



Amelia Street is a RPS school located in Richmond's historically redlined, underserved Randolph neighborhood that serves students ages 5 to 21 years with significant intellectual disabilities, behavioral issues, or complex health needs (which include physical disabilities).

Principal Mark Phillips has a goal of creating partnerships within the community that can bring expertise and resources to his school. Specifically, he is seeking partnerships that support hands-on activities, build functional life skills, highlight the outdoors, and create safe learning environments for his students.

The goals of this project are to improve water quality in the James River and Chesapeake Bay by reducing nutrient and sediment pollution and to promote outdoor learning and environmental literacy through enhanced access to the schoolyard for all students, regardless of ability level. Proposed best management practices (BMPs) include bioretention areas, conservation landscaping, increased native tree canopy, and a permeable, ADA-accessible trail. At the Amelia Street School, the Alliance focused on utilizing stormwater infrastructure as a child-friendly, playful demonstration of how water flows.

The Path Forward at the Amelia Street School will demonstrate that the confluence of accessibility, ecology, safety and education can also be beautiful and engaging.

This project had been funded wholly or in part by the United States Environmental Protection Agency under assistance agreement 4I-95325001, 4I-95303301, and/or C2-96387301.









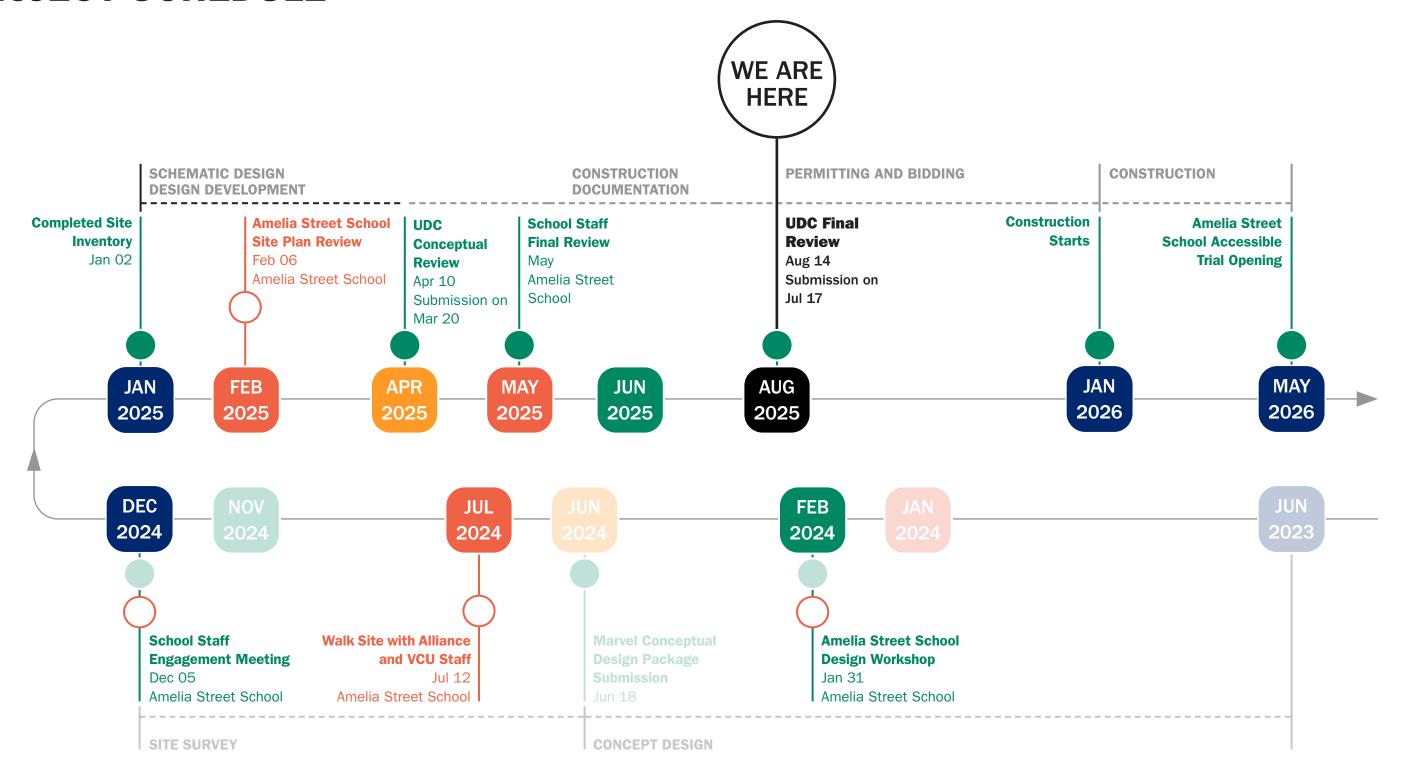








PROJECT SCHEDULE





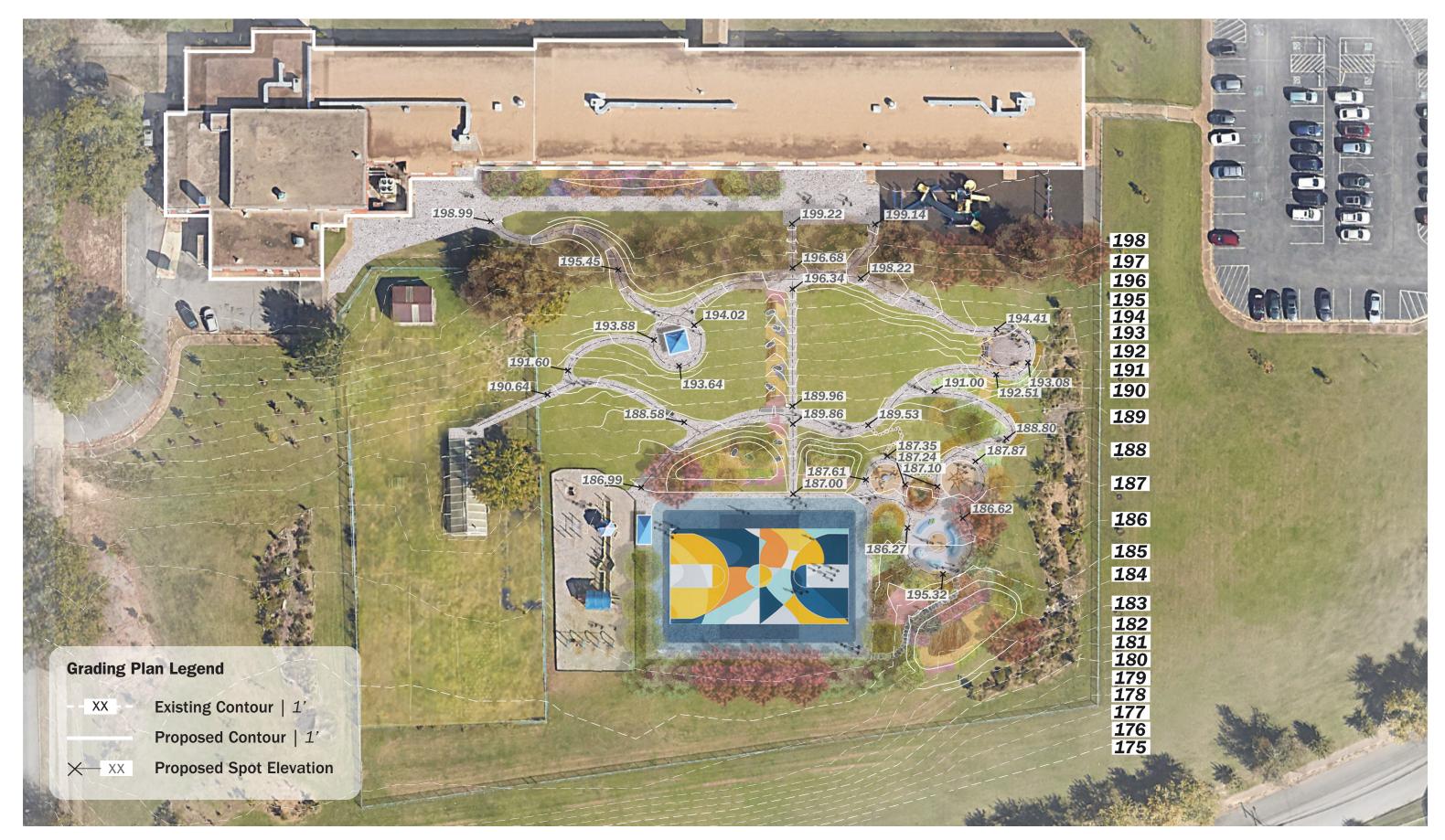




PROPOSED PLAN



PROPOSED GRADING PLAN



CANOPY PLANTING PLAN



UNDERSTORY PLANTING PLAN





PLANT PALETTE - BIORETENTION AREAS







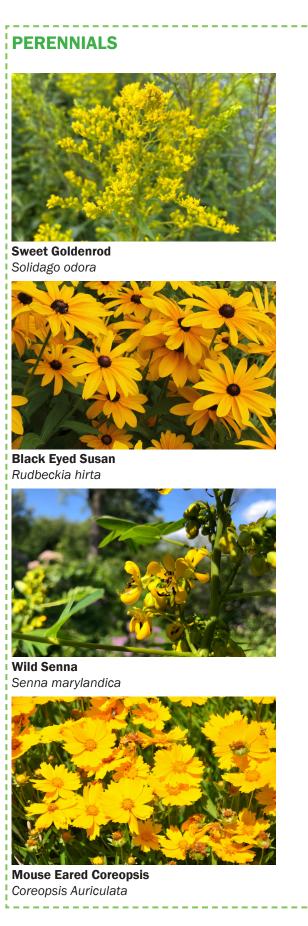
Cornus amomum



8

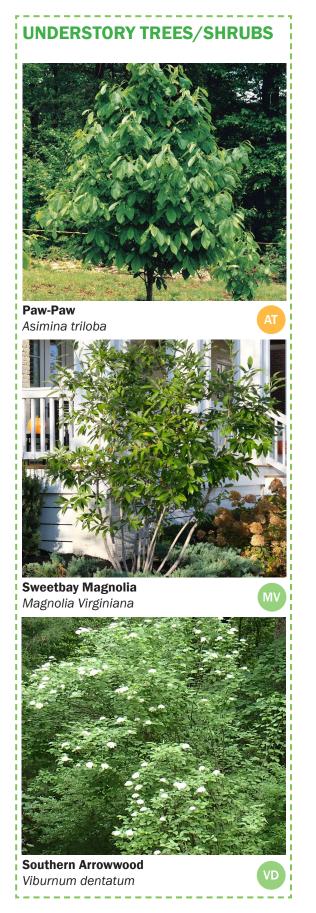
PLANT PALETTE - CONSERVATION AREAS

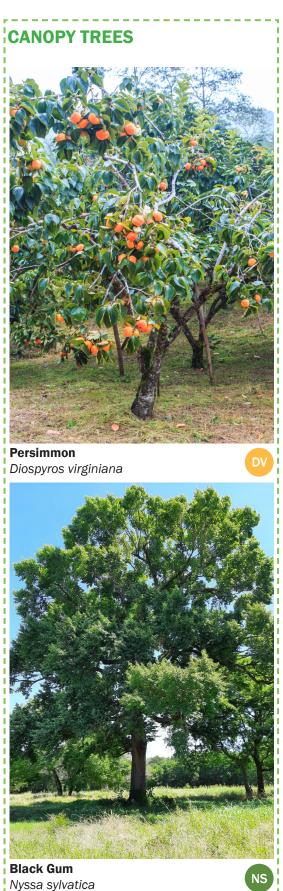






Asclepias tuberosa





PLANTING SCHEDULE

PERENNIAL & GRASSES			
PLANTING GROUP	COMMON NAME	SCIENTIFIC NAME	QUANTITY
PERENNIALS GROUP - YELLOW			700 SF
	Wild Senna	Senna hebecarpa	91
	Sweet Goldenrod	Solidago odora	91
PERENNIALS GROUP - WHITE			1,182 SF
	Rough Blazing Star	Liatris aspera	104
	Spotted Horsemint	Monarda punctata	104
	Beardtongue	Penstemon digitalis	104
PERENNIALS GROUP - PINK			976 SF
	Swamp Milkweed	Asclepias incarnata	126
	Hollow Joe Pye Weed	Eutrochium fistulosum	126
PERENNIALS GROUP - ORANGE			438 SF
	Butterfly Milkweed	Asclepias tuberosa	38
	Lobed Tickseed	Coreopsis auriculata	38
	Black-eyed Susan	Rudbeckia hirta	38
PERENNIALS GROUP - BLUE			580 SF
	New England Aster	Symphyotrichum novae-angliae	74
	New York Aster	Symphyotrichum novi-belgii	74
GRASSES GROUP - PINK			1,220 SF
	Purple Lovegrass	Eragrostis spectabilis	158
	Shenandoah Switch Grass	Panicum virgatum 'Shenandoah'	158
GRASSES GROUP - BROWN			2,504 SF
	Broomsedge Bluestem	Andropogon virginicus	325
	Little Bluestem	Schizachyrium scoparium	325
WILD RYE			2,075 SF
	Virginia Wild Rye	Elymus virginicus	539

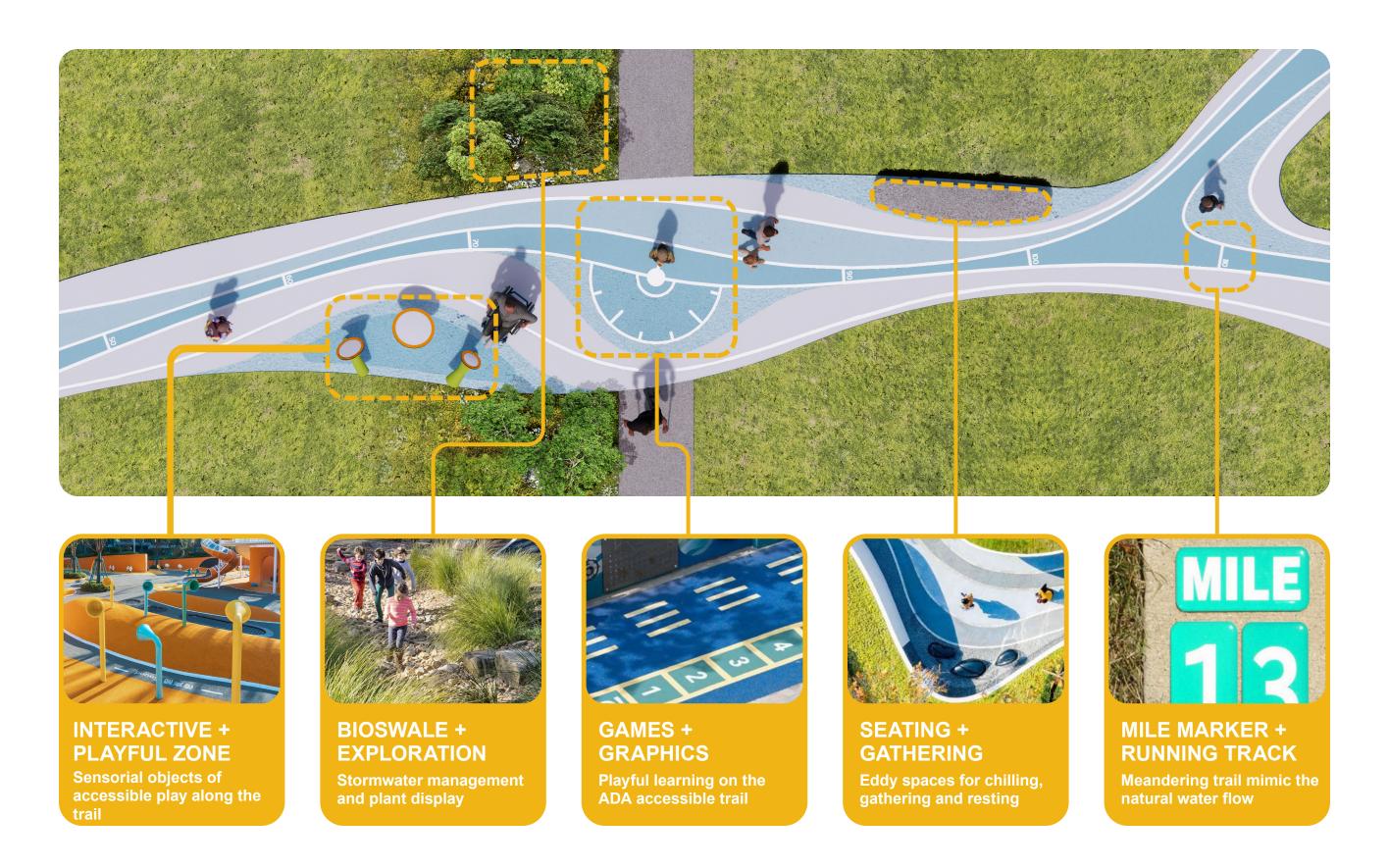
TREE AT Pawpaw Asimina triloba B&B 1.5" Cal. 2 BR River Birch Betula nigra B&B 1.5" Cal. 5 DV Common Persimmon Diospyros virginiana B&B 1.5" Cal. 2	
AT Pawpaw Asimina triloba B&B 1.5" Cal. 2 BR River Birch Betula nigra B&B 1.5" Cal. 5 DV Common Persimmon Diospyros virginiana B&B 1.5" Cal. 2	QUANTITY
BR River Birch Betula nigra B&B 1.5" Cal. S DV Common Persimmon Diospyros virginiana B&B 1.5" Cal. 2	36
DV Common Persimmon Diospyros virginiana B&B 1.5" Cal.	2
The second secon	9
JE Eastern Redcedar Juniperus virginiana B&B 1.5" Cal.	2
	4
MS Sweetbay Magnolia Magnolia virginiana B&B 1.5" Cal.	9
ML Sweet Crabapple Malus coronaria B&B 2" Cal.	1
NS Tupelo Nyssa sylvatica B&B 2" Cal. 9	9
SHRUB	23
CA Silky Dogwood Cornus amomum 180" o.c. 5 gal.	7
MN Northern Bayberry Morella pensylvanica 120" o.c. 3 gal.	6
VD Arrowwood Viburnum Viburnum dentatum 'Arrowwood' 120" o.c. 3 gal.	9



