BY: K. ATKINSON

REVISIONS AS BUILT



VIRGINIA | NORTH CAROLINA | WEST VIRGINIA THIS DRAWING PREPARED AT THE **DOWNTOWN OFFICE**117 S 14th Street Suite 303 | Richmond, VA 23219
TEL 804.521.1065 FAX 804.521.1068 www.timmons.cor

36157.001

JAMES RIVER PARK SYSTEM WAREHOUSE SITE **IMPROVEMENTS** FLOODPLAIN CERTIFICATION AND

DEPARTMENT OF PARKS AND RECREATION

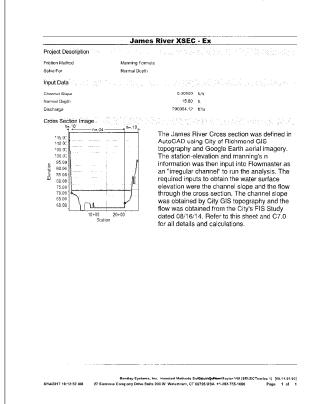
SHEET NO.

JRPS

SITE-

JAMES RIVER X-SEC

FLOWMASTER MODEL REPORT - EXISTING CONDITIONS



	James Ri	ver Calcs -	Ex	
Project Description	and the second			February Programs
Friction Method	Manning Formula			
Solve For	Normal Depth			
Input Date		414. 120. km		35436° - 35446 s
Channel Slope		0.00500 ry		
Discharge		290004.12 f(²)		
Section Definitions				
Station (it)	E	evetion (fi)		Monrang's n
	0+02.04	1*	19.18	.10
	0+21.96	1*	18.00	1
	f+17.1	7	70.00	.10
	2+90.94		98.00	.04
	3+86.57	-	36.00	
	4+10.15	ŧ	52.00	
	4+61.84	6	50.00	
	4+69.7		56.00	
	6+28.9	ţ	6.00	
	6+53	6	52.00	
	8+25.49		32.D0	
	8+59.7		50.00	
	8+63.42		58.00	
	9+50		58.00 58.00	
	9+50 9+68.90	6	58.00 58.00 52.00	
	9+50 9+68.90 9+84.67	6 6	58.00 58.00 52.00 52.00	
	9+80 9+68.90 9+84.67 10+02.85	6 6 6	58.00 58.00 52.00 52.00	
	9+50 9+68.99 9+84.67 10+02.85 22+12.42	6 6 6	58.00 58.00 52.00 52.00 58.00	.04
	9+80 9+68,99 9+84,67 10+02.85 22+12.42 22+23,69	5 6 5 2	58.00 58.00 52.00 52.00 58.00 58.00	.04 .10
	9+850 9+88.99 9+84.67 10+02.85 22+12.42 22+23.69 24+10.74	5 6 5 0 6	58.00 58.00 52.00 32.00 58.00 58.00 32.01	
	9+50 9+68,99 9+84,67 10+02,85 22:12,42 22:23,69 24:10,74 24:74,59	5 6 6 6 6 6	58,00 58,00 52,00 52,00 52,00 58,00 58,00 52,01 54,00	
	9+50 9+69:99 9+69:97 10+02:85 22+12:42 22+23:69 24+10:74 24+74:59 25+23:71	5 6 6 6 6 6 8	58,00 58,00 52,00 52,00 58,00 58,00 58,00 54,00 54,00 54,00	
	9+50 9+68,99 9+84,67 10+02,85 22:12,42 22:23,69 24:10,74 24:74,59	6 6 5 6 6 8 8 8	58,00 52,00 52,00 52,00 58,00 58,00 58,00 58,00 58,00 58,00 54,00 54,00	
	9+60 9+68,90 9+64,67 10+02,85 22+12-42 22+23,69 24+10,74 24+74.59 25+23,71 25+66,06	6 6 6 6 8 8	58,00 58,00 52,00 52,00 58,00 58,00 58,00 54,00 54,00 54,00	
	9+50 9+68.67 10+02.85 22+12-42 22+23.69 24+10.74 24+74.59 25+23.71 26+60.66	6 6 6 6 6 8 8 7 7	58.00 58.00 52.00 52.00 58.00 58.00 58.00 58.00 58.00 58.00 58.00 58.00 58.00	