EDUCATIONAL TRAILS AND NODES



EDUCATIONAL TRAIL - UPSTREAM



EDUCATIONAL TRAIL - DOWNSTREAM



EDUCATIONAL TRAIL - BAY AREA



EDUCATIONAL NODES











Climbing Feature



Cozy Dome



Seating Boulders



Stepping Logs



Stepping Features



Sand Pit



Poured Rubber Surface

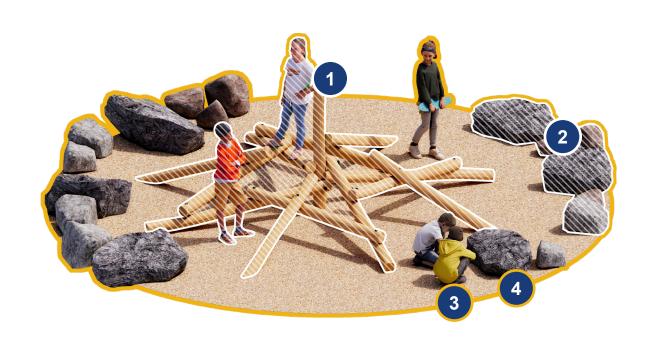


Stepping Stones



Engineered Wood Fiber

EDUCATIONAL NODES





Crawling Structure



Stepping/Seating Stones



Engineered Wood Fiber





Moveable Seating



Stepping Logs



Climbing Structure



Engineered Wood Fiber

FURNISHING AND PLAYFUL ELEMENTS



PLAYFUL ELEMENTS



Interactive Playscape Along Trails



Companion SeatingAlong Trials

Suggested **Elements**



Basketball Hoop On Court



Seating Boulders In Nodes



Stepping Logs In Nodes



Berm In Nodes



Stepping Stone In Nodes



Climbing Wall Along Trials



Educational Signage Along Trails

Optional Elements



Playful Instruments Along Trails



Outdoor Classroom Along Trials



Shade Structure Along Trials



Climbing Structure In Nodes



Movable Seating In Nodes



Stepping Features In Nodes



Crawling Structure In Nodes



Climbing Holds In Nodes



Cozy Dome In Nodes



Soccer Goal On Court



Accent Seating On Court + Along Trails



Bird Houses In Gardens



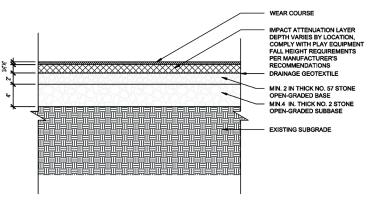
Artwork Installation In Gardens

MATERIAL PLAN



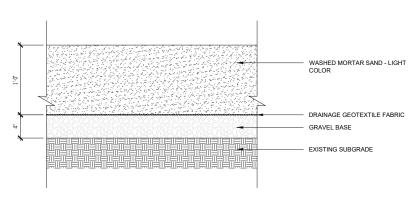
PAVING MATERIAL DETAILS

Rubber Safety Surface



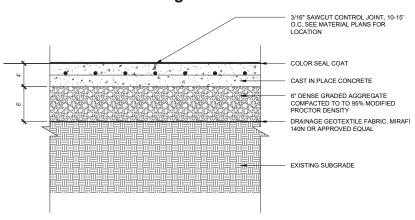


Sand Pit



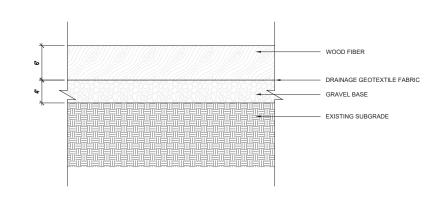


C.I.P. Concrete Paving



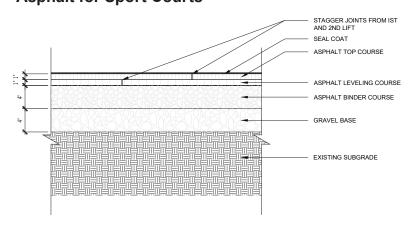


Wood Fiber



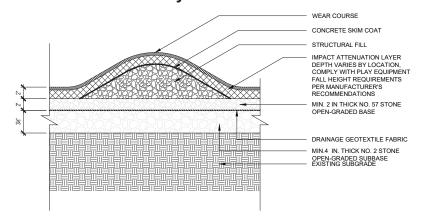


Asphalt for Sport Courts





Rubber Mound Safety Surface

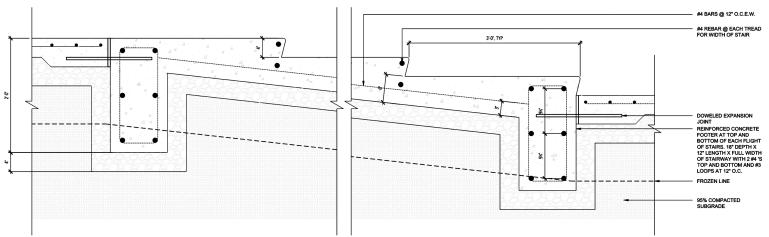


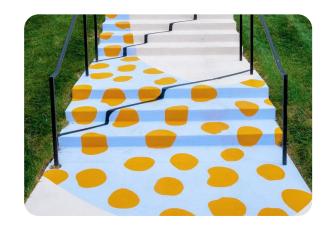




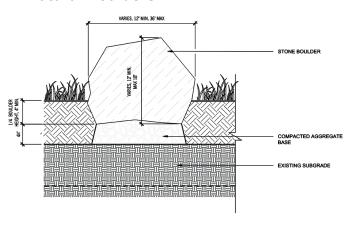
SITE FURNISHING DETAILS

C.I.P. Concrete Stairs



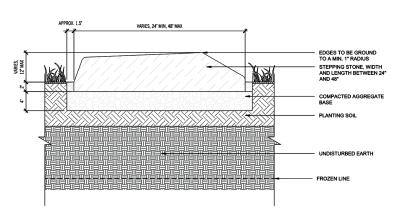


Natural Boulders



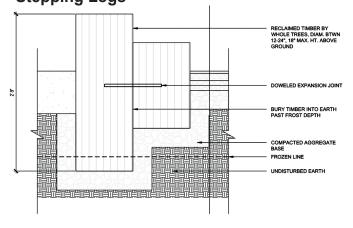


Stepping Stones



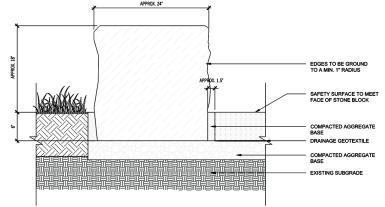


Stepping Logs





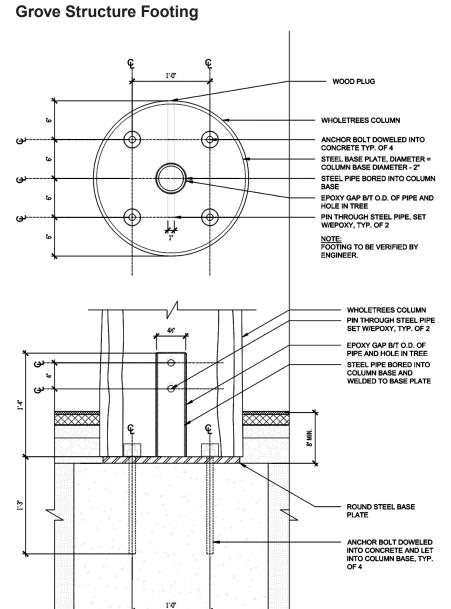
Granite Block Seating





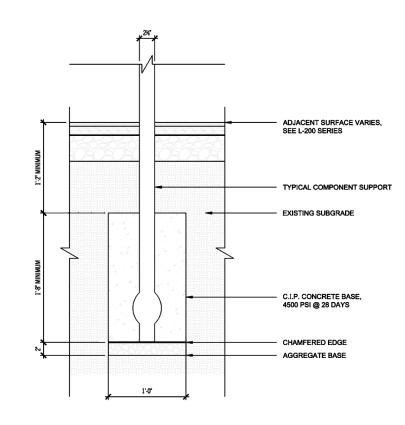


PLAY EQUIPMENT DETAILS





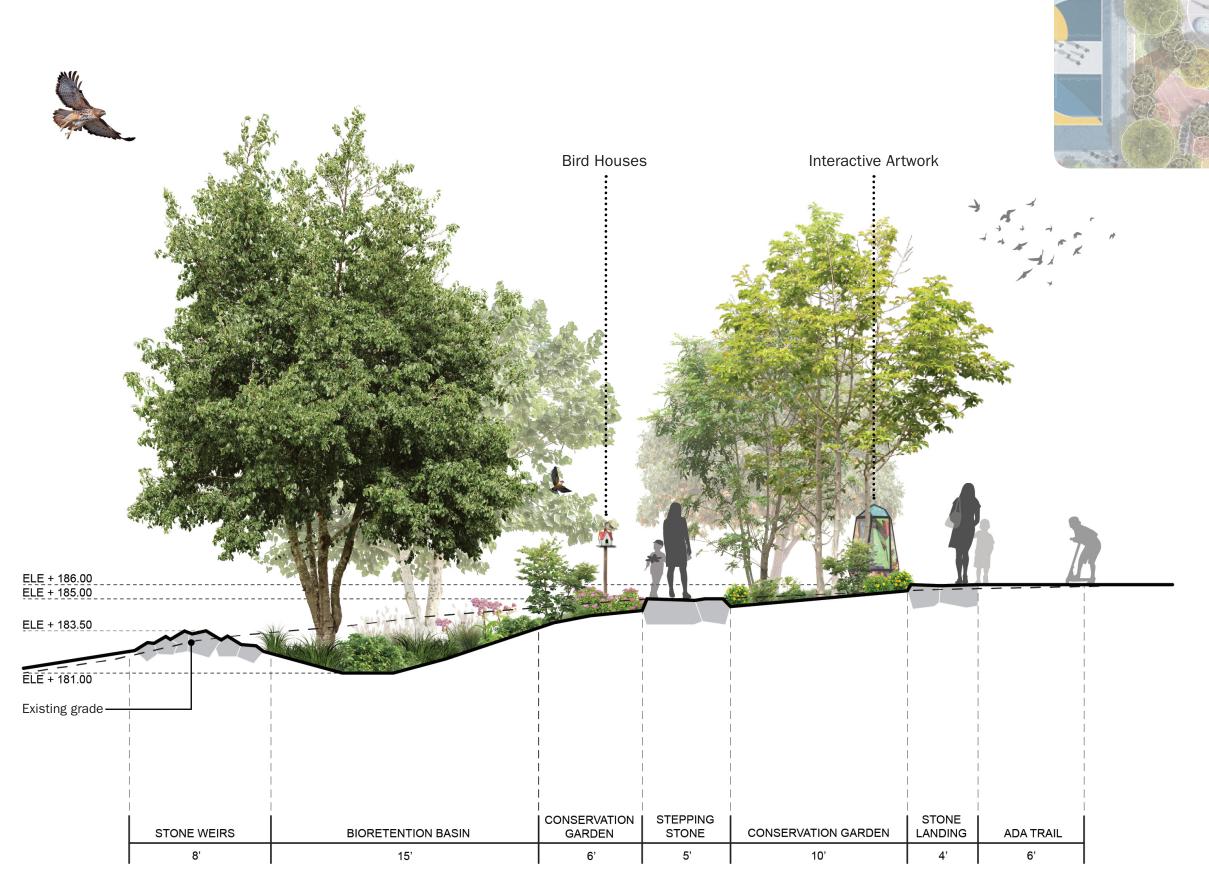
Play Equipment Footing





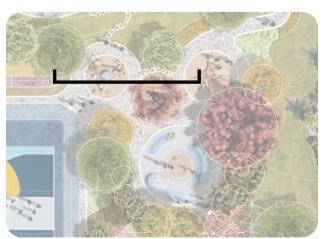
2'-6"

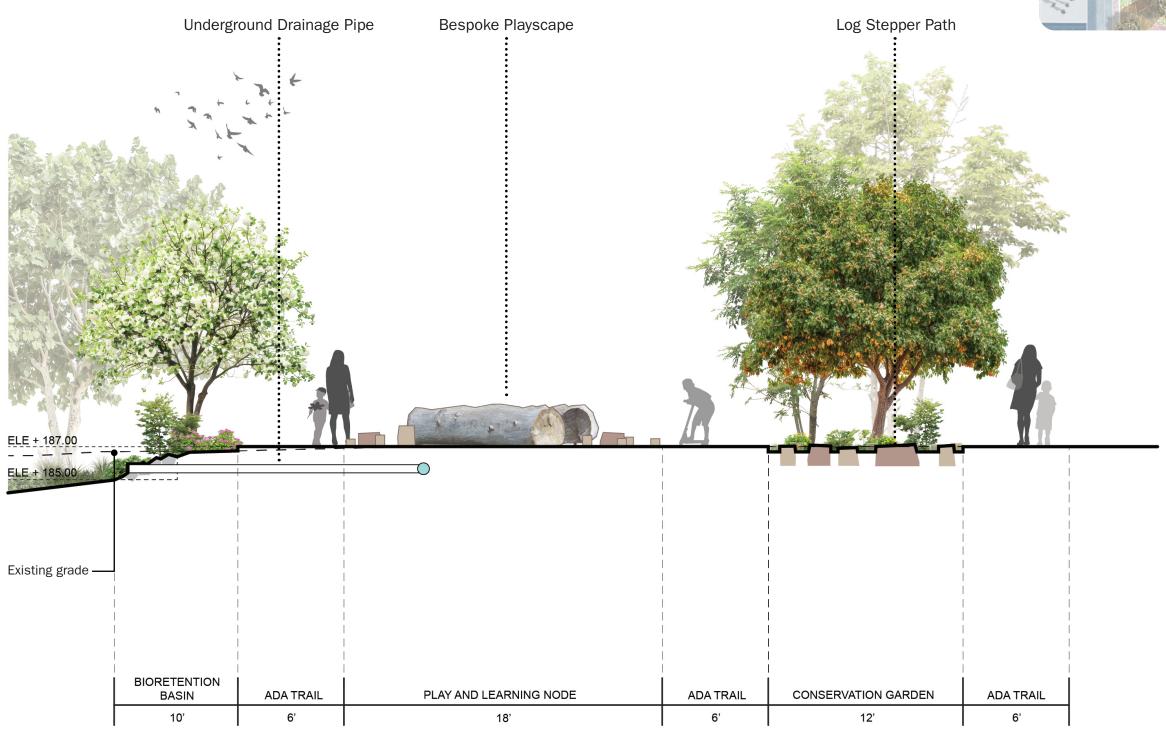
LANDSCAPE SECTION - BIORETENTION





LANDSCAPE SECTION - CONSERVATION





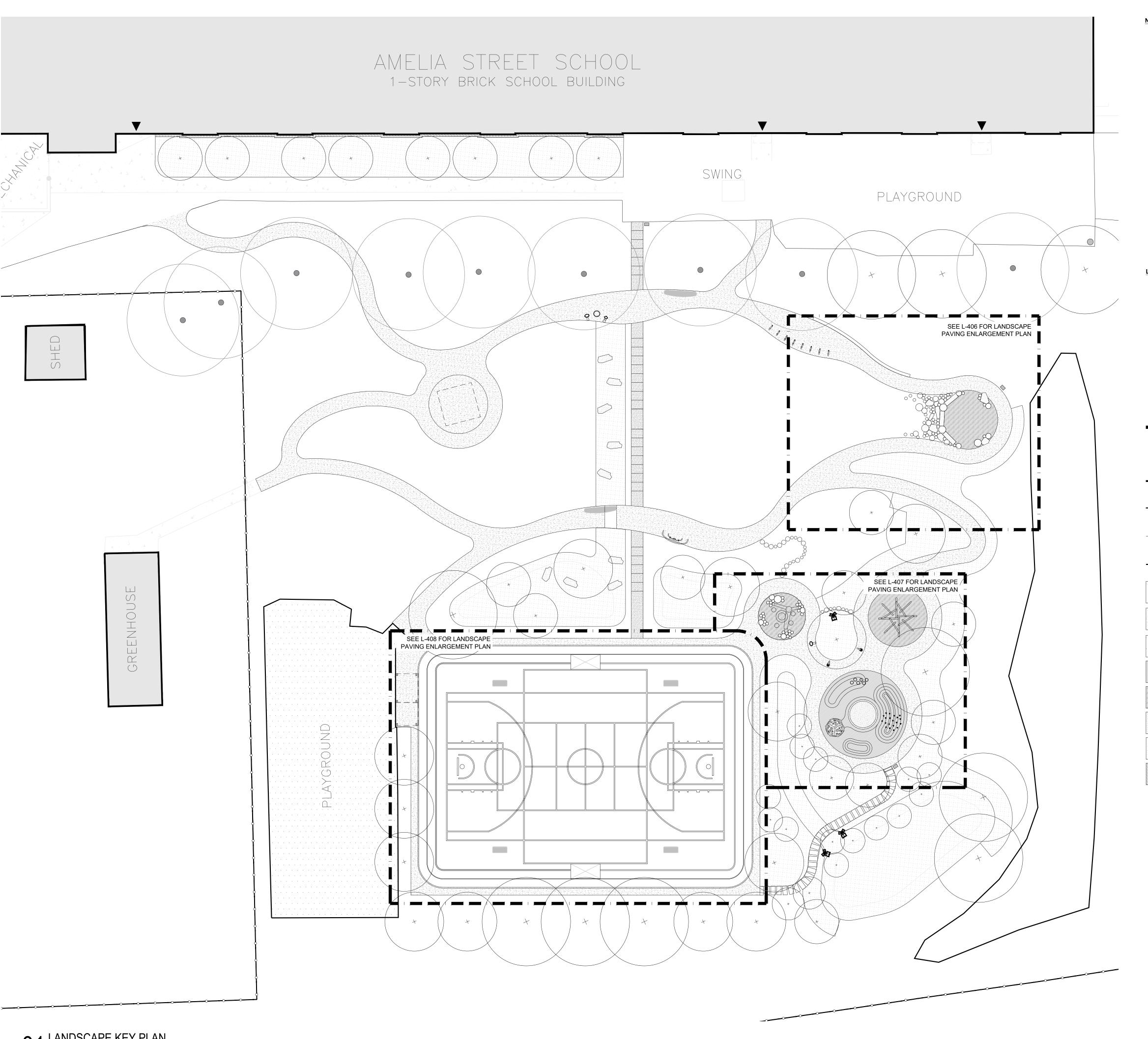






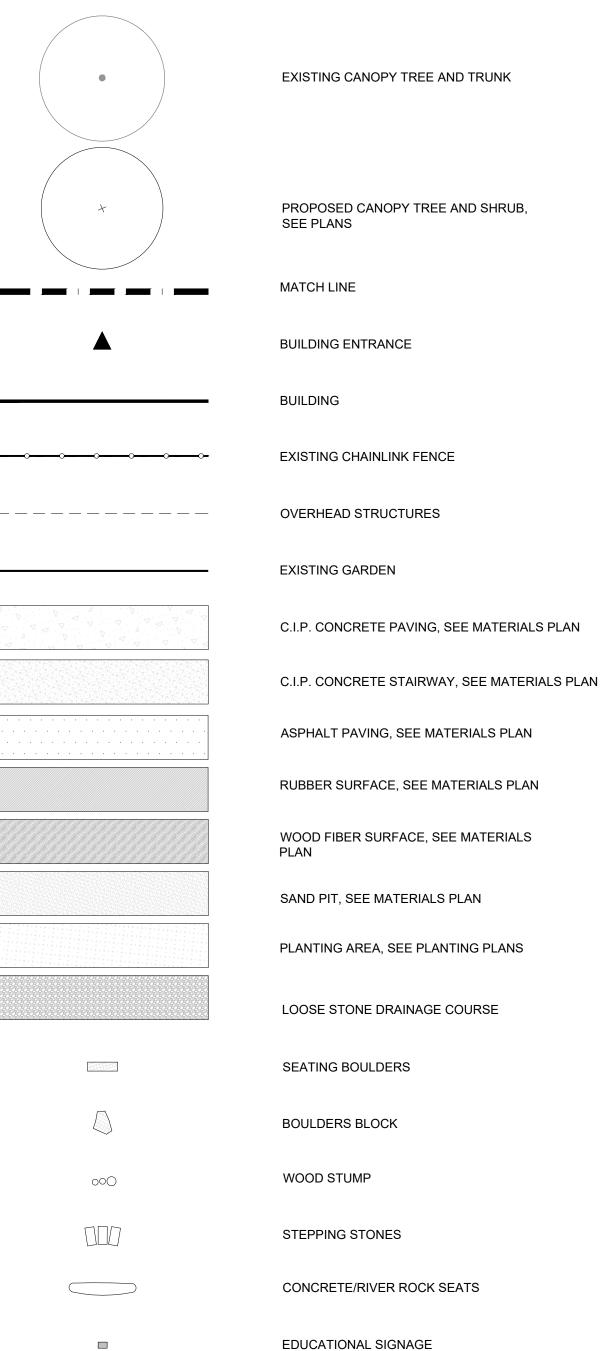






- SEE L-001 FOR NOTES.
 SEE L-100 SERIES FOR LAYOUT.
- 3. SEE L-200 SERIES FOR MATERIALS. 4. SEE L-300 SERIES FOR PLANTING
- 5. SEE L-400 SERIES FOR GRAPHIC. 6. SEE CIVIL DRAWINGS FOR SURFACE GRADING AND
- DRAINAGE. 7. THE CONTRACTOR SHALL VERIFY ALL CONDITIONS BEFORE COMMENCING ANY INSTALLATION. ANY DISCREPANCIES BETWEEN INFORMATION SHOWN ON THE DRAWINGS AND ACTUAL FIELD CONDITIONS SHALL BE BROUGHT TO THE ATTENTION OF THE LANDSCAPE ARCHITECT IN WRITING, PRIOR TO COMMENCING WORK.
- 8. THE CONTRACTOR SHALL EXERCISE EXTREME CAUTION AND CARE, DURING ALL OPERATIONS TO AVOID DISTURBING ADJACENT FACILITIES. ALL DAMAGE RESULTING FROM CONSTRUCTION SHALL BE THE CONTRACTOR'S RESPONSIBILITY AND SHALL BE REPAIRED AT NO EXPENSE TO THE OWNER, OR OTHER DISCIPLINES. ALL REPAIR WORK SHALL BE TO THE SATISFACTION OF THE OWNER AND LANDSCAPE ARCHITECT.
- 9. CONTRACTOR SHALL LAYOUT AND MARK IN THE FIELD THE ALIGNMENT OF ALL PAVERS AND OTHER IMPROVEMENTS FOR REVIEW BY THE LANDSCAPE ARCHITECT. ADJUSTMENTS MAY BE MADE ONLY AS APPROVED BY THE LANDSCAPE ARCHITECT. CONTRACTOR MAY NOT PROCEED WITH CONSTRUCTION OF IMPROVEMENTS UNTIL RECEIVING FINAL APPROVAL OF THE LAYOUT FROM THE LANDSCAPE ARCHITECT.

LEGEND



TRASH RECEPTACLES WITH DOG

WASTE CONTAINER



MARVEL

OWNER/CLIENT ALLIANCE FOR THE CHESAPEAKE BAY

PROJECT MANAGER **NEAL FRIEDMAN**

CIVIL ENGINEERING NITSCH ENGINEERING

SURVEY **Nyfeler Survey**

REV. **07.17.2025 75% CD**

KEY PLAN:NTS

MA PROJECT NO. 2402 **AMELIA STREET SCHOOL**

1821 AMELIA STREET CITY OF RICHMOND, VA

ACCESSIBILITY TRAIL

LANDSCAPE **KEY PLAN**

L-003.00

AMELIA STREET SCHOOL 1-STORY BRICK SCHOOL BUILDING N:3721235.915 E:11781529.967 N:3721233.423 N:3721315.404 N:3721233.995 E:11781356.493 E:11781528.886 E:11781534.584 CURB UNLESS OTHERWISE N:3721299.812 N:3721224.671 N:3721310.029 N:3721231.503 E:11781533.503 PLAYGROUND E:11781353.200 E:11781373.378 E:11781518.832 MATERIALS. N:3721221.220 N:3721219.834 E:11781510.360 E:11781520.097 N:3721264.892 N:3721221.196 N:3721218.074 E:11/781401.402 N:3721317.058 E:11781502.975 E:11781515.172 N:3721237.128 E:11781336.654 N:3721213.758 E:11781406.111 N:3721309,304 E:11781514.777 N:3721237.418 N:3721237.380 E:11781347.820 E:11781416.010 E:11781455.388 N 3721197.248 N:3721294.820 E:11781534.147 N:372/1236.612 E:117/81373.658 N:3721178.906 N:3721266.249 \E:11781430.221 **LEGEND** E:11781396.590 E:11781546.540 N:3721159.926 N:3721154.985 N:3721232.096 N:3721237.46 E:11781571.303 E:11781570.540 E:11781405.633 E:11781366.048 N:3721228.924 N:3721232.502 N:3721218.851 T (S) E:11781401.461 N:3721231.716 E:11781366.635 E:11781487.567 E:11781431,239 N:3721228.637 N:3721229.14 N:3721199.607 N:3721223.788 E:11781397.354 N:3721183.227 E:11781358.22 E:11781518.363 E:11781426.615 E:11781533.642 N:3721233.152 N:3721224.578 N:3721147.557 R10'-0"\ N:3721172.820 E:11781355.230 E:11781468 835 E:11781527.630 E:11781544.069 N:3721143.72 N:3721228.846 N:3721145.102 N:3721150.015 N:3721144.268 E:11781526.731 E:11781351.589 N:3721218.753 E:11781569.013 E:11781471.189 E:11781497.281 R7:6 6. N:3721224.710 N:3721218.295 E:11781412.485 N:3721222.902 N:3721137.901 N:3721143.954 E:11781394.259 E:11781397.492 E:11781339.3 N:3721180.224 E:11781562.**0**74 E:11781478.877 R2'-0" N:3721194.342 E:11781400.904 N:3721134.300 E:11781390.361 N:3721114.219 N:3721171.917 E:11781558.**6**05 E:11781551.684 E:11781420.212 N:3721116.615 N:3721142.765 E:11781556.**d**73 ∕N:3721218.201 E:11781513.861 E:11781339.406 N:3721109.210 N:**\$**721218.851 E:11781558.569 E:11781536.911 E:11781352.447 N:3721108.460 N:3721103.623 N:3721191.899 N:37211 0.066 E:11781536.329 E:11781553.625 E:11781385.998 E:1 781356.029 E:11**78**1499.974 N:3721105.674 N:3721222.371_ E:11781357.653 N:3721144.308 E:11781463.955 N:3721176.367 N:3721109.054 E:11781550.4**9**0 E:11781397.686 E:11781484.465 N:3721107,151 N:3721100.854 N:3721171.284 N:3721159.602 N:3721114.763 E:11781514.966 E:11781549.1**6**5 E:11781396.083 E:11781376.336 E:11781472.783 N:3721166.908 N:3721110.907 N:3721147.810 N:372110Ø.011 E:11781376.651 E:11781380.472 E:11781461.680 E:11781**5**33.717 Z U N:3721159.770 N:37211**9**9.329 E:1178**/**526.041 E:11781362.331 N:3721153.545 _N:3721**/**93.160 N:3721092.112 E:11781365.064 E:11781468.960 E:1178/1520.372 N:3721994:095 E:11781505.383 N:3721151.150 _N:37210**7**8.275 N:3721099.616 E:11781359.924 E:11781**§**18.513 E:11781493 405 _N:3721074.293 E:11781575.315 N:3721082.247 E:11781482 863 _N:3721067\590 E:1178149**0**.472 N:3721126.777 E:11781349.347 N:3721124.711 E:11781354.111 N:3721072.419 N:3721025.310 E:11781331.417 E:11781439.969

NOTES:

- 1. SEE L-001 FOR NOTES. 2. SEE L-100 SERIES FOR LAYOUT.
- SEE L-200 SERIES FOR MATERIALS.
- 4. SEE L-300 SERIES FOR PLANTING. SEE L-400 SERIES FOR GRAPHIC.
- 6. SEE CIVIL DRAWINGS FOR SURFACE GRADING AND DRAINAGE. 7. SEE L-900 SERIES FOR PAVING
- ENLARGEMENTS AND DESIGN. 8. THE CONTRACTOR SHALL BLEND NEW WORK WITH EXISTING CONDITIONS WITH A SMOOTH
- TRANSITION 9. THE CONTRACTOR SHALL PROVIDE AND INSTALL ALL MATERIALS AND PERFORM ALL WORK IN
- ACCORDANCE WITH RECOGNIZED GOOD STANDARDS OF PRACTICE. 10. ALL RADII ARE TO THE FACE OF
- INDICATED. 11. LANDSCAPE ARCHITECT TO APPROVE ALL LAYOUT IN THE FIELD PRIOR TO INSTALLATION OF

EXISTING CANOPY TREE, SEE PLANS

PROPOSED CANOPY TREE AND SHRUB, SEE PLANS

BUILDING ENTRANCE

EXISTING CHAINLINK FENCE

E:X.X COORDINATE POINT N:X.X

CENTER LINE

OVERHEAD STRUCTURES ______

BUILDING

EXISTING GARDEN

SAW CUT

C.I.P. CONCRETE PAVING, SEE MATERIALS PLAN

C.I.P. CONCRETE STAIRWAY, SEE MATERIALS PLAN

ASPHALT PAVING, SEE MATERIALS PLAN

RUBBER SURFACE, SEE MATERIALS PLAN

WOOD FIBER SURFACE, SEE MATERIALS

SAND PIT, SEE MATERIALS PLAN

PLANTING AREA, SEE PLANTING PLANS



MARVEL

OWNER/CLIENT ALLIANCE FOR THE CHESAPEAKE BAY

PROJECT MANAGER **NEAL FRIEDMAN**

CIVIL ENGINEERING **NITSCH ENGINEERING**

SURVEY **NYFELER SURVEY**

REV. **07.17.2025 75% CD**

KEY PLAN:NTS

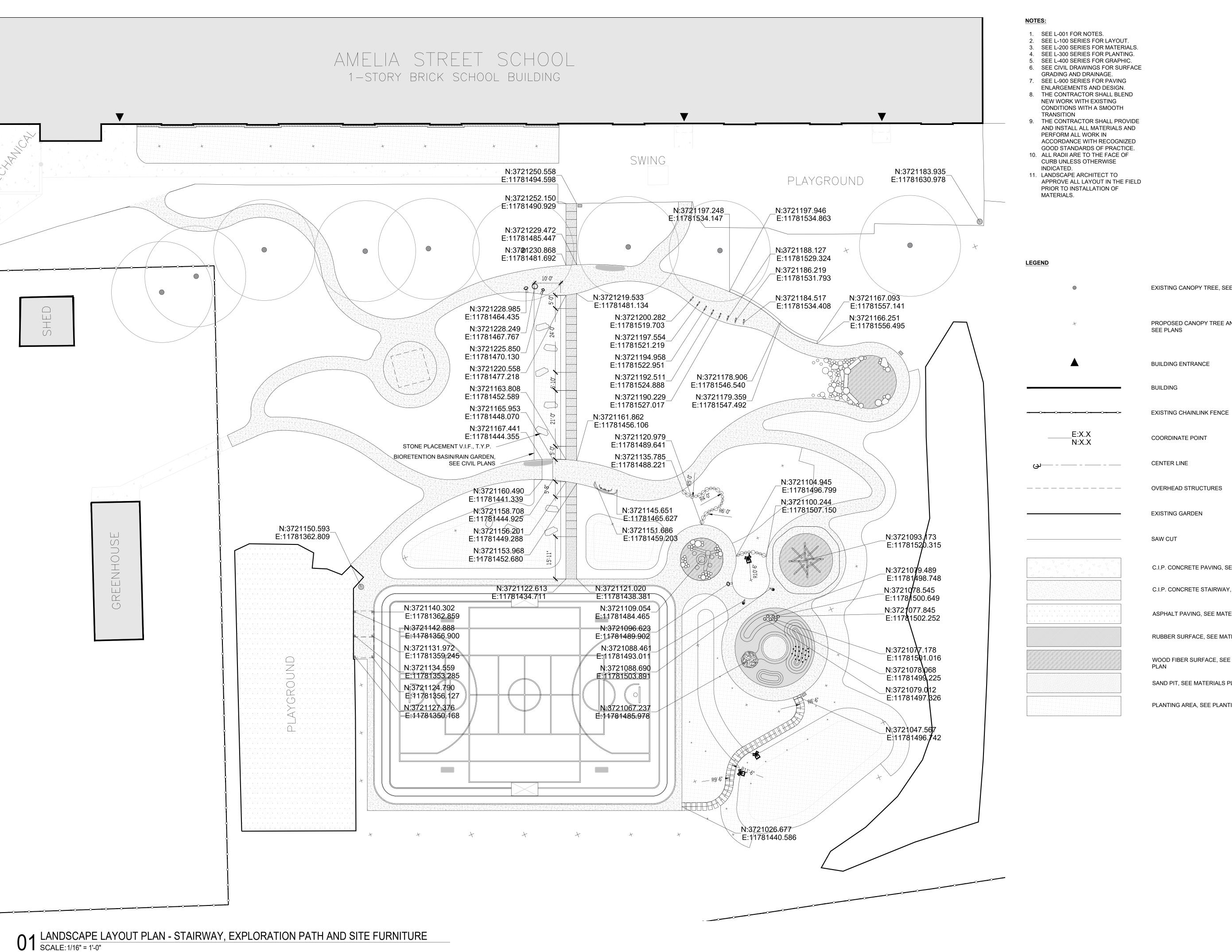
MA PROJECT NO. 2402

AMELIA STREET SCHOOL ACCESSIBILITY TRAIL

1821 AMELIA STREET CITY OF RICHMOND, VA

LANDSCAPE LAYOUT PLAN

L-101.00



EXISTING CANOPY TREE, SEE PLANS

PROPOSED CANOPY TREE AND SHRUB, SEE PLANS

BUILDING ENTRANCE

OVERHEAD STRUCTURES

C.I.P. CONCRETE PAVING, SEE MATERIALS PLAN

C.I.P. CONCRETE STAIRWAY, SEE MATERIALS PLAN

ASPHALT PAVING, SEE MATERIALS PLAN

RUBBER SURFACE, SEE MATERIALS PLAN

WOOD FIBER SURFACE, SEE MATERIALS

SAND PIT, SEE MATERIALS PLAN

PLANTING AREA, SEE PLANTING PLANS

LANDSCAPE LAYOUT PLAN

KEY PLAN:NTS

MA PROJECT NO. 2402

1821 AMELIA STREET

CITY OF RICHMOND, VA

AMELIA STREET SCHOOL

ACCESSIBILITY TRAIL

L-102.00

REV. **07.17.2025 75% CD**

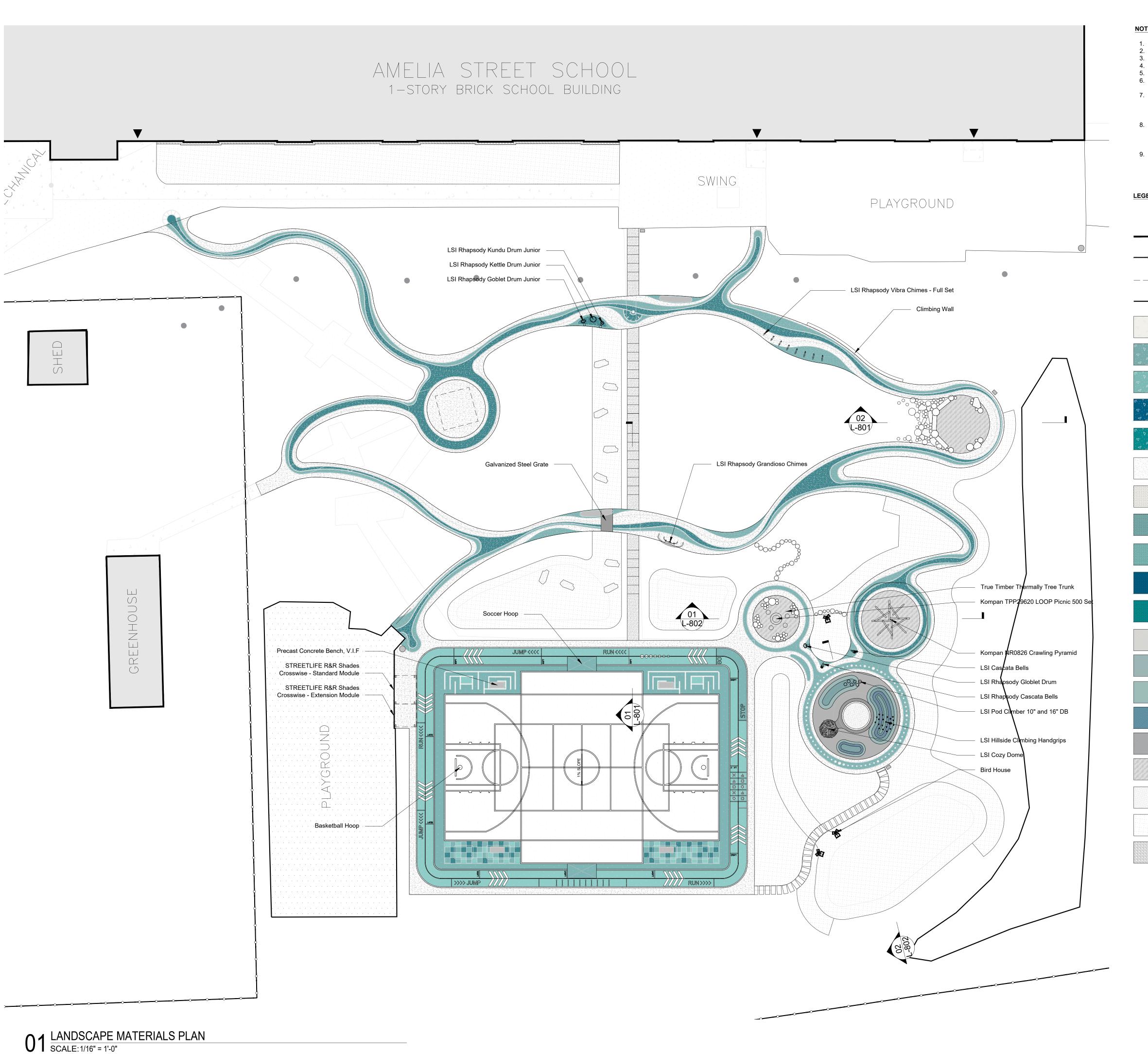
MARVEL

OWNER/CLIENT ALLIANCE FOR THE CHESAPEAKE BAY

PROJECT MANAGER **NEAL FRIEDMAN**

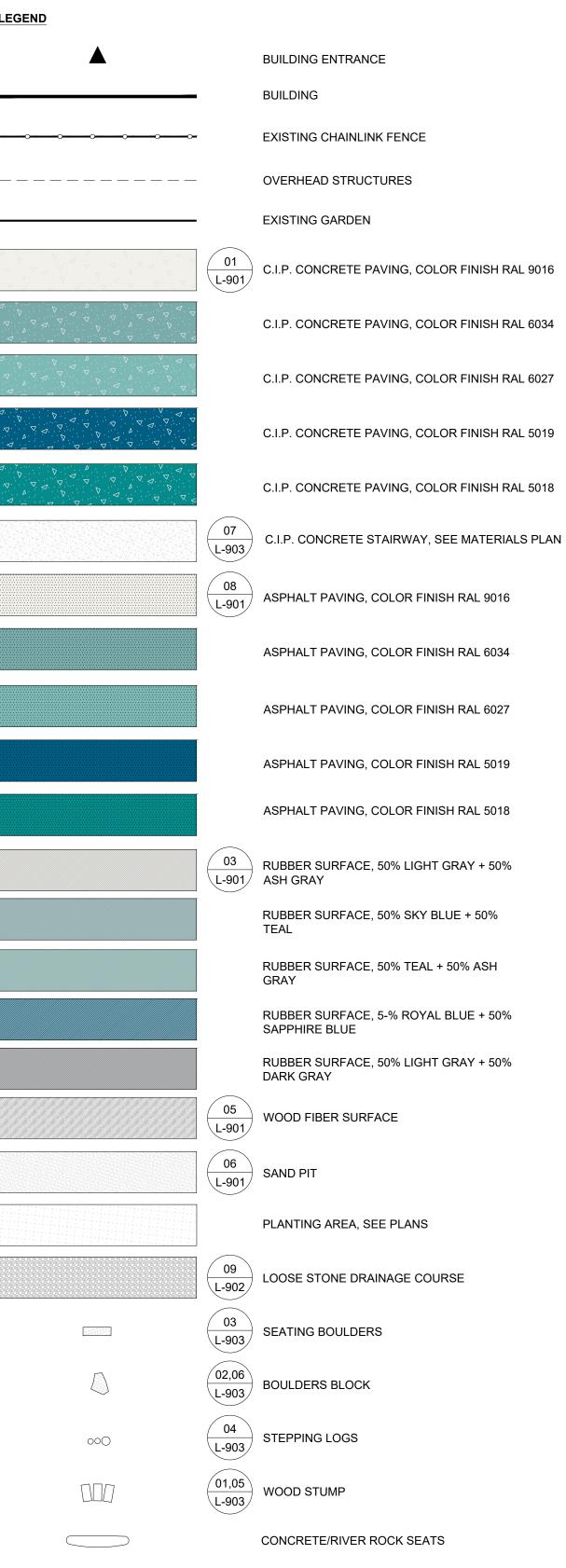
SURVEY **NYFELER SURVEY**

CIVIL ENGINEERING **NITSCH ENGINEERING**



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 SEE L-100 SERIES FOR LAYOUT.
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- 6. ALL PROPOSED TOP OF WALL AND BOTTOM OF WALL DATA TAKEN FROM CIVIL DWGS. SEE CIVIL DRAWINGS FOR SURFACE GRADING AND DRAINAGE.
- 7. THE CONTRACTOR SHALL VERIFY ALL EXISTING TOPOGRAPHIC INFORMATION, ANY DISCREPANCIES BETWEEN THE SURVEY AND EXISTING CONDITIONS, BETWEEN PLANS AND SPECIFICATIONS, OR BETWEEN DIFFERENT PLANS SHALL BE BROUGHT TO THE ATTENTION OF THE LANDSCAPE ARCHITECT IN WRITING PRIOR TO COMMENCING WORK.
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LEGEND