27 + : 27+:	Elevation 8, 12 38, 11 56, 26 56, 04	(n)	97.05	
27+ 27+ 27+	18.12 38.11 58.26	(M)	97.05	
27+ 27+ 27+	18.12 38.11 58.26	(ft) .	97.05	
27 + : 27+:	38.11 58.26		97.05	
27+	58.26			.10
			96.94	
214			95.58 95.94	.10
			50.54	.iu
Options	n se me a des Ass			
Current Kougnness Weignled Method Open Channel Weighting Method	Pavlovskii's Method Pavlovskii's Wethod			
Closed Channel Weighting Method	Psylovskii's Method			
Results	. Galaki se			and the first section of the section
Normal Depth		15.80	А	
Elevation Range	56.00 to 119.18 ft			
Flow Area	2	7334.73	tt²	
Wetted Perimeter		2378.18	ft	
Hydraulic Radius		11.49	ft.	
Top Width		2372.72	ft	
Normal Depth		15.80	ft	
Critical Depth		11.67	ft	
Critical Slope		0.01841	UNE.	
Velocity		10.61	ft/s	
Velocity Head		1.75	ft	
Specific Energy Froude Number		17.55 0.55	R	
Flow Type	Subcritical	5.35		
GVF Input Data		14.59°		
Downstream Depth		0.00	ft	
i ength		0.00	ft	
Number Of Steps		0		
GVF Output Data		nyi.	J-1850	
Upstream Depth		0.00	ft	

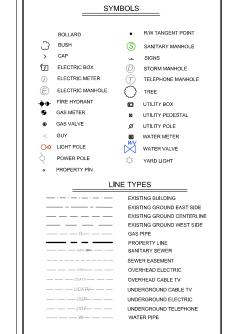
Jame	es River Calcs	3 - Ex
GVF Output Data		ana ataon, no cambo
Profile Description		
Profile Headless	0.00	rt .
Downstream Velocity	Infinity	Rt/s
Upstream Velocity	Infinity	IU's
Normal Depth	15.80	π
Critical Depth	11.67	R.
Channel Slope	0.00509	
Critical Slope	0.01841	funt

James River Calcs - Prop

Infinity It/s
Infinity It/s
Infinity It/s
15.80 ft
11.67 ft
0.00500 ft/ft

GVF Output Data Profile Headloss

Channel Slope Critical Slope



THIS SHEET IS PROVIDED FOR CALCULATION **PURPOSES ONLY AND IS** NOT TO BE USED FOR CONSTRUCTION.

DATE: MARCH 12, 2018 DRAWN BY: K. ATKINSON REVISIONS AS BUILT



VIRGINIA | NORTH CAROLINA | WEST VIRGINIA THIS DRAWING PREPARED AT THE **DOWNTOWN OFFICE**117 S 14th Street Suite 303 | Richmond, VA 23219
TEL 804.521.1065 FAX 804.521.1068 www.timmons.com 36157.001

JAMES RIVER PARK SYSTEM WAREHOUSE SITE *IMPROVEMENTS*

FLOODPLAIN CERTIFICATION

DEPARTMENT OF PARKS AND RECREATION CITY OF RICHMOND, VIRGINIA

FLOWMASTER MODEL REPORT - PROPOSED CONDITIONS

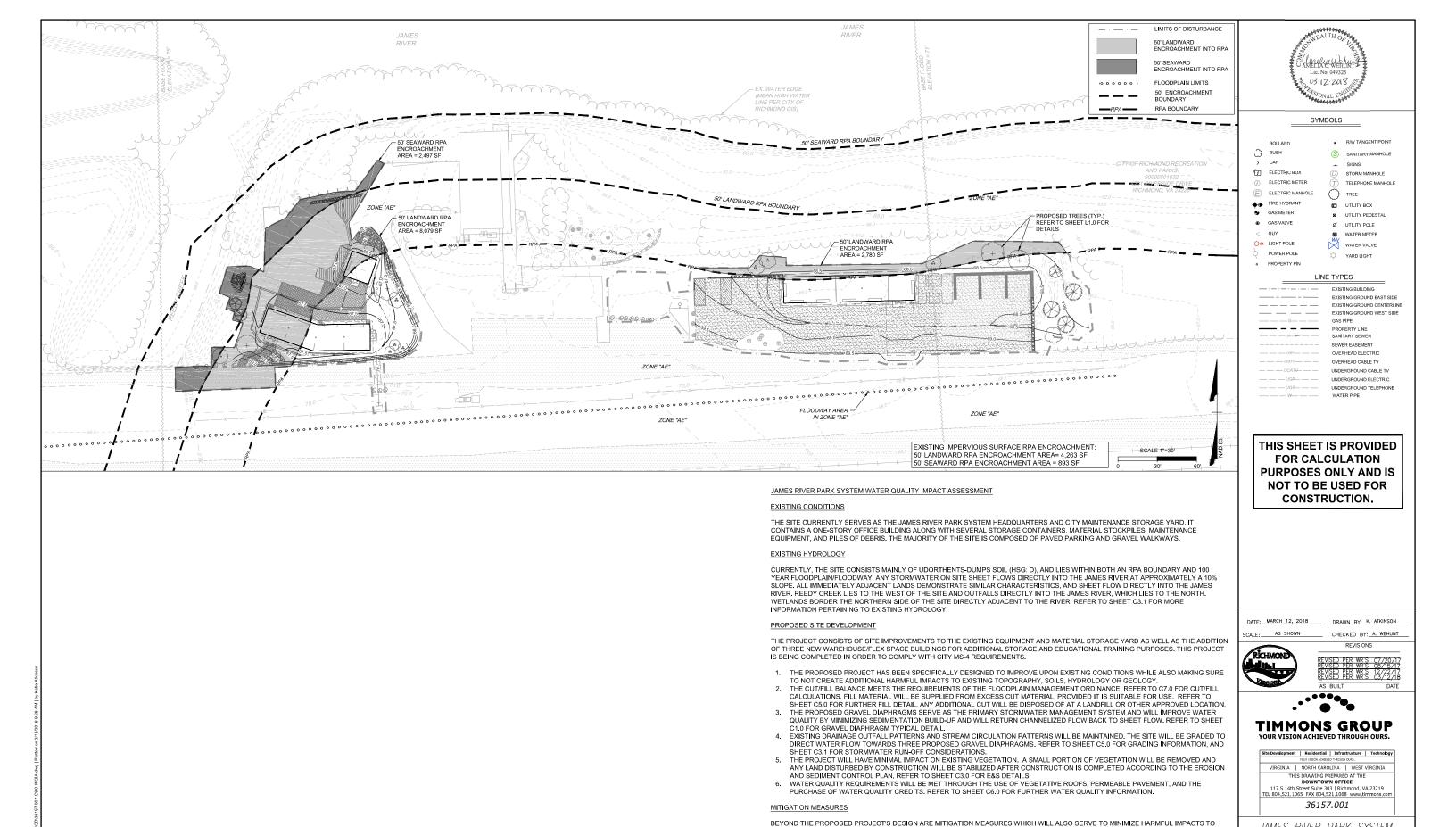
	James River XSEC - Prop
Project Description	NEW YORK OF THE SERVER AND THE SERVER
Friction Method	Manning Formula
Solve For	Normal Depth
Input Data	
Channel Slope	0.00500 ft/ft.
Normal Depth	15.80 n
Discharge	290007.22 ft ³ /s
Cross Section Image 10 n = .04 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	The James River Cross section was defined in AutoCAD using City of Richmond GIS topography and Google Earth aerial imagery. The station-elevation and manning's n information was then input into Flowmaster as an "irregular channel" to run the analysis. The required inputs to obtain the water surface elevation were the channel slope and the flow rough the cross section. The channel slope was obtained by City GIS topography and the flow was obtained from the City's FIS Study dated 08t 16'14. Refer to this sheet and C7.0 for all details and calculations.
8/14/2617 10:14:63 AM 27 Siemon:	Sentiev Systems, Inc. Macestad Methods SolBlindingsFlaceMaster VRI (SELECT-ceries, 1) (Inc. 1 of 1 of Company Drive Suite 280 W Watertown, CT 00795 USA +1-233-755-1566 Page 1 of

	James Rive	r Calcs	- Prop	
Project Description				a Nasay en la jaran a
Friction Method Salve Fer	Manning Formula Normal Depth			
Input Data	te lugi espaia		5 . SAN	
Channel Slope Discharge Section Definitions		0.00500 290007.22		
Station (ff).	Eleva	ition (fi)	6145 64.3	Monning's n
	0+02.04		119.18	.10
	0+21.96		118.00	Į.
	1+17.1		70.00	.10
	2+90.94		68.00	.04
	3+86.57		66.00	
	4+10.15 4+61.84		62.00 60.00	
	4+61.04		56.00	
	6+28.9		56.00	
	6+50		62.00	
	8+25.49		62.00	
	5=59.7		60.00	
	8+63.42		58.00	
	9+50		58.00	
	9+68.99		62.00	
	9+84.67		62.00	
	0+02.85		58.00	
	2:12.42		58.00	.04
	2+23.60		62.01	.10
	4+10.74		64.00	
	4+74.59		68.00	
	5+23.71		84.00	
	5+56.06		84.00	
	6+67.63 6+11.69		78.00	
			78.00	
	%+34.8B %+57.3B		86.00 88.00	.10