EDUCATIONAL SIGNAGE

WASTE CONTAINER

TRASH RECEPTACLES WITH DOG



MARVEL

OWNER/CLIENT ALLIANCE FOR THE CHESAPEAKE BAY

PROJECT MANAGER **NEAL FRIEDMAN**

CIVIL ENGINEERING NITSCH ENGINEERING

SURVEY **NYFELER SURVEY**

REV. **07.17.2025 75% CD**

KEY PLAN:NTS

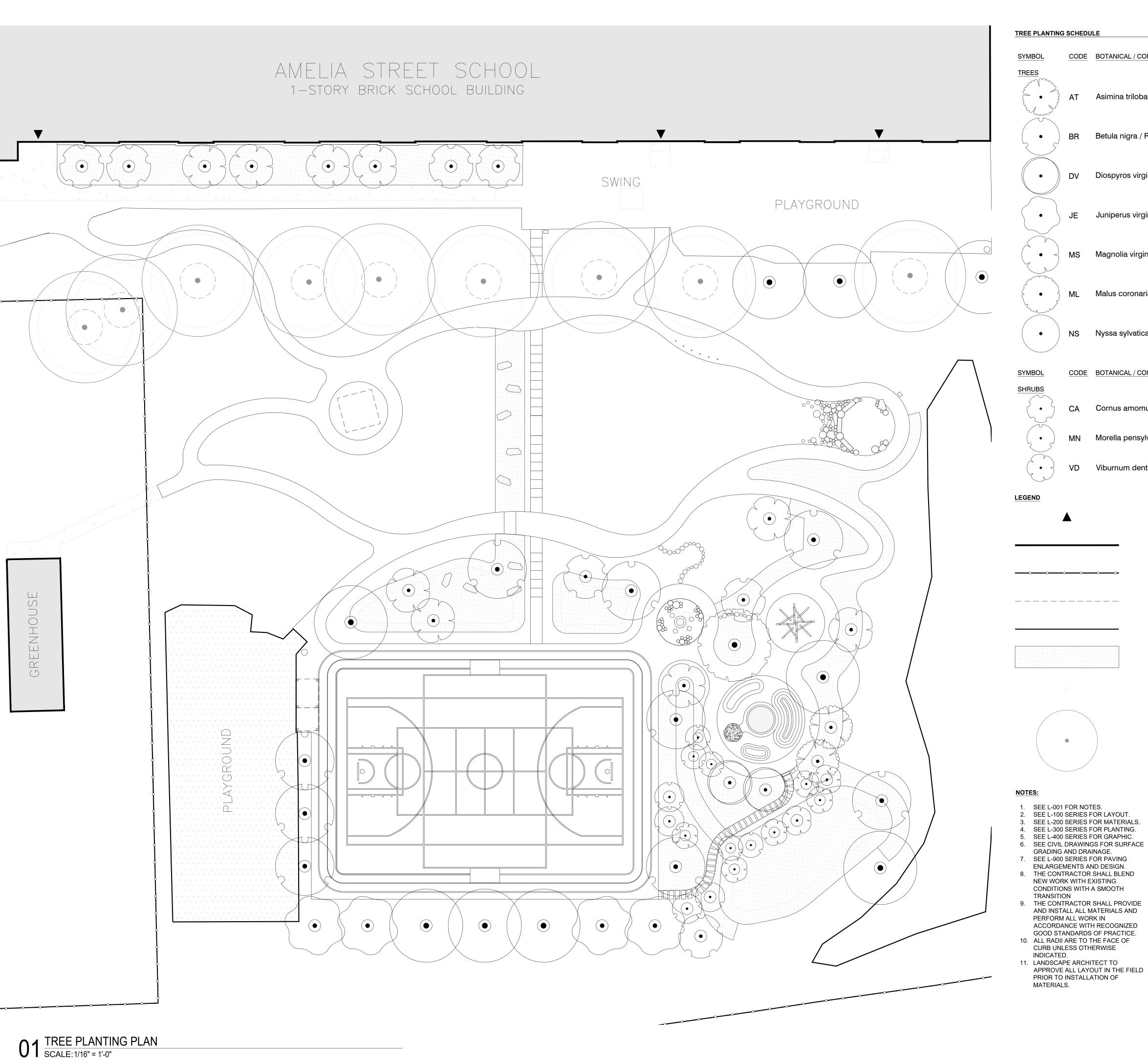
MA PROJECT NO. 2402 **AMELIA STREET SCHOOL**

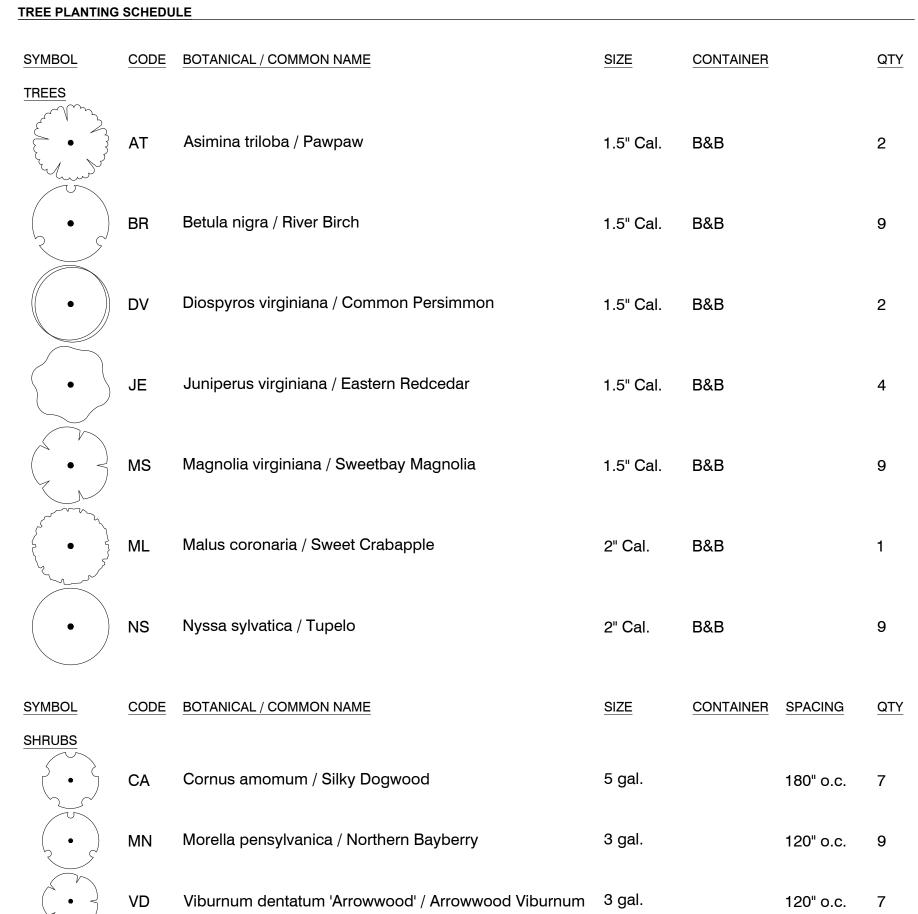
1821 AMELIA STREET CITY OF RICHMOND, VA

ACCESSIBILITY TRAIL

LANDSCAPE **MATERIALS PLAN**

L-201.00





BUILDING ENTRANCE

EXISTING CHAINLINK FENCE

OVERHEAD STRUCTURES

PLANTING AREA, SEE PLANS

PROPOSED TREES/SHRUBS AND ROOT

EXISTING GARDEN

EXISTING TREE

BUILDING

REV. **07.17.2025 75% CD**

MARVEL

OWNER/CLIENT ALLIANCE FOR THE CHESAPEAKE BAY

PROJECT MANAGER **NEAL FRIEDMAN**

SURVEY **NYFELER SURVEY**

CIVIL ENGINEERING NITSCH ENGINEERING

KEY PLAN:NTS

MA PROJECT NO. 2402

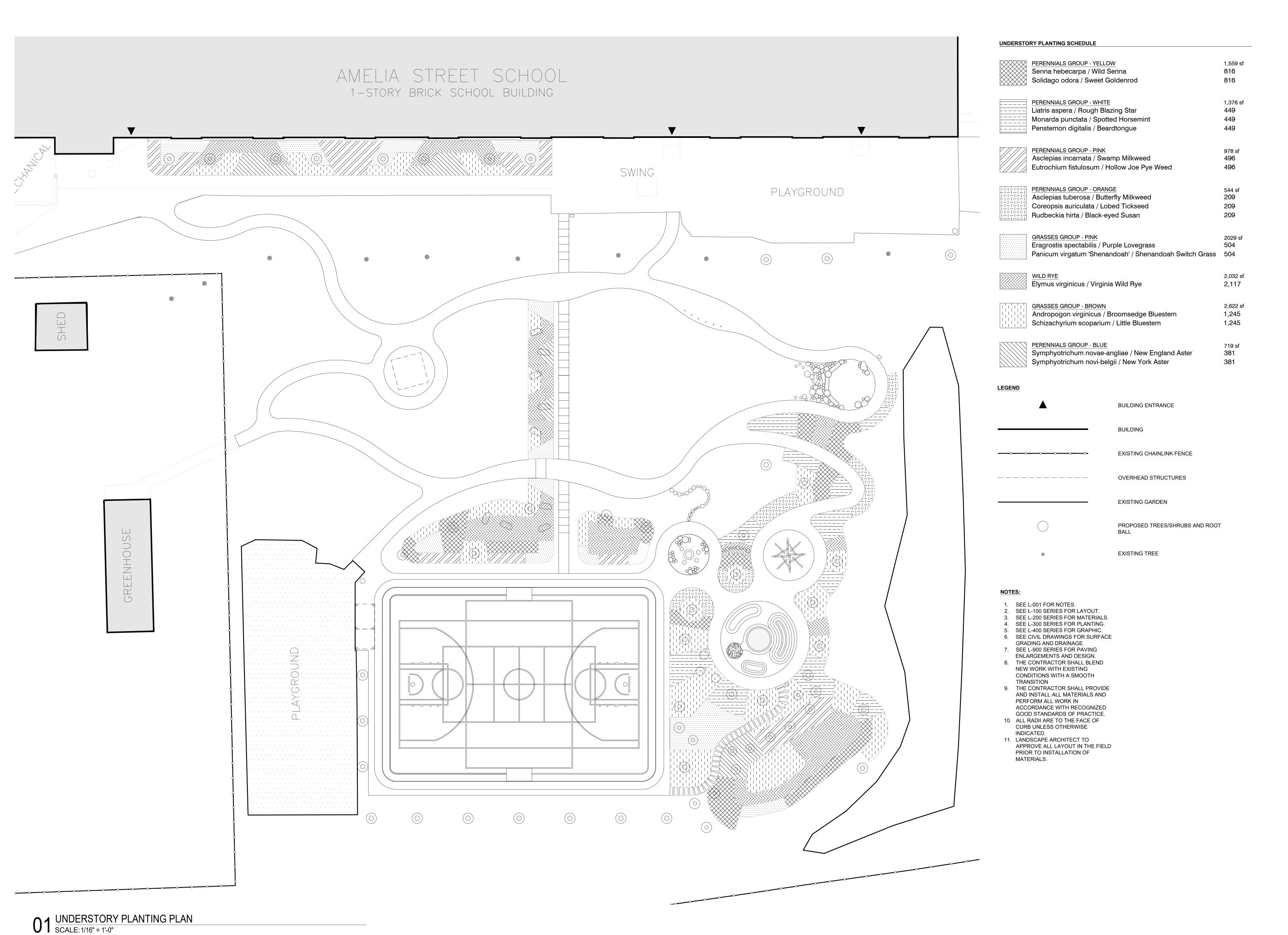
AMELIA STREET SCHOOL

1821 AMELIA STREET CITY OF RICHMOND, VA

ACCESSIBILITY TRAIL

LANDSCAPE
PLANTING PLAN CANOPY

L-301.00





145 HUDSON STREET, FLR.3 NEW YORK, NY 10013 212.616.0420

OWNER/CLIENT ALLIANCE FOR THE CHESAPEAKE BAY

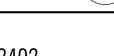
PROJECT MANAGER **NEAL FRIEDMAN**

CIVIL ENGINEERING NITSCH ENGINEERING

SURVEY **Nyfeler Survey**

REV. **07.17.2025 75% CD**

KEY PLAN:NTS



MA PROJECT NO. 2402

AMELIA STREET SCHOOL

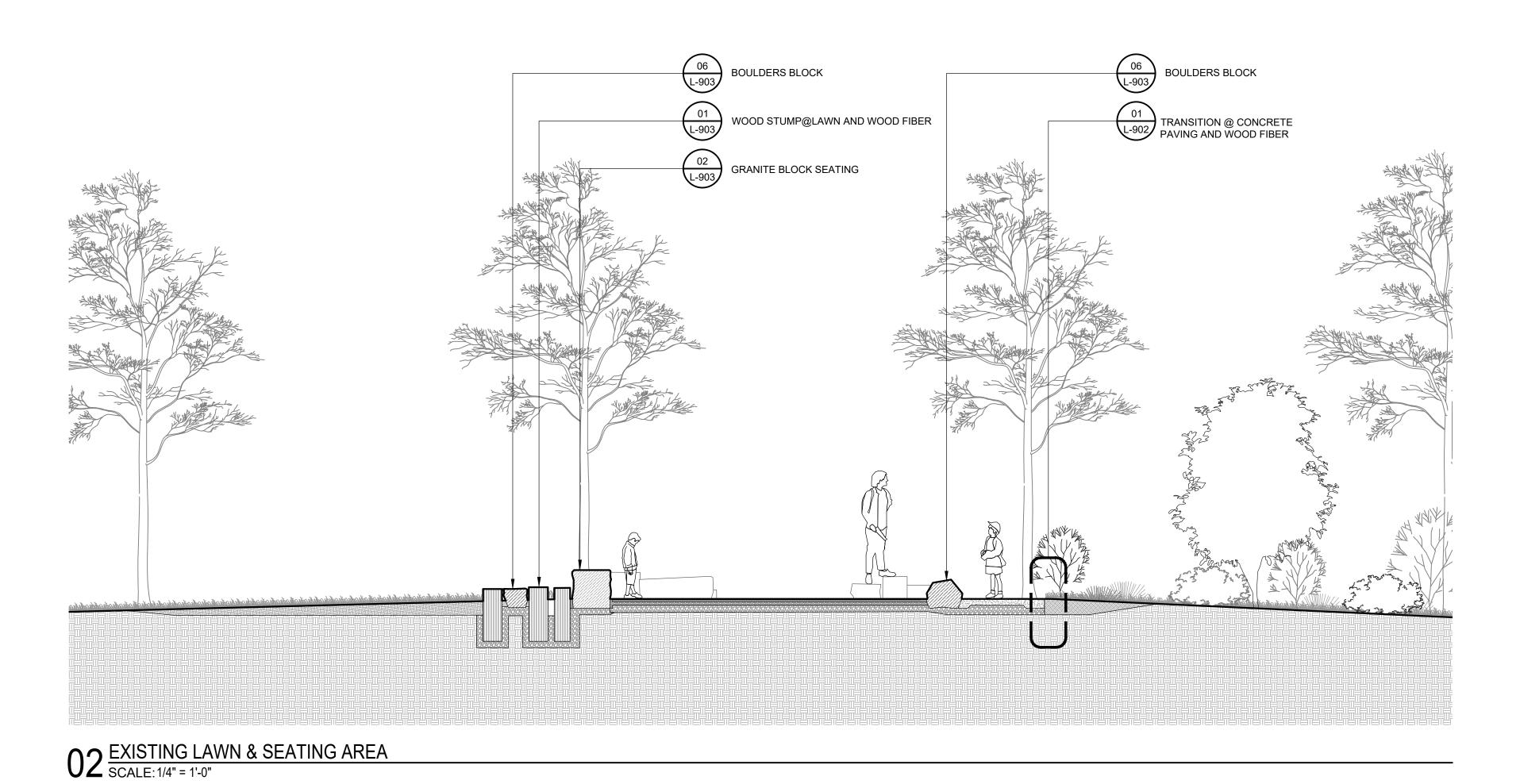
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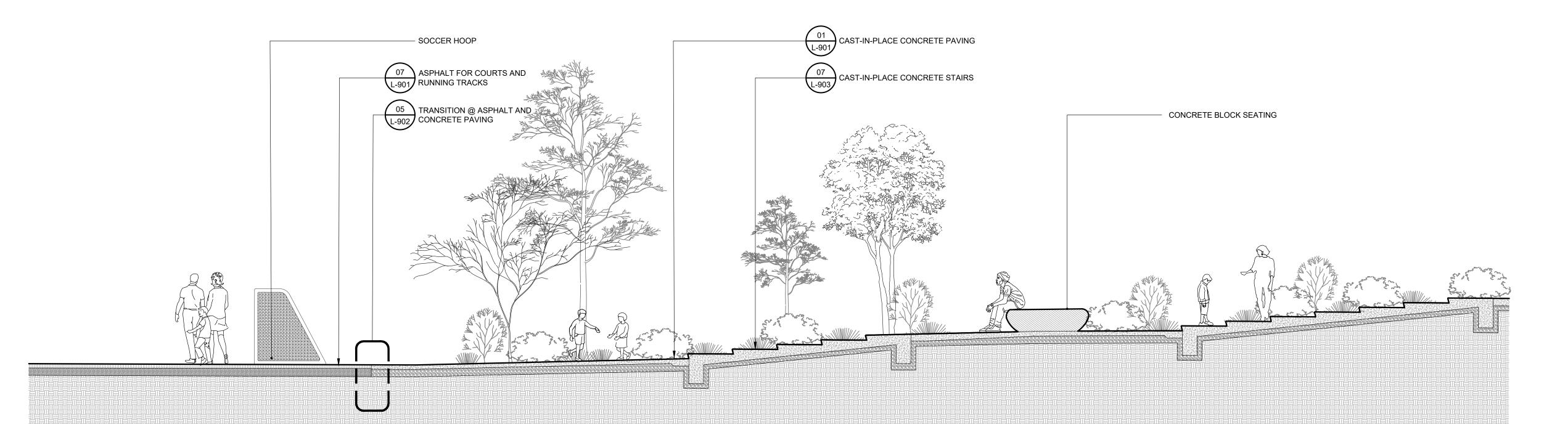
1821 AMELIA STREET CITY OF RICHMOND, VA