(n)out Data Station (ii) 27-06.12 27-96.11 27-96.26 27-98.04	lori (ff)	97.05 .10 99.94 96.58 \$95.94 .10	
27+08.12 27+38.11 27+58.28	ion (fi)	96.94 95.58	
27+38.11 27+56.28		96.94 95.58	
27+58.28		95.58	
27+98.04		95.94 .10	
Options		garden Nagerigatia	
Gurrent Kougnness weignted Pavlovskii's Method Method			
Open Channel Weighting Method Pavlovskii's Method			
Closed Channel Weighting Method Pavlovskii's Method			
Results			
Normal Depth	15.80	PL .	
Elevation Range 56.00 to 119.18 ft			
Flow Area	27334.91	M ₄	
Wetted Perimeter	2378.18	n	
Hydraulic Radius	11.49	π	
Top Width	2372.72	R.	
Normal Depth	15.80	Rt.	
Critical Depth	11.67	ft	
Critical Slope	0.01841	11/11	
Velocity	10.61	ft/s	
Velocity Head	1.75	ft	
Specific Energy	17.55	п	
Froude Number Flow Typs Subcritical	0.55		
GVF Input Data		Dela 7 A Jepapa Baldane na 3	7.5
Downstream Depth	0.00	ting menggal milag lam yapi lag yasi se A	
Length		T T	
Number Of Steps	0		
GVF Output Data	45.46		
Upstream Depth	0.00	ft	

15.80 r 27334.91 r	l
27334.91	
27334.91	
2378.18 f	•
11.49 f	•
2072.72	
15.80 f	
11.67 f	
0.01841 1	
10.61 f	
1.75 f	t
17.55 r	t.
0.55	
William Pack	54.74.5 46 1365715176376.
0.00 f	t
0.00 n	
0	
0.145/36(F)	
0.00 f	
	<u> </u>

CALCULATIONS



EXISTING ENVIRONMENTAL CONDITIONS.

• A COMPLETE EROSION AND SEDIMENT CONTROL PLAN HAS BEEN PREPARED TO ENSURE PROPER EROSION AND STORMWATER RUNOFF CONTROL THROUGHOUT THE CONSTRUCTION. REFER TO SHEET C3.0 FOR DETAILS.

THE NECESSARY GRADING MEASURES WERE MINIMIZED TO AVOID MAJOR ENVIRONMENTAL IMPACTS.
THE PROPOSED PROJECT HAS BEEN DESIGNED TO MINIMIZE REQUIRED VEGETATION CLEARING, CUT AND FILL EARTHWORK AND

DISTURBANCE OF SURROUNDING NATURAL AREAS. ALL DENUDED AREAS WILL BE STABILIZED ACCORDING TO THE EROSION AND SEDIMENT CONTROL PLAN. REFER TO SHEET C5.0 FOR ADDITIONAL GRADING AND DRAINAGE INFORMATION.