LANDSCAPE
PLANTING PLAN UNDERSTORY

L-302.00

03.04.2025 © MARVEL ARCHITECTS, PLLC 2025





145 HUDSON STREET, FLR.3 NEW YORK, NY 10013 212.616.0420

 ${\tt OWNER/CLIENT} \ \ \textbf{Alliance for the chesapeake bay}$

PROJECT MANAGER **NEAL FRIEDMAN**

CIVIL ENGINEERING NITSCH ENGINEERING

SURVEY **Nyfeler Survey**

REV. **07.17.2025 75% CD**

KEY PLAN:NTS

MA PROJECT NO. 2402

AMELIA STREET SCHOOL

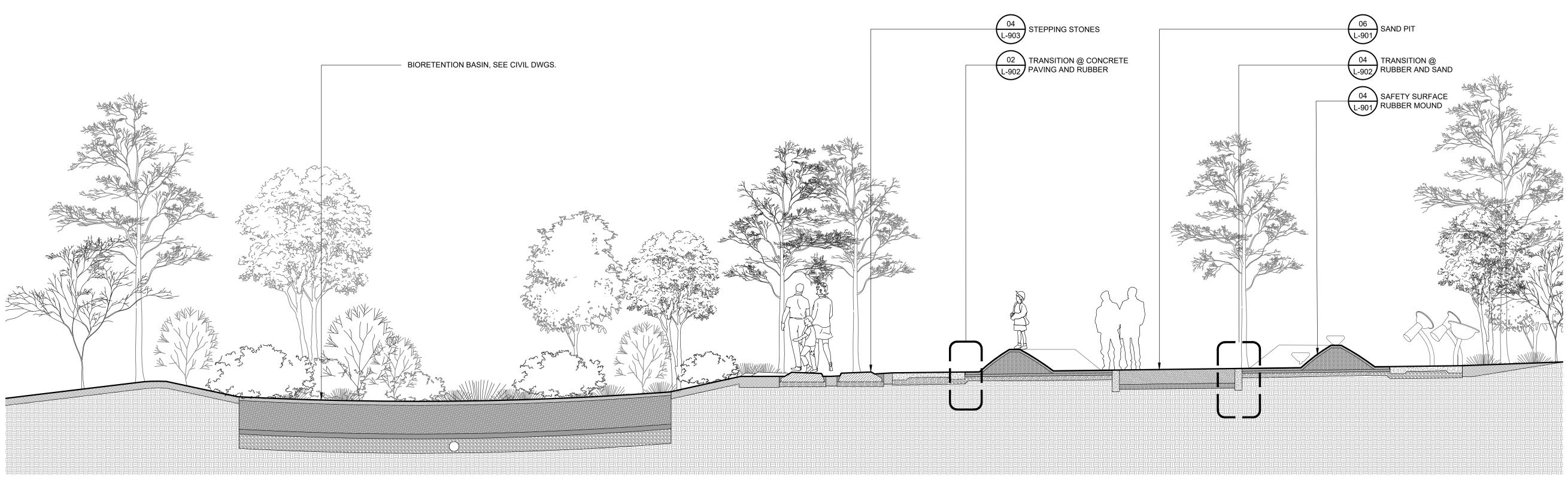
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1821 AMELIA STREET CITY OF RICHMOND, VA

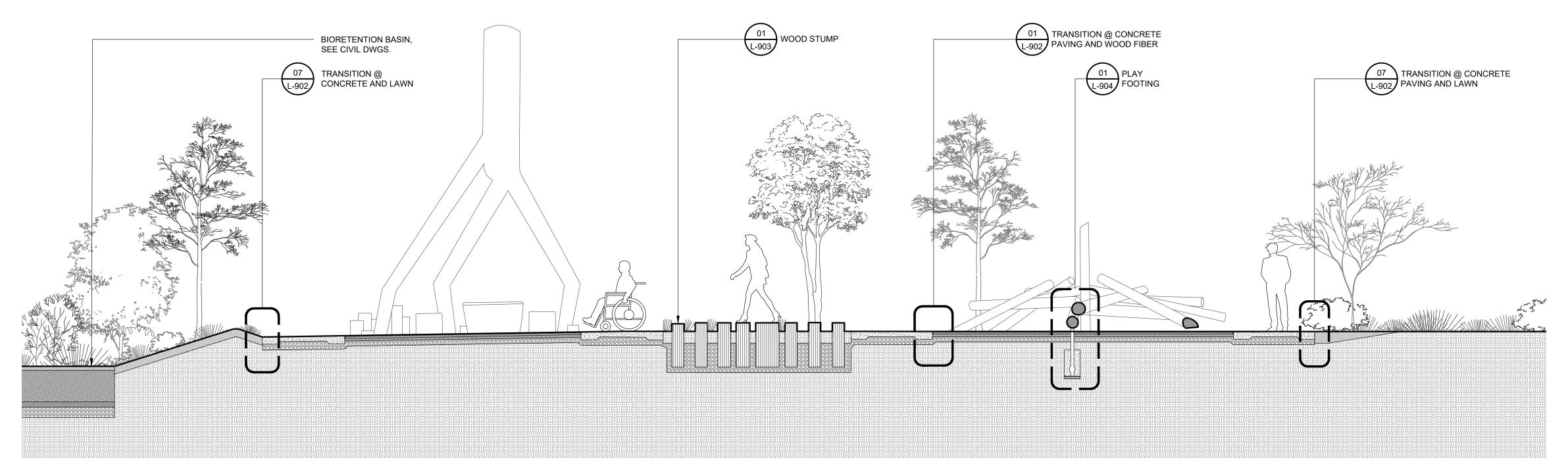
LANDSCAPE SECTIONS

L-801.00

01 STAIRWAY DOWN TO PLAY COURT SCALE: 1/4" = 1'-0"



02 BIORENTION BASIN & CLIMBING BERM SCALE: 1/4" = 1'-0"



145 HUDSON STREET, FLR.3 NEW YORK, NY 10013 212.616.0420

 ${\tt OWNER/CLIENT} \ \ \textbf{ALLIANCE FOR THE CHESAPEAKE BAY}$

PROJECT MANAGER **NEAL FRIEDMAN**

CIVIL ENGINEERING NITSCH ENGINEERING

SURVEY **Nyfeler Survey**

REV. **07.17.2025 75% CD**

KEY PLAN:NTS

MA PROJECT NO. 2402

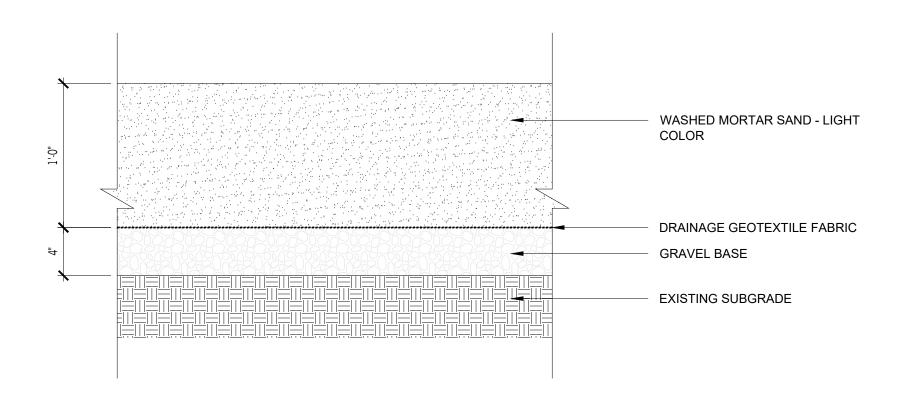
AMELIA STREET SCHOOL

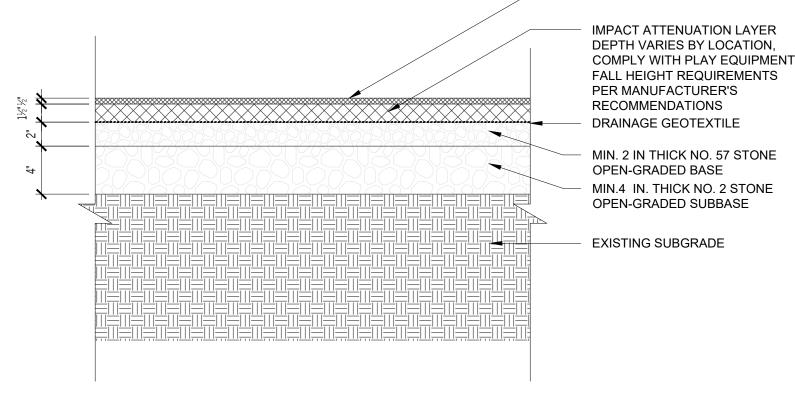
1821 AMELIA STREET CITY OF RICHMOND, VA

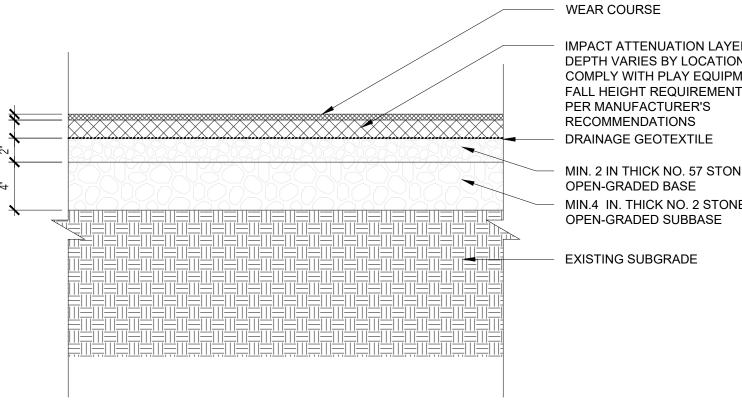
ACCESSIBILITY TRAIL

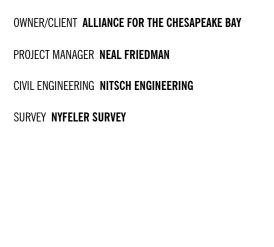
LANDSCAPE SECTIONS

L-802.00



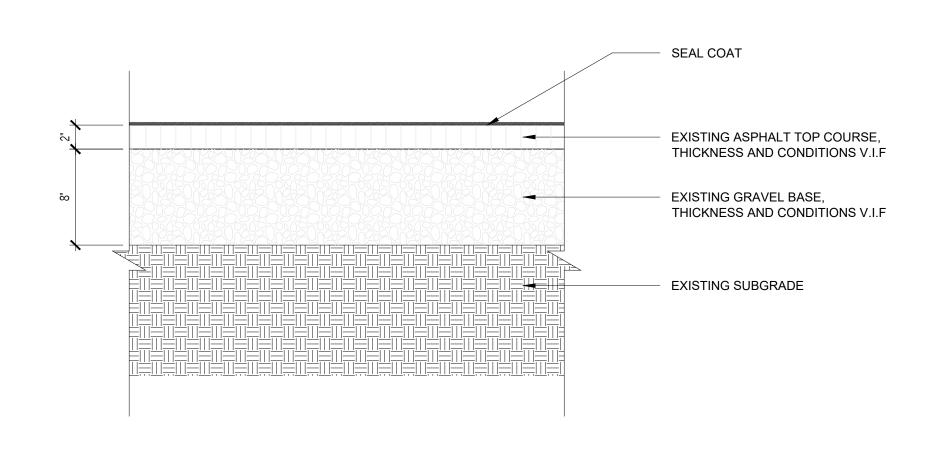


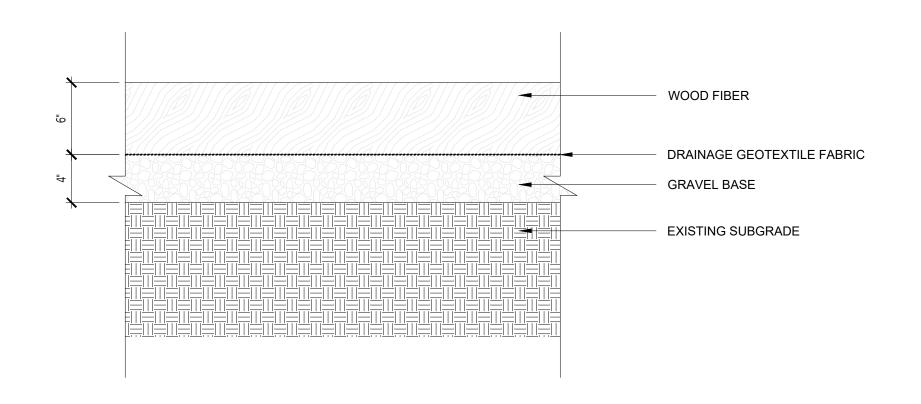




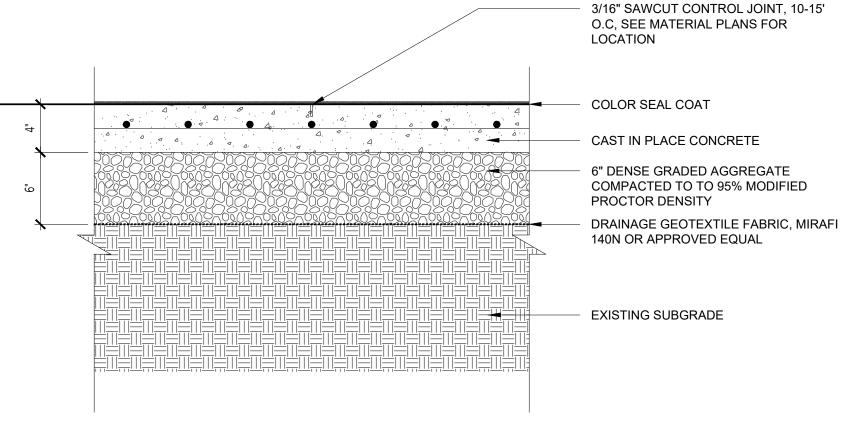
MARVEL

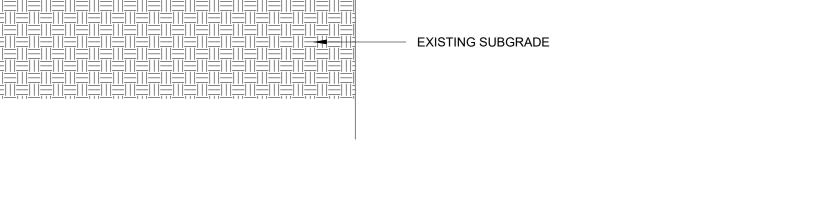
 $06 \frac{\text{SAND PIT}}{\text{SCALE: } 1 \cdot 1/2"=1"}$ O3 SAFETY SURFACE - RUBBER SCALE: 1-1/2"=1'



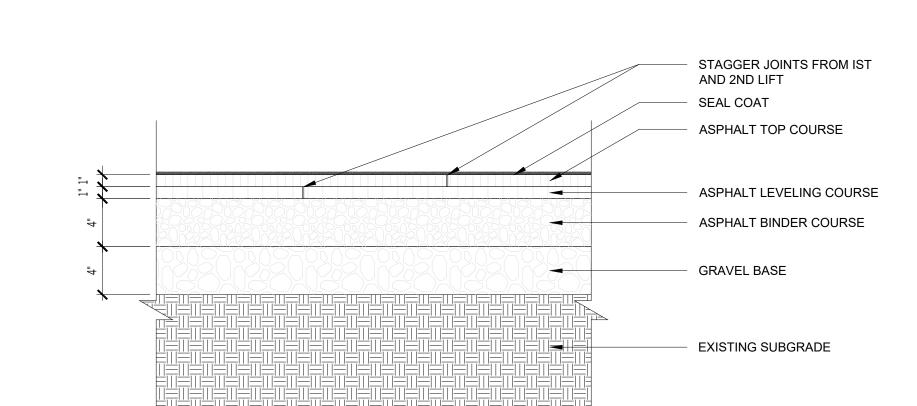


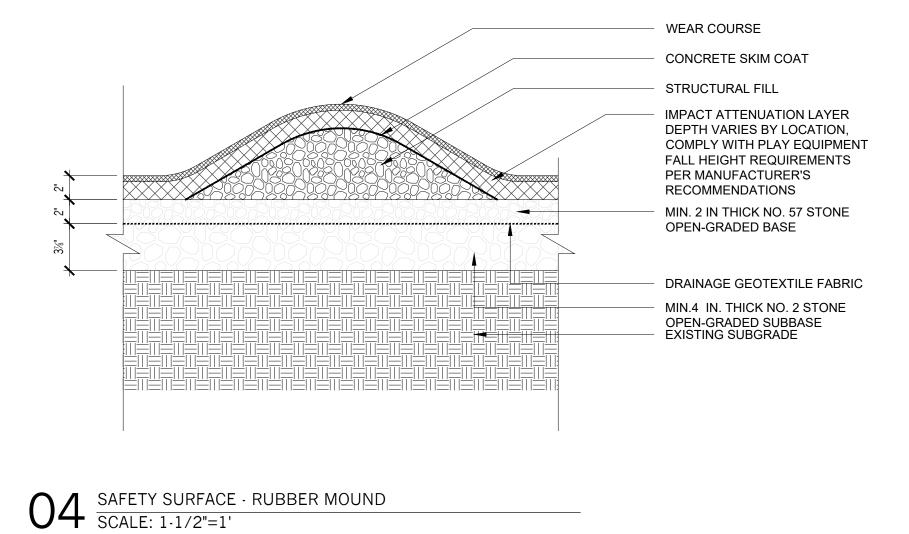
 $05 \frac{\mathsf{SAFETY} \; \mathsf{SURFACE} \cdot \mathsf{WOOD} \; \mathsf{FIBER}}{\mathsf{SCALE} \colon 1 \cdot 1 / 2" = 1"}$

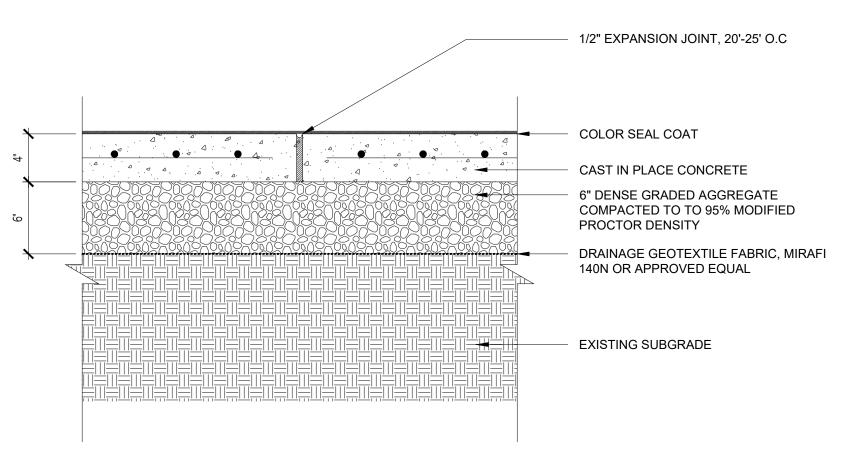




 $02^{\frac{\text{CAST-IN-PLACE CONCRETE PAVING JOINT}}{\text{SCALE: }1\cdot1/2"=1"}}$







REV. **07.17.2025 75% CD**

KEY PLAN:NTS

MA PROJECT NO. 2402 **AMELIA STREET SCHOOL ACCESSIBILITY TRAIL**

1821 AMELIA STREET CITY OF RICHMOND, VA

LANDSCAPE **DETAILS** -**PAVING**

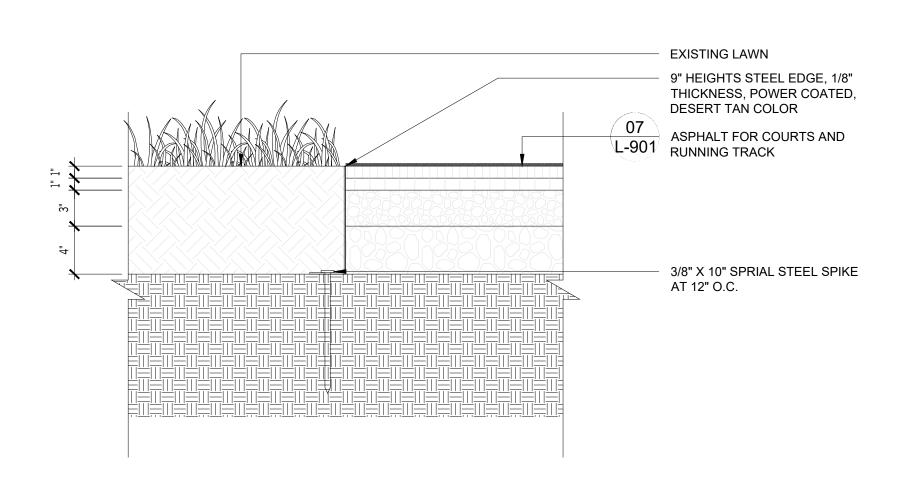
L-901.00

 $07 \frac{\text{ASPHALT FOR COURTS AND RUNNING TRACKS}}{\text{SCALE: } 1 \cdot 1 / 2 \text{"} = 1 \text{"}}$