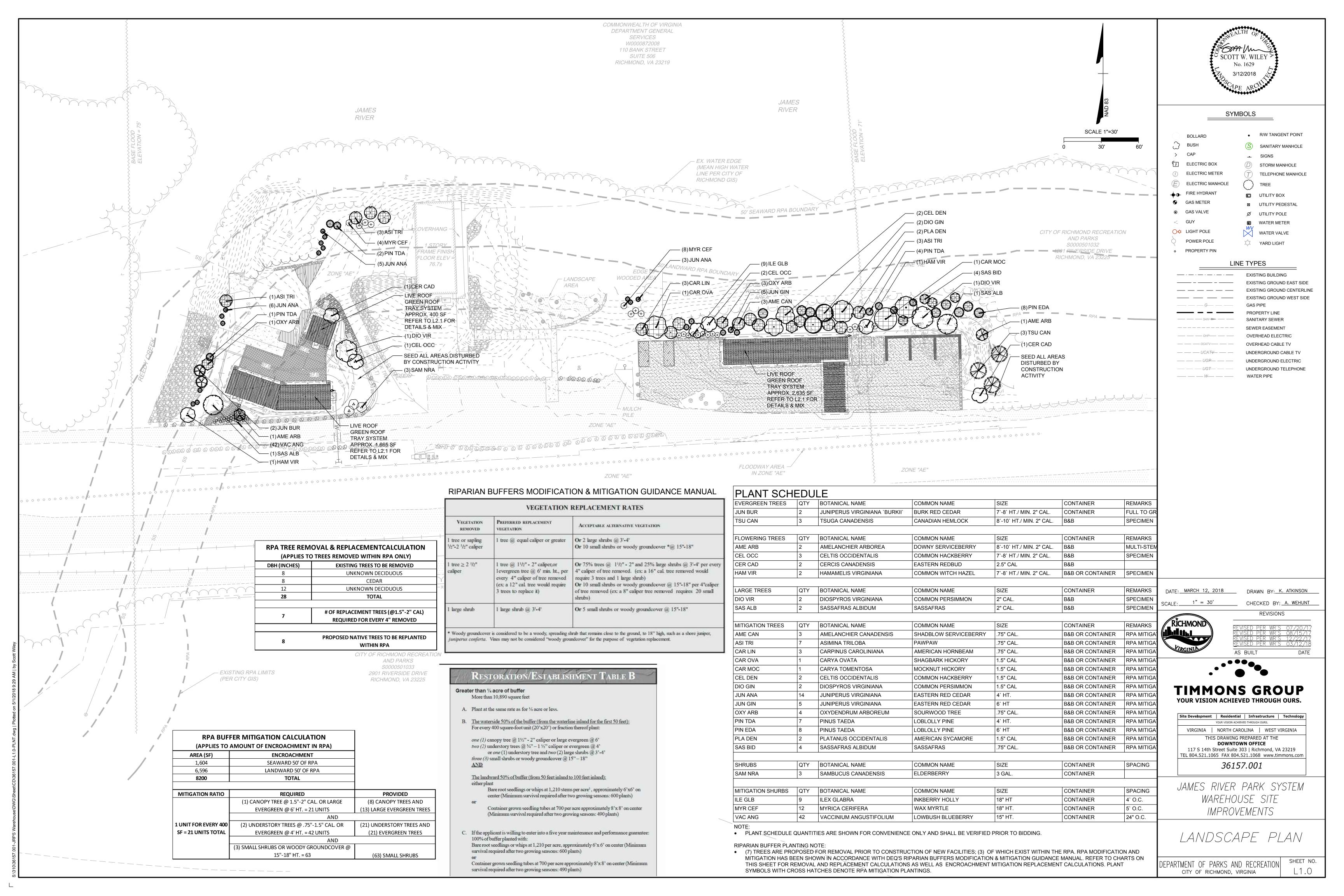
 LANDSCAPING HAS BEEN PROVIDED IN AREAS WHERE EXISTING VEGETATION WILL BE DISTURBED. REFER TO LANDSCAPING PLAN ON SHEETS L1.0 AND L1.1 FOR DETAILS.