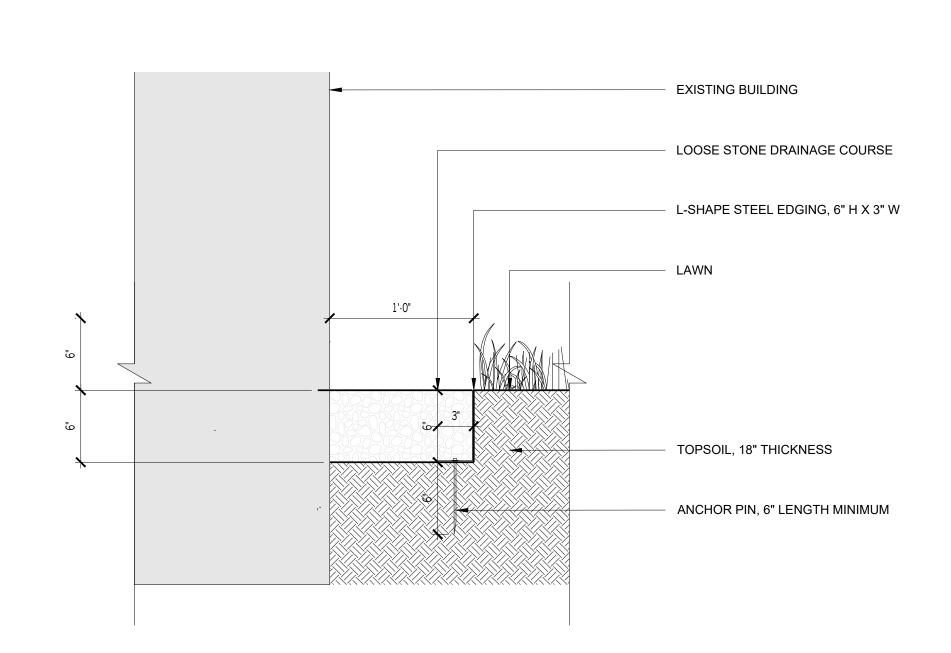
 $08^{\frac{\text{ASPHALT REPAVING}}{\text{SCALE: 1-1/2"=1'}}}$

 $01 \frac{\text{CAST-IN-PLACE CONCRETE PAVING}}{\text{SCALE: 1-1/2"=1'}}$

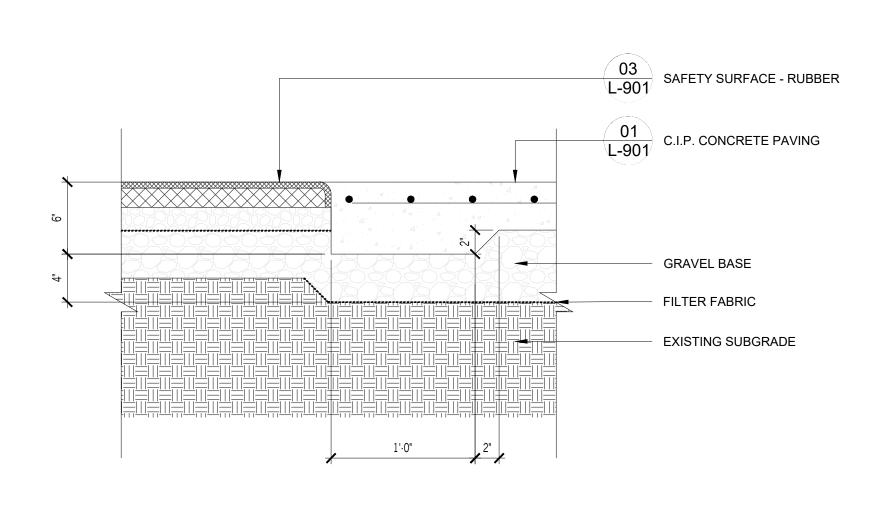
DATE 03.04.2025







O3 STONE DRAINAGE COURSE SCALE: 1-1/2"=1'

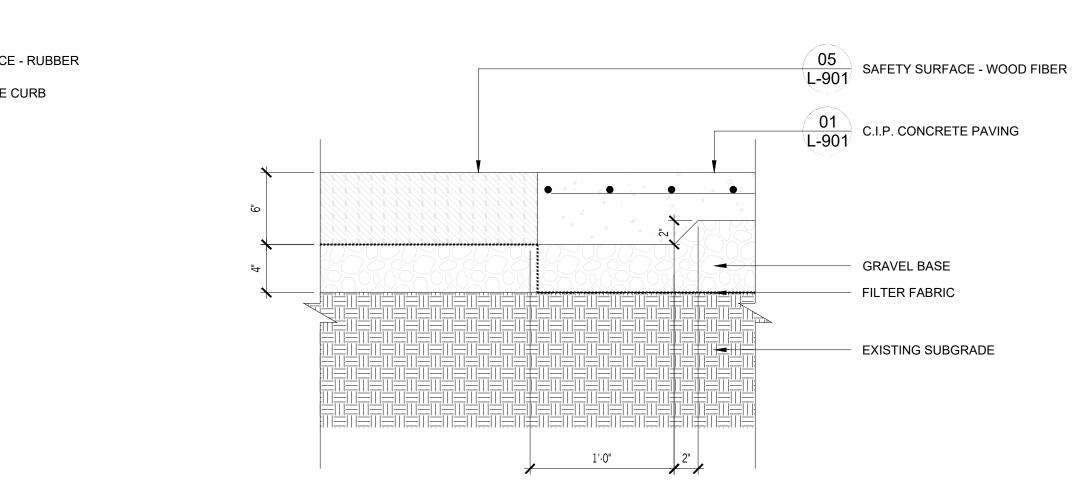


GRAVEL BASE

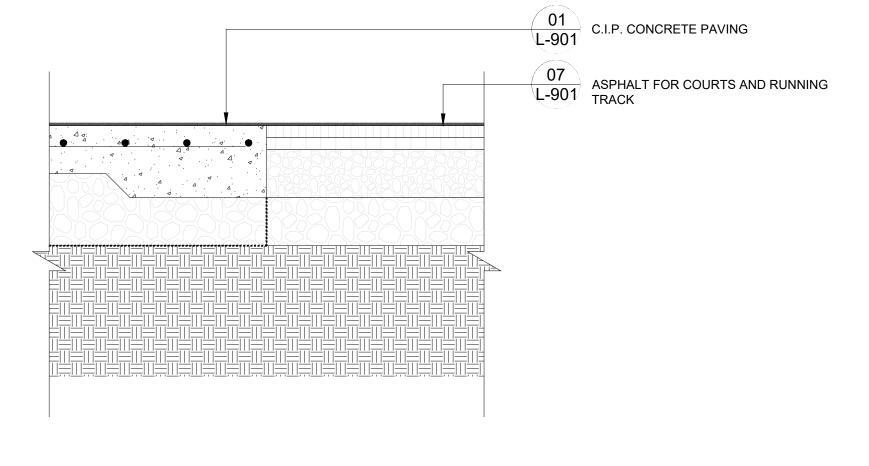
FILTER FABRIC

EXISTING SUBGRADE

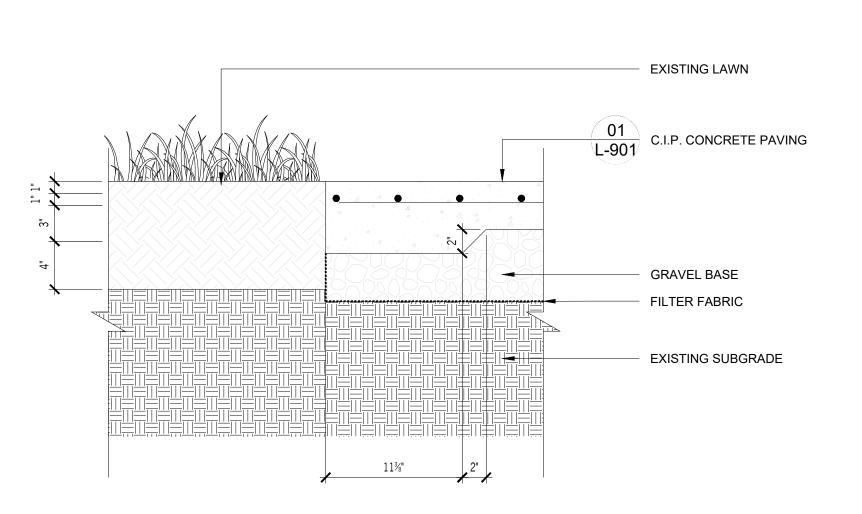
7 TRANSITION DETAIL@CONCRETE PAVING AND RUBBER SCALE: 1-1/2"=1'



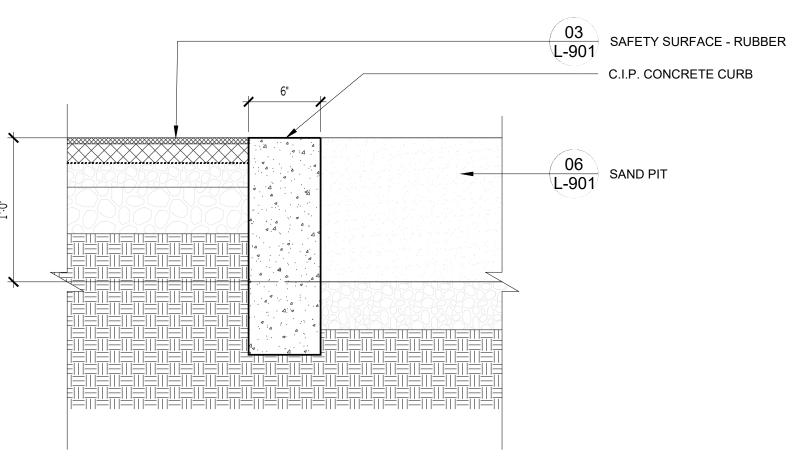
 $01 \scriptstyle{\frac{\text{TRANSITION DETAIL @ CONCRETE PAVING AND WOOD FIBER}{\text{SCALE: 1-1/2"=1'}}}$



05 TRANSITION DETAIL@ASPHALT AND CONCRETE PAVING SCALE: 1-1/2"=1'



7 TRANSITION DETAIL@CONCRETE AND LAWN SCALE: 1-1/2"=1'



 $04 \frac{\text{TRANSITION DETAIL@RUBBER AND SAND}}{\text{SCALE: } 1 \cdot 1/2"=1"}$