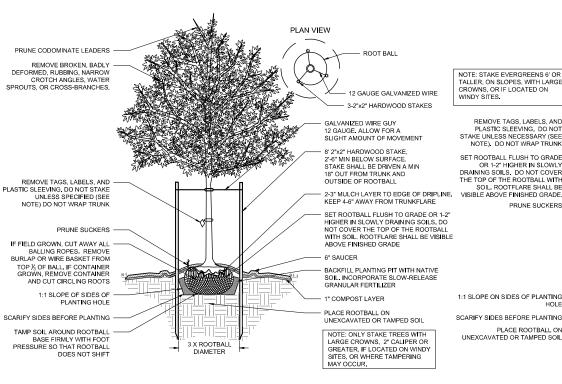
JAMES RIVER PARK SYSTEM WAREHOUSE SITE IMPROVEMENTS

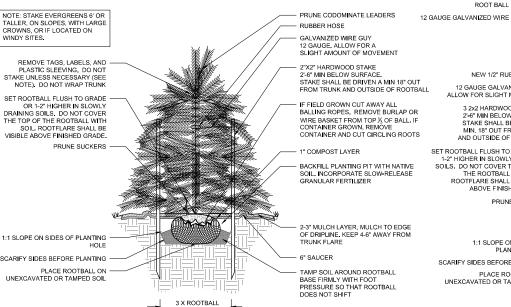
WATER QUALITY
IMPACT ASSESSMENT

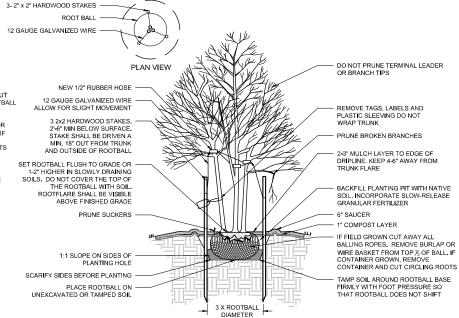
DEPARTMENT OF PARKS AND RECREATION CITY OF RICHMOND, VIRGINIA

C8.0











MULTI-STEM TREE - STAKING SPECIFIED

GENERAL NOTES

- CONTRACTOR IS RESPONSIBLE FOR CONTACTING "MISS UTILITY" AT 1.800.552,7001 FOR LOCATION OF ALL UTILITY LINES.TREES SHALL BE LOCATED A MINIMUM OF 5 FEET FROM SEWER/WATER CONNECTIONS. NOTIFY LANDSCAPE
- VERIFY ALL PLANT MATERIAL QUANTITIES ON THE PLAN PRIOR TO BIDDING, PLANT LIST TOTALS ARE FOR CONVENIENCE ONLY AND SHALL BE VERIFIED PRIOR TO BIDDING.
- ONLY AND SHALL BE VERHEID PRIOR TO BIDDING.
 PROVIDE PLANT MATERIALS OF QUANTITY, SIZE, GENUS, SPECIES, AND VARIETY INDICATED ON PLANS, ALL PLANT
 MATERIALS AND INSTALLATION SHALL COMPLY WITH RECOMMENDATIONS AND REQUIREMENTS OF ANSI Z60.1
 "AMERICAN STANDARD FOR NURSERY STOCK". IF SPECIFIED PLANT MATERIAL IS NOT OBTAINABLE, SUBMIT PROOF OF
 NON AVAILABILITY TO THE ARCHITECTS, TOGETHER WITH PROPOSAL FOR USE OF EQUIVALENT MATERIAL.
 PROVIDE AND INSTALL ALL PLANTS AS IN ACCORDANCE WITH DETAILS AND CONTRACT SPECIFICATIONS
 ONLY TESTS AND A CONTRACT OF THE PROPOSAL OF THE OWNER OF THE OWNER OF THE OWNER OF THE OWNER OWNER.
- SOIL TESTS SHALL BE PERFORMED TO DETERMINE SOIL CHARACTER AND QUALITY, NECESSARY SOIL AMENDMENTS SHALL BE PERFORMED PER TEST RESULTS TO ENSURE PLANT HEALTH.

CONSTRUCTION/INSTALLATION

- LANDSCAPE ARCHITECT RESERVES THE RIGHT TO REJECT ANY PLANTS AND MATERIALS THAT ARE IN AN UNHEALTHY OR UNSIGHTLY CONDITION. AS WELL AS PLANTS AND MATERIALS THAT DO NOT CONFORM TO ANSI Z60.1 "AMERICA

- OR UNSIGHTLY CONDITION, AS WELL AS PLANTS AND MATERIALS THAT DO NOT CONFORM TO ANSI 260,1 "AMER STANDARD FOR NURSERY STOCK"

 LABEL AT LEAST ONE TREE AND ONE SHRUB OF EACH VARIETY AND CALIPER WITH A SECURELY ATTACHED, WATERPROOF TAG BEARING THE DESIGNATION OF BOTANICAL AND COMMON NAME.

 INSTALL LANDSCAPE PLANTINGS AT ENTRANCES/EXITS AND PARKING AREAS ACCORDING TO PLANS SO THAT MATERIALS WILL NOT INTERFERE WITH SIGHT DISTANCES.