DATE 03.04.2025 © MARVEL ARCHITECTS, PLLC 2025

KEY PLAN:NTS

MA PROJECT NO. 2402 **AMELIA STREET SCHOOL ACCESSIBILITY TRAIL**

REV. **07.17.2025 75% CD**

MARVEL

OWNER/CLIENT ALLIANCE FOR THE CHESAPEAKE BAY

PROJECT MANAGER **NEAL FRIEDMAN**

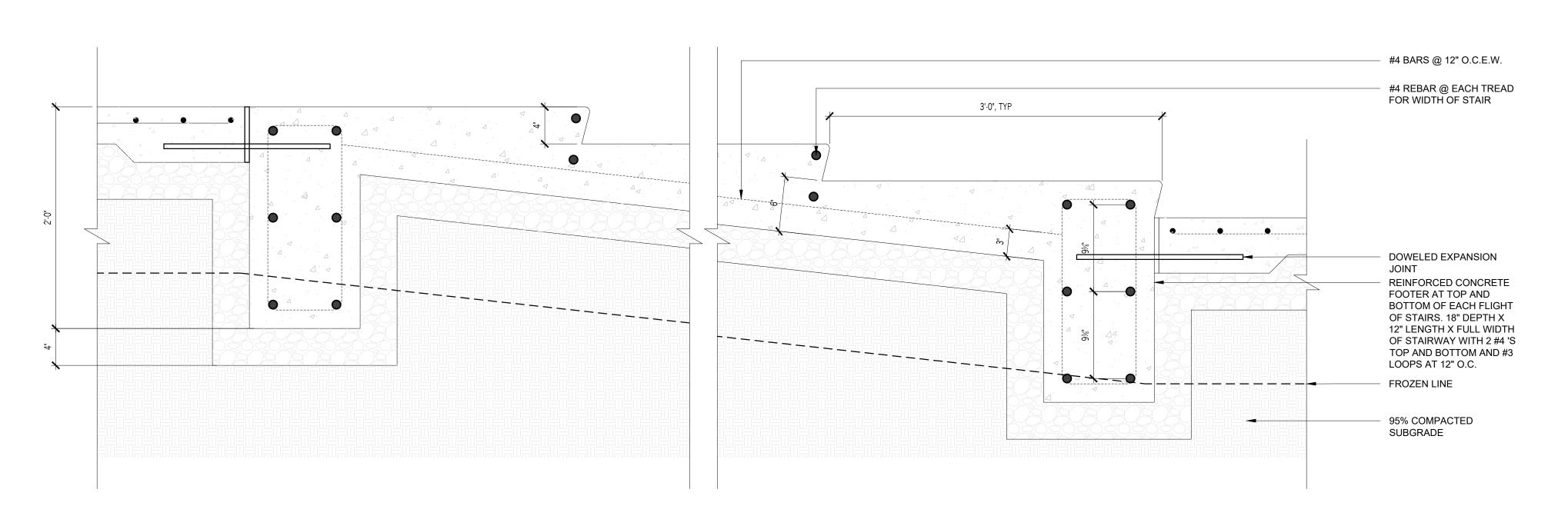
SURVEY **Nyfeler Survey**

CIVIL ENGINEERING NITSCH ENGINEERING

1821 AMELIA STREET CITY OF RICHMOND, VA

LANDSCAPE **DETAILS** -**TRANSITION**

L-902.00





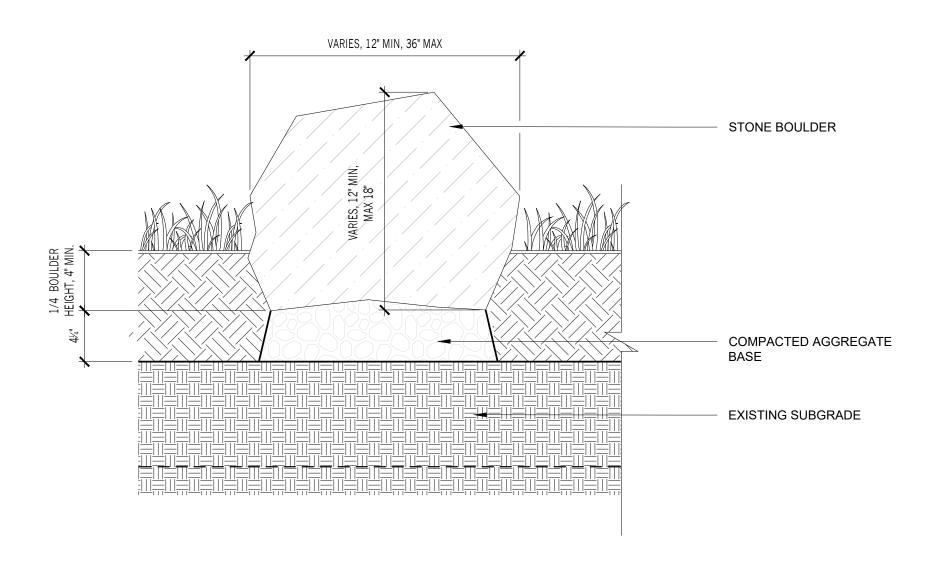
OWNER/CLIENT **ALLIANCE FOR THE CHESAPEAKE BAY**

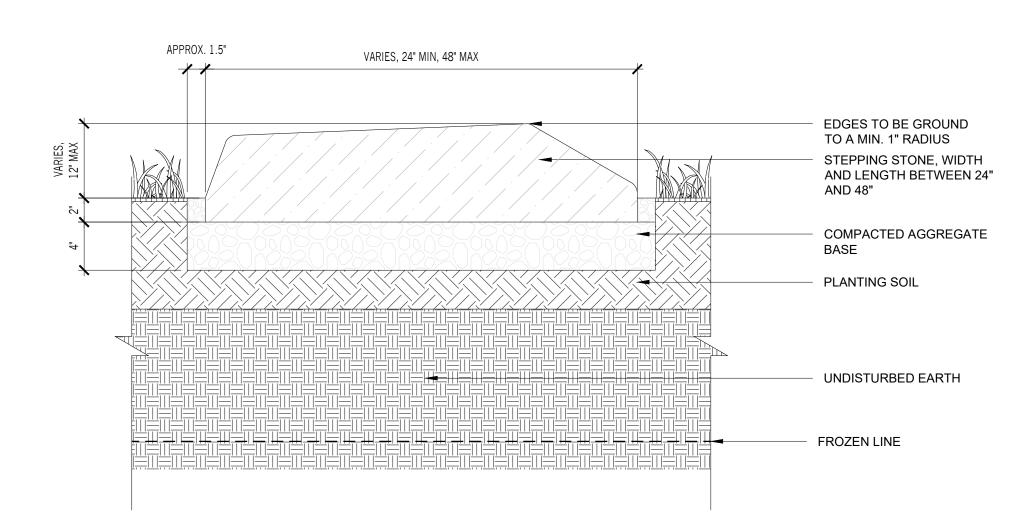
PROJECT MANAGER **NEAL FRIEDMAN**

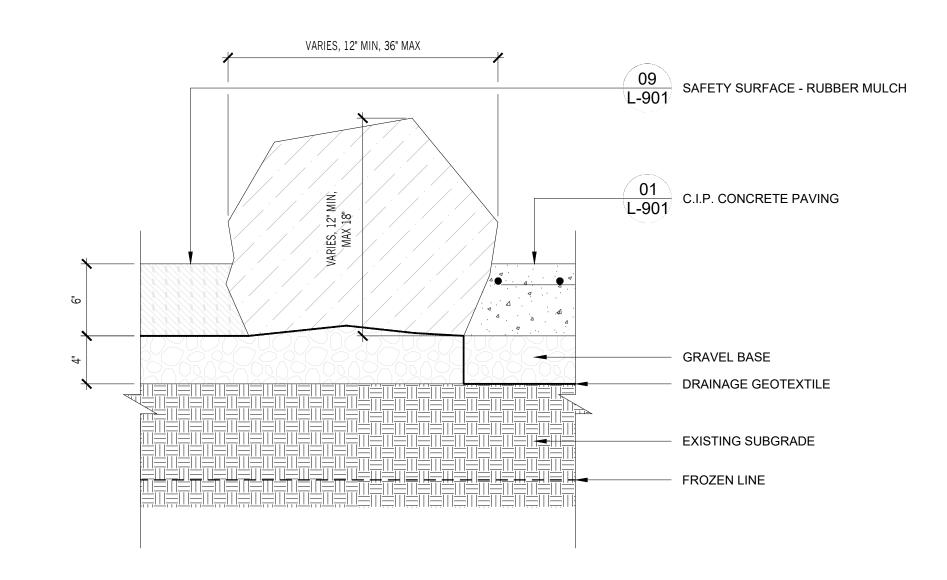
CIVIL ENGINEERING NITSCH ENGINEERING

SURVEY **Nyfeler Survey**

$07 \frac{\text{CAST-IN-PLACE CONCRETE STAIRS}}{\text{SCALE: 1-1/2"=1'}}$







REV. **07.17.2025 75% CD**

KEY PLAN:NTS

MA PROJECT NO. 2402 AMELIA STREET SCHOOL

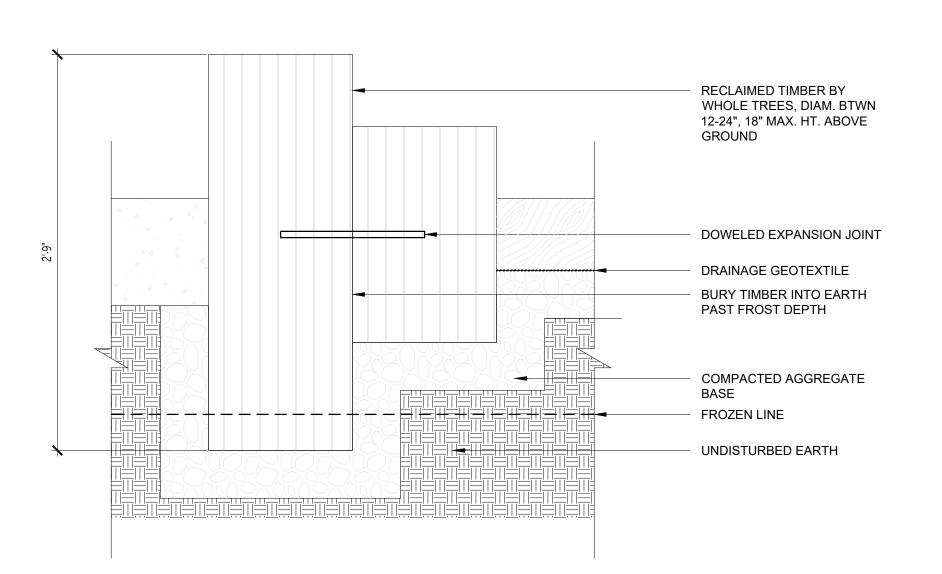
ACCESSIBILITY TRAIL

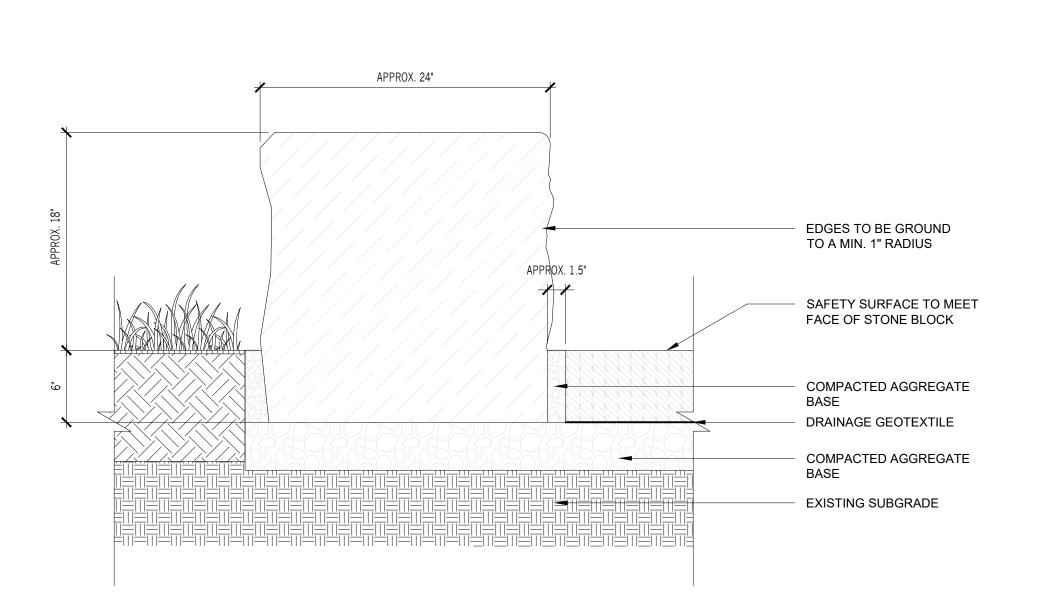
1821 AMELIA STREET CITY OF RICHMOND, VA

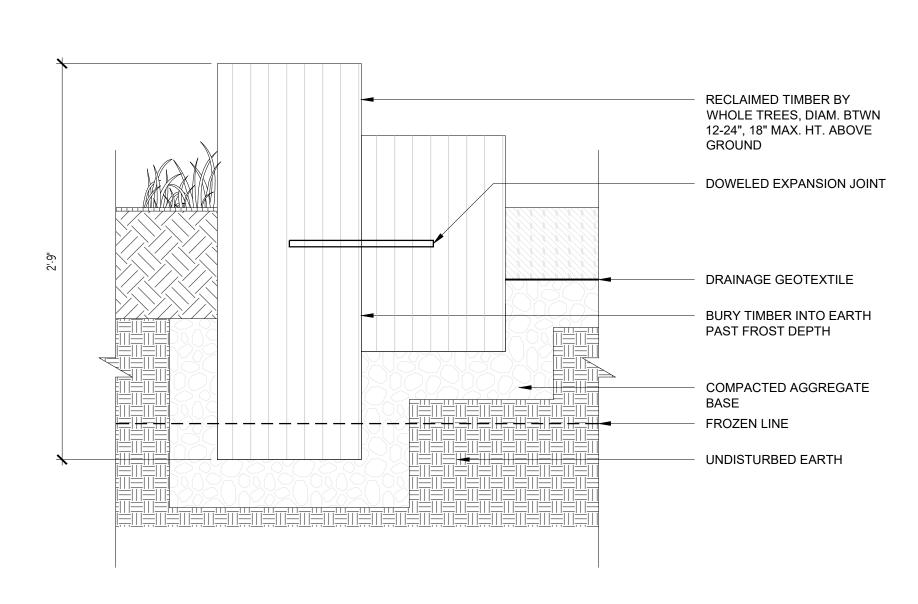
LANDSCAPE DETAILS -STRUCTURES

L-903.00

06 BOULDERS BLOCK
SCALE: 1-1/2"=1'







01 wood stump@lawn and wood fiber scale: 1-1/2"=1'

 $02^{\frac{\text{BOULDERS BLOCK}}{\text{SCALE: }1\cdot1/2"=1'}}$

O3 GRANITE BLOCK SEATING SCALE: 1-1/2"=1'

 $04 \frac{\text{STEPPING STONES}}{\text{SCALE: } 1 \cdot 1 / 2" = 1"}$

05 WOOD STUMP@CONCRETE AND WOOD FIBER SCALE: 1-1/2"=1'

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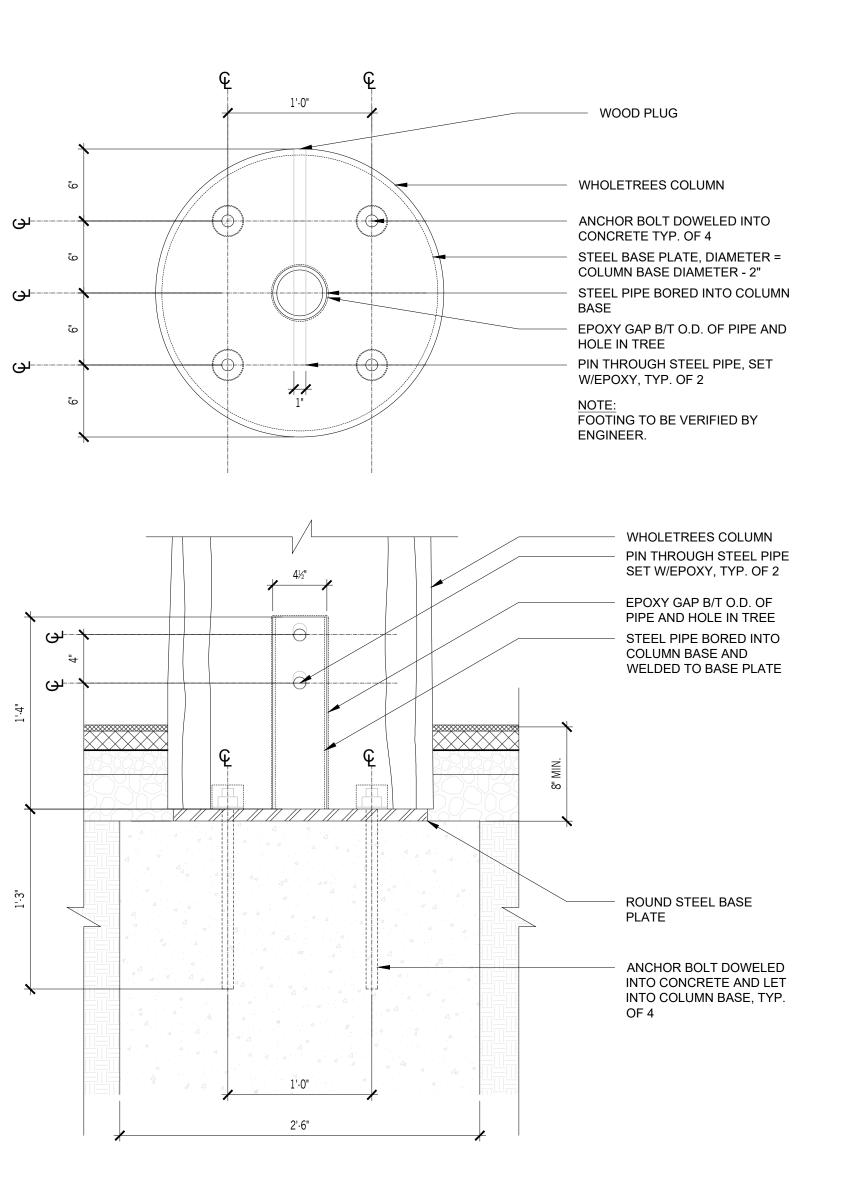


OWNER/CLIENT ALLIANCE FOR THE CHESAPEAKE BAY

PROJECT MANAGER **NEAL FRIEDMAN**

CIVIL ENGINEERING NITSCH ENGINEERING

SURVEY **Nyfeler Survey**



ADJACENT SURFACE VARIES,
SEE L-200 SERIES

TYPICAL COMPONENT SUPPORT

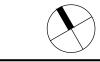
EXISTING SUBGRADE

C.I.P. CONCRETE BASE,
4500 PSI @ 28 DAYS

CHAMFERED EDGE
AGGREGATE BASE

REV. **07.17.2025 75% CD**

KEY PLAN:NTS



MA PROJECT NO. 2402

AMELIA STREET SCHOOL

ACCESSIBILITY TRAIL

1821 AMELIA STREET CITY OF RICHMOND, VA

LANDSCAPE DETAILS -GRAPHIC

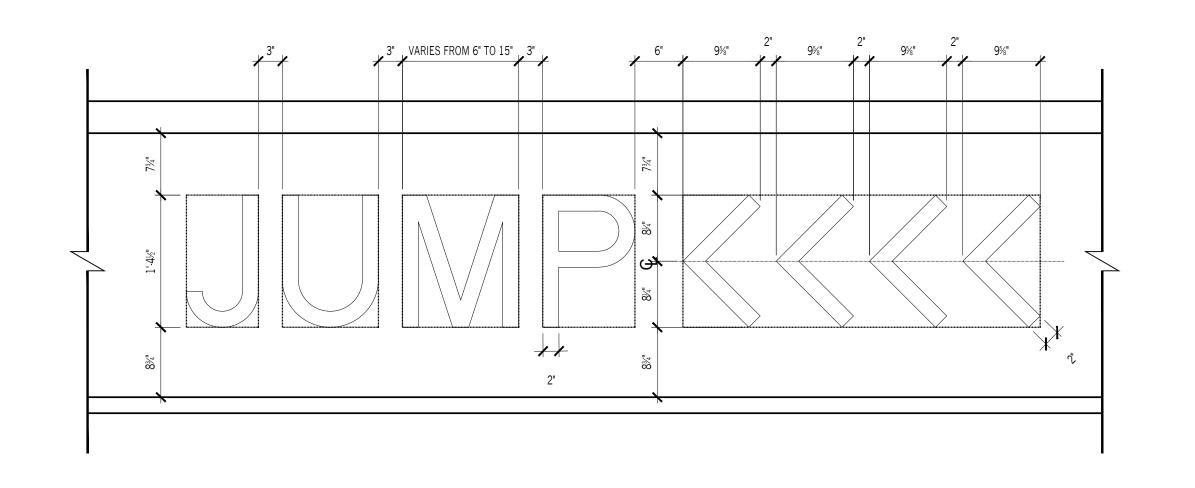
L-904.00

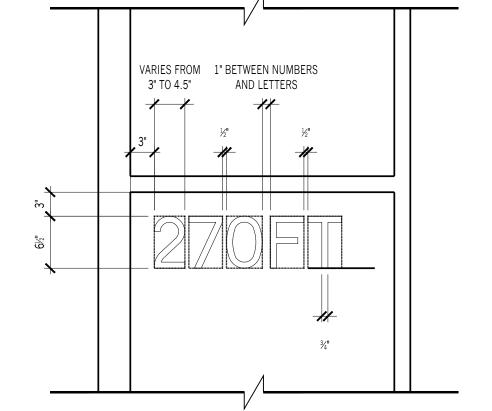
O 1 PLAY EQUIPMENT FOOTING SCALE: 1-1/2"=1'

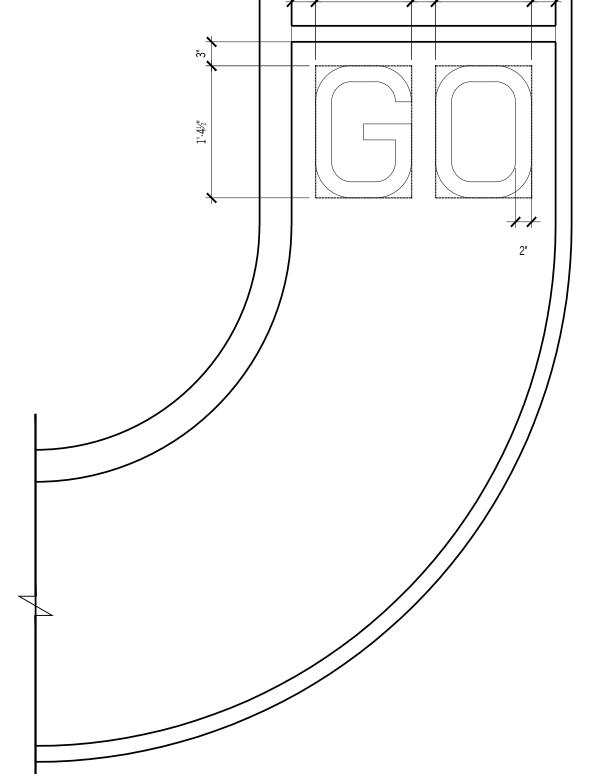
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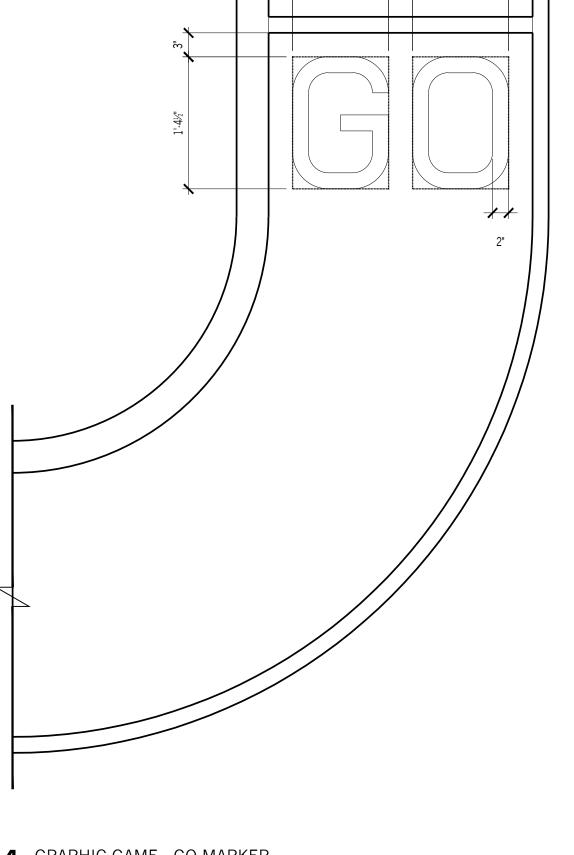
- 1. COLOR IS TO BE COLOR SEAL COAT RAL 9016 TRAFFIC WHITE BY ADVANCED POLYMER TECHNOLOGY OR APPROVED EQUAL.
- 2. CONTRACTOR TO PROVIDE PAINT SAMPLES FOR APPROVAL PRIOR TO PAINTING.
- 3. ALL NUMERALS ARE TO BE PATTERNED AFTER "ARIAL" STYLE. SAMPLE TO BE SUBMITTED TO LANDSCAPE ARCHITECT PRIOR TO PAINTING.
- 4. CONTRACTOR SHALL NOTIFY LANDSCAPE ARCHITECT PRIOR TO INSTALLATION TO APPROVE LAYOUT IN FIELD
- 5. ALL CIRCLES IN DETAIL 05 TO BE UNIFORM IN SIZE.
- 6. ALL ARROWS IN DETAIL 02 AND 07 TO BE UNIFORM IN SIZE.
- 7. THE TEXT IN DETAIL 07 MAY VARIES, REFER TO PLAN.

O5 GRAPHIC GAME - JUMPING HURDLES SCALE: 1"=1'









REV. **07.17.2025 75% CD**

KEY PLAN:NTS

MA PROJECT NO. 2402

1821 AMELIA STREET

CITY OF RICHMOND, VA

LANDSCAPE

DETAILS -

GRAPHIC

AMELIA STREET SCHOOL

ACCESSIBILITY TRAIL

MARVEL

OWNER/CLIENT ALLIANCE FOR THE CHESAPEAKE BAY

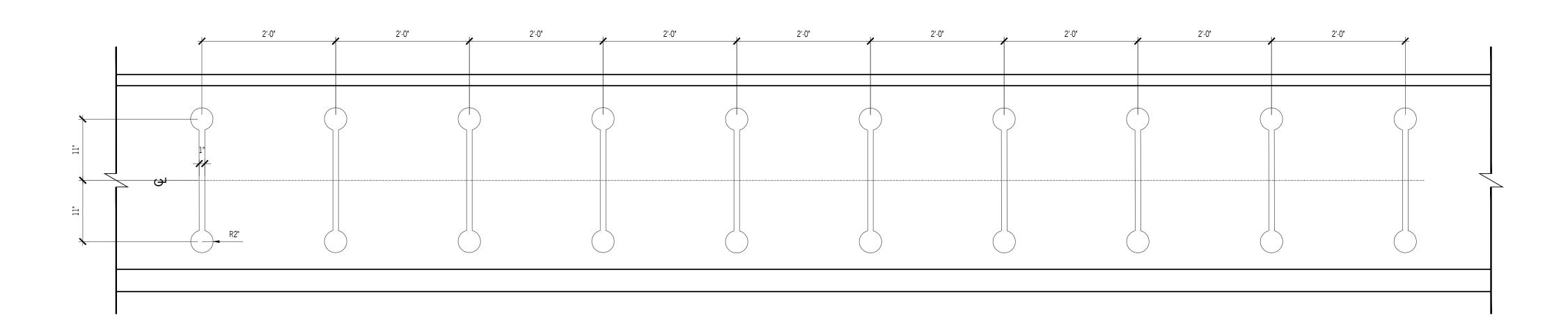
PROJECT MANAGER **NEAL FRIEDMAN**

SURVEY **NYFELER SURVEY**

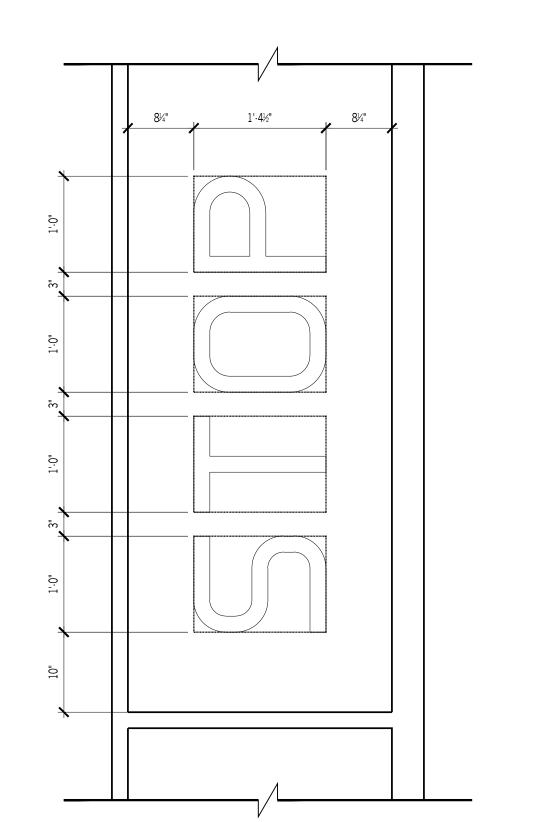
CIVIL ENGINEERING **NITSCH ENGINEERING**

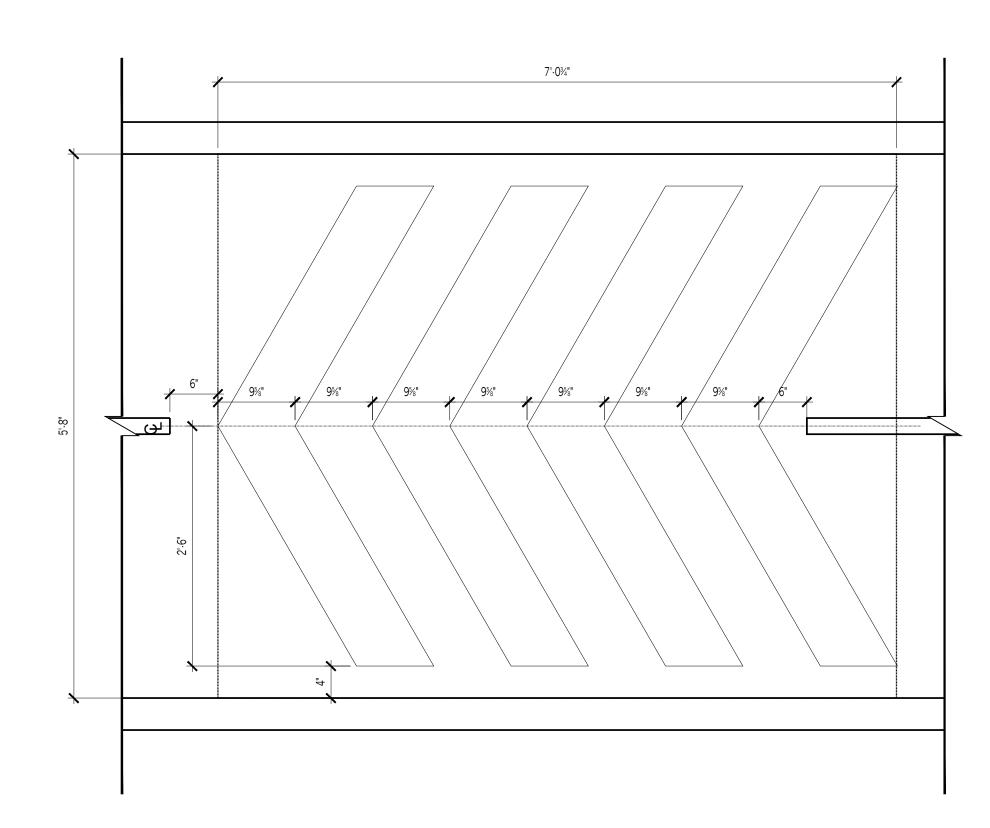
O7 GRAPHIC GAME - MARKER WITH ARROWS SCALE: 1"=1"

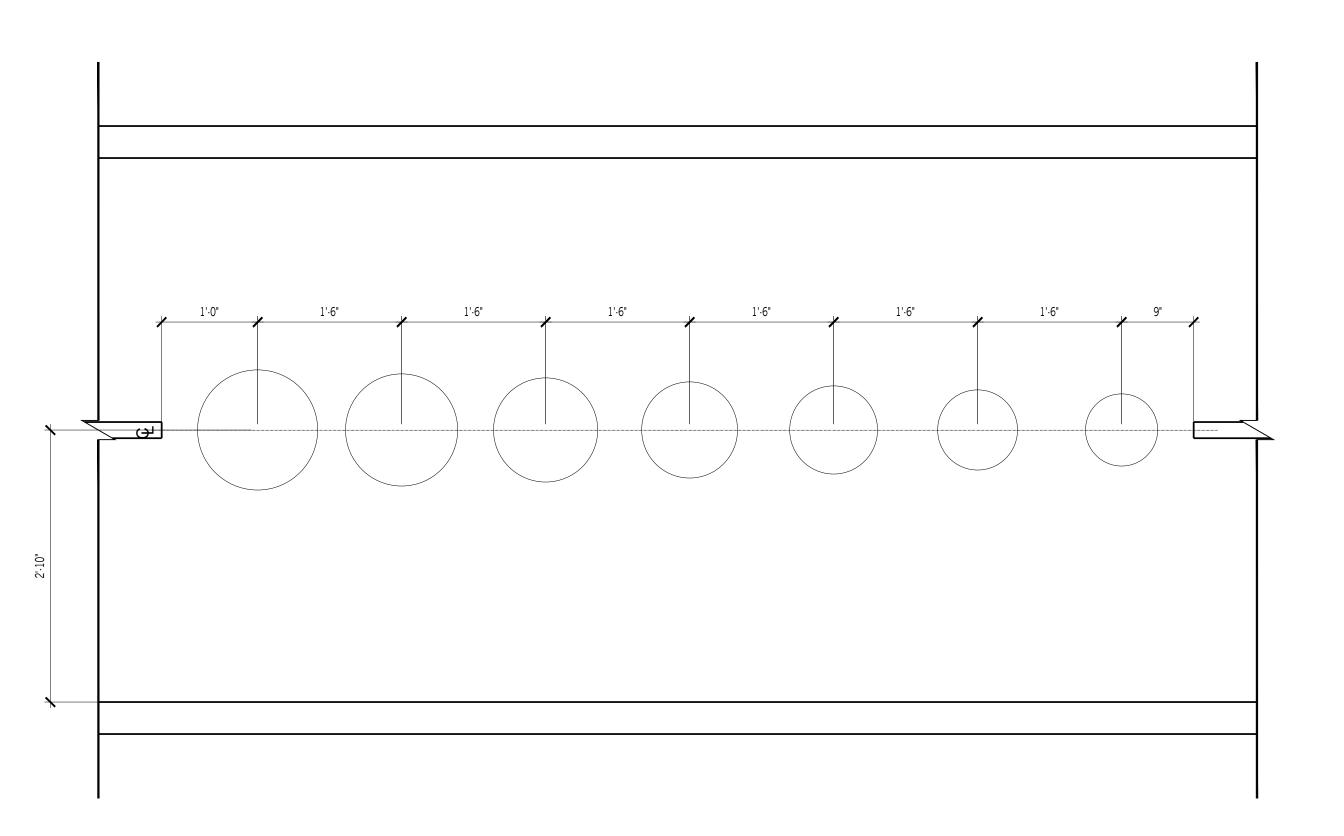
06 GRAPHIC GAME - MILE MARKER SCALE: 1"=1'



 $O4 \frac{\text{GRAPHIC GAME - GO MARKER}}{\text{SCALE: 1"=1'}}$







L-905.00

 $03^{\frac{\text{GRAPHIC GAME - STOP MARKER}}{\text{SCALE: 1"=1'}}}$

 $02^{\frac{\text{GRAPHIC GAME - GO STRAIGHT ARROWS}}{\text{SCALE: 1"=1'}}}$

O 1 GRAPHIC GAME - GO STRAIGHT DOTS SCALE: 1"=1'

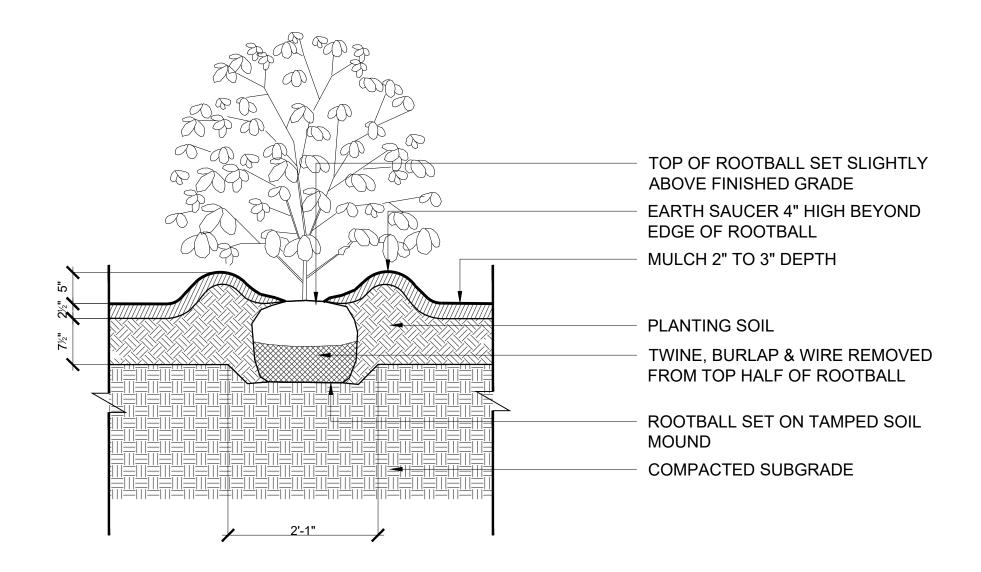


OWNER/CLIENT ALLIANCE FOR THE CHESAPEAKE BAY

PROJECT MANAGER **NEAL FRIEDMAN**

CIVIL ENGINEERING **NITSCH ENGINEERING**

SURVEY **NYFELER SURVEY**

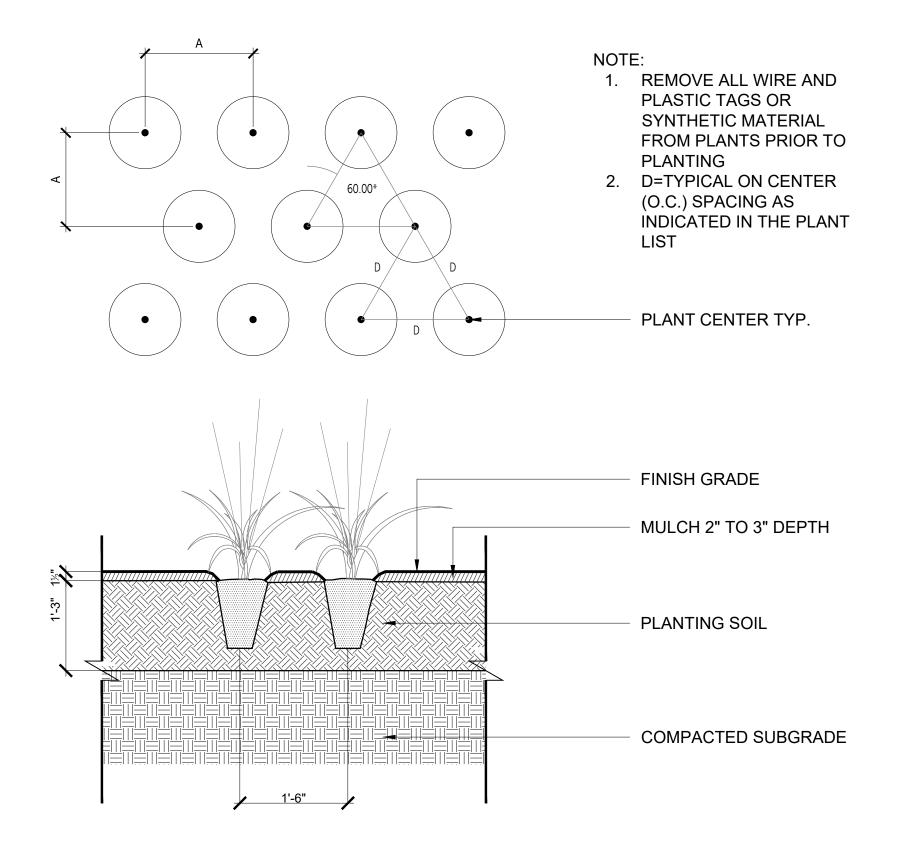


MULCH 2" DEPTH
PLANTING BED SOIL

COMPACTED SUBGRADE

03 TYPICAL PLANTING BED ON GRADE SCALE: 3/4"=1'

05 SHRUB PLANTING SCALE: 3/4"=1'

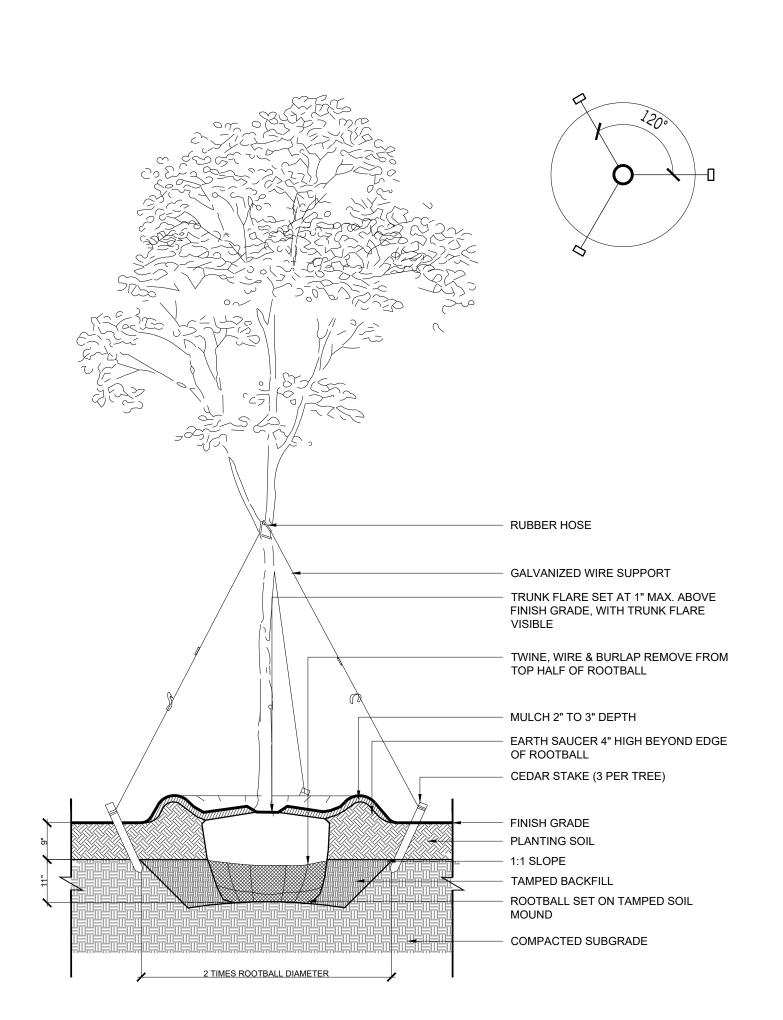


RUBBER HOSE

GALVANIZED WIRE SUPPORT

MULCH 2" TO 3" DEPTH

FINISH GRADE
CEDAR STAKE (3 PER TREE)
TWINE, WIRE & BURLAP REMOVE
FROM TOP 23 OF ROOTBALL
TOP SOIL MIXTURE
COMPACTED SUBGRADE



REV. **07.17.2025 75% CD**

KEY PLAN:NTS

MA PROJECT NO. 2402

AMELIA STREET SCHOOL

ACCESSIBILITY TRAIL

1821 AMELIA STREET CITY OF RICHMOND, VA

L-909.00

 $02^{\frac{\text{EVERGREEN TREE PLANTING AT GRADE}}{\text{SCALE: }3/4"=1"}}$

O 1 TREE PLANTING AT GRADE SCALE: 3/4"=1'

GENERAL NOTES

- 1. TOPOGRAPHIC DATA, PROPERTY LINE INFORMATION, AND EXISTING SITE FEATURES WERE OBTAINED FROM A SURVEY TITLED "TOPOGRAPHIC SURVEY FOR A PORTION OF AMELIA STREET SCHOOL" DATED 12/30/2024 BY NYFELER SURVEY, PREPARED FOR MARVEL DESIGNS.
- 2. FLOODPLAIN INFORMATION WAS OBTAINED FROM THE FLOOD INSURANCE RATE MAP (FIRM) NO. 5101290036D DATED APRIL 2, 2009. THE SITE IS IN ZONE X.
- 3. THE CONTRACTOR SHALL COMPLY WITH ALL APPLICABLE FEDERAL, STATE, AND LOCAL LAWS, RULES, REGULATIONS AND SAFETY CODES IN THE CONSTRUCTION OF ALL IMPROVEMENTS.
- 4. THE LOCATIONS AND ELEVATIONS OF ALL EXISTING UTILITIES ARE APPROXIMATE AND ALL UTILITIES MAY NOT BE SHOWN. PRESENCE AND LOCATIONS OF ALL UTILITIES WITHIN THE LIMIT OF WORK MUST BE DETERMINED BY THE CONTRACTOR PRIOR TO COMMENCEMENT OF CONSTRUCTION ACTIVITY. THE CONTRACTOR SHALL BE RESPONSIBLE FOR IDENTIFYING AND CONTACTING THE CONTROLLING AUTHORITIES AND/OR UTILITY COMPANIES RELATIVE TO THE LOCATIONS AND ELEVATIONS OF THEIR LINES. THE CONTRACTOR SHALL KEEP A RECORD OF ANY DISCREPANCIES OR CHANGES IN THE LOCATIONS OF ANY UTILITIES SHOWN OR ENCOUNTERED DURING CONSTRUCTION. ANY DISCREPANCIES SHALL BE REPORTED TO THE OWNER AND NITSCH ENGINEERING. ANY DAMAGE RESULTING FROM THE FAILURE OF THE CONTRACTOR TO MAKE THESE DETERMINATIONS AND CONTACTS SHALL BE BORNE BY THE CONTRACTOR.
- 5. THE CONTRACTOR SHALL, THROUGHOUT CONSTRUCTION, TAKE ADEQUATE PRECAUTIONS TO PROTECT ALL WALKS, GRADING, SIDEWALKS AND SITE DETAILS OUTSIDE OF THE LIMIT OF DISTURBANCE AS DEFINED ON THE DRAWINGS AND SHALL REPAIR AND REPLACE OR OTHERWISE MAKE GOOD AS DIRECTED BY THE ENGINEER OR OWNER'S DESIGNATED REPRESENTATIVE ANY SUCH OR OTHER DAMAGE SO CAUSED.
- 6. THE CONTRACTOR SHALL BE SOLELY RESPONSIBLE FOR JOB SITE SAFETY AND ALL CONSTRUCTION MEANS AND METHODS.
- 7. PRIOR TO BEGINNING CONSTRUCTION, THE CONTRACTOR SHALL BECOME FAMILIAR WITH THE SITE AND CONSTRUCTION DOCUMENTS TO DEVELOP A THOROUGH UNDERSTANDING OF THE PROJECT, INCLUDING ANY SPECIAL CONDITIONS AND CONSTRAINTS.
- 8. IT IS THE CONTRACTOR'S RESPONSIBILITY TO BECOME FAMILIAR WITH THE PROJECT SITE AND TO VERIFY ALL CONDITIONS IN THE FIELD AND REPORT DISCREPANCIES BETWEEN PLANS AND ACTUAL CONDITIONS TO THE OWNER OR OWNER'S REPRESENTATION IMMEDIATELY.
- 9. THE CONTRACTOR SHALL CONDUCT ALL NECESSARY CONSTRUCTION NOTIFICATIONS AND APPLY FOR AND OBTAIN ALL NECESSARY CONSTRUCTION PERMITS.
- 10. THE CONTRACTOR IS SOLELY RESPONSIBLE FOR THE ESTABLISHMENT AND USE OF ALL VERTICAL AND HORIZONTAL CONSTRUCTION CONTROLS.
- 11. ELEVATIONS REFER TO DATUM NAVD 88.

EARTH MOVING AND GRADING NOTES:

- 1. ALL TOPSOIL ENCOUNTERED WITHIN THE WORK AREA SHALL BE STRIPPED TO ITS FULL DEPTH AND STOCKPILED FOR REUSE. EXCESS TOPSOIL SHALL BE REMOVED FROM THE SITE UNLESS OTHERWISE DIRECTED BY THE OWNER TO REMAIN. TOPSOIL PILES SHALL REMAIN SEGREGATED FROM EXCAVATED SUBSURFACE SOIL MATERIALS.
- 2. CROSS SLOPES OF ALL PEDESTRIAN WALKS SHALL NOT EXCEED 1.5%.

ELEVATIONS TAKE PRECEDENCE OVER CONTOUR LINES.

- 3. RUNNING SLOPE OF ALL PEDESTRIAN WALKS SHALL NOT EXCEED 4.5%, UNLESS OTHERWISE NOTED.
- 4. THE CONTRACTOR SHALL EXERCISE CAUTION IN ALL EXCAVATION ACTIVITY DUE TO POSSIBLE EXISTENCE OF UNRECORDED UTILITY LINES.
- 5. ALL PAVED AREAS MUST PITCH TO DRAIN AT A MINIMUM OF 1% UNLESS OTHERWISE NOTED.
- 