 CONTRACTOR IS RESPONSIBLE FOR WATERING ALL PLANT MATERIAL DURING INSTALLATION AND DISTANCES. INSPECTION AND ACCEPTANCE BY OWNER. CONTRACTOR SHALL NOTIFY OWNER OF CONDITIONS WHICH AFFECTS

INSPECTIONS/GUARANTEE

- UPON COMPLETION OF LANDSCAPE INSTALLATION, THE LANDSCAPE CONTRACTOR SHALL NOTIFY THE GENERAL CONTRACTOR WHO WILL VERIFY COMPLETENESS, INCLUDING THE REPLACEMENT OF ALL DEAD PLANT MATERIAL.
 CONTRACTOR IS RESPONSIBLE FOR SCHEDULING A FINAL INSPECTION BY THE LANDSCAPE ARCHITECT.
 ALL EXTERIOR PLANT MATERIALS SHALL BE GUARANTEED FOR ONE FULL YEAR AFTER DATE OF FINAL INSPECTION
- AGAINST DEFECTS INCLUDING DEATH AND UNSATISFACTORY GROWTH, DEFECTS RESULTING FROM NEGLECT BY
- AGAINST IDEFECTS INCLUSING PARTH AND OBSTITEMENT GROWTH, SEPTEMENT SECTION FROM PROBLEMENT THE OWNER, ABUSE OR DAMAGE BY OTHERS, OR UNUSUAL PHENOMENA OR INCIDENTS WHICH ARE BEYOND THE CONTRACTORS CONTROL ARE NOT THE RESPONSIBILITY OF THE CONTRACTOR PROBLEMENT FOR THE PLANT MATERIAL QUANTITIES AND SIZES WILL BE INSPECTED FOR COMPLIANCE WITH APPROVED PLANS BY A SITE PLAN REVIEW AGENT OF THE PLANNING DEPARTMENT PRIOR TO THE RELEASE OF THE CERTIFICATE OF
- OCCUPANCY.

 REMOVE ALL GUY WIRES AND STAKES 12 MONTHS AFTER INSTALLATION.



PROPERTY LINE SEWER EASEMENT OVERHEAD ELECTRIC

No. 1629

3/12/2018

SYMBOLS

SIGNS

TREE

UTILITY BOX

Ø UTILITY POLE

WATER METER

YARD LIGHT

EXISTING BUILDING

GAS PIPE

EXISTING GROUND EAST SIDE

EXISTING GROUND WEST SIDE

EXISTING GROUND CENTERLINE

LINE TYPES

WATER VALVE

■ UTILITY PEDESTAL

STORM MANHOLE

TELEPHONE MANHOLE

BOLLARD

ELECTRIC BOX

ELECTRIC METER

FIRE HYDRANT

ELECTRIC MANHOLE

CAP

GAS METER

○ LIGHT POLE

POWER POLE

PROPERTY PIN

 \Box BUSH

Ð

OVERHEAD CABLE TV UNDERGROUND ELECTRIC UNDERGROUND TELEPHONE

DATE: MARCH 12, 2018 DRAWN BY: K. ATKINSON CHECKED BY: A. WEHUNT

REVISIONS RICHMOND AS BUILT



Site Development | Residential | Infrastructure | Technology VIRGINIA | NORTH CAROLINA | WEST VIRGINIA THIS DRAWING PREPARED AT THE **DOWNTOWN OFFICE**117 S 14th Street Suite 303 | Richmond, VA 23219
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36157.001

JAMES RIVER PARK SYSTEM WAREHOUSE SITE

IMPROVEMENTS

LANDSCAPE NOTES & DETAILS

DEPARTMENT OF PARKS AND RECREATION

SHEET NO.



INSTALL SHRUBS SO THAT THE TOP OF THE ROOTBALL IS AT THE SAME GRADE AS ORIGINALLY GROWN OR 1-2"

SOILS, DO NOT COVER THE TOP

REMOVE ALL STRING, WIRE, AND

BURLAP FROM TOP 1/3 OF BALL

BACKFILL PLANTING PIT WITH NATIVE SOIL INCORPORATE

SLOW-RELEASE GRANULAR

SCARIFY BOTTOM AND SIDES

FERTILIZER

B&B

CONTAINER

ABOVE IN POOR DRAINING

OF THE ROOTBALL WITH SOIL

SHRUB PLANTING

5

1:1 SLOPE ON SIDES OF PLANTING SCARIEY SIDES BEFORE PLANTING PLACE ROOTBALL ON UNEXCAVATED OR TAMPED SOIL DIAMETER **DECIDUOUS TREE - STAKING SPECIFIED EVERGREEN TREE - STAKING SPECIFIED**

REMOVE ALL DEAD, BROKEN,

2-3" MULCH LAYER, KEEP AWAY

ROVIDE MULCH UP AND OVER

REMOVE CONTAINER, SCARIFY

BRANCHES AT TIME OF

1" COMPOST LAYER

FROM TRUNK

6" SAUCER

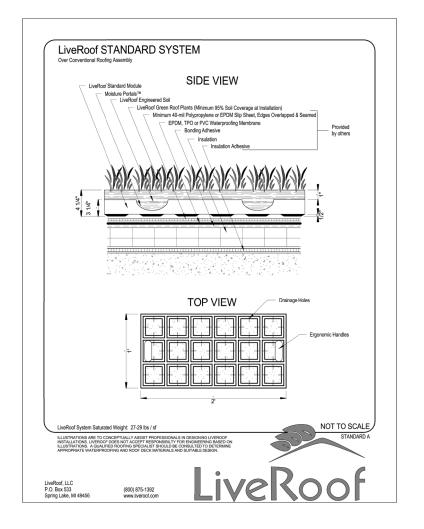
MULCH AS SPECIFIED EXISTING GRADE OR GRADE OF PLANTING AREA MAXIMUM 2:1 SLOPE SHOVEL DUG EDGE LAWN AREA OR

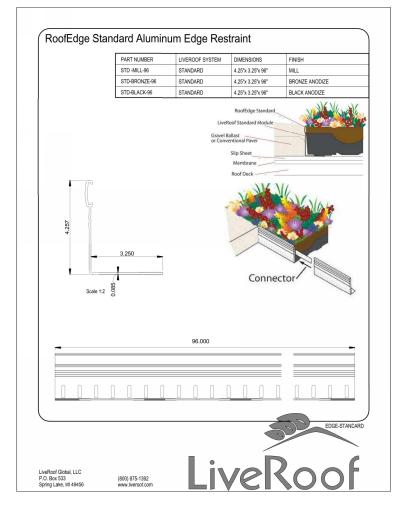
NOTE:

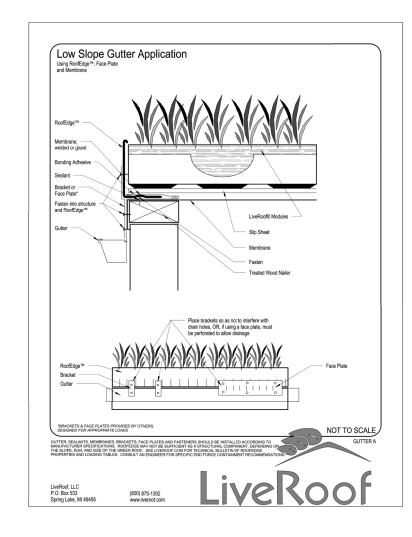
1. TRENCH EDGE DETAIL SHALL BE USED AT ALL LAWN EDGES & AT

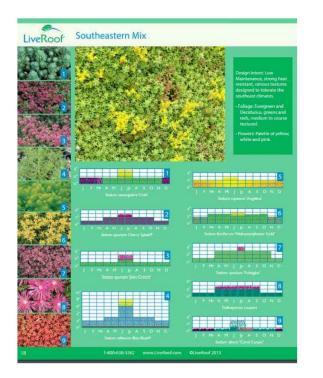
1. INENOH EUGE DE HALL SHALL BE USED AT ALL LAWN EUGES & AT EDGES OF MULCHED AREA (FOR CONTAINMENT).
2. TRENCH EDGE SHALL CREATE A CLEAN SEPARATION BETWEEN AREAS & SHALL CREATE SMOOTH & EVEN LINES (AS INDICATED ON THE PLANS).

LANDSCAPE BED OR MULCH RING TRENCH EDGE

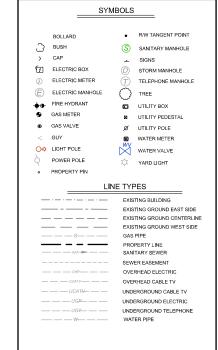












DATE: MARCH 12, 2018 DRAWN BY; K. ATKINSON

SCALE: N/A CHECKED BY; A. WEHUNT

REVISED BER WIS 8, 7/20/2/14





JAMES RIVER PARK SYSTEM WAREHOUSE SITE IMPROVEMENTS

LANDSCAPE NOTES & DETAILS

DEPARTMENT OF PARKS AND RECREATION CITY OF RICHMOND, VIRGINIA

L2.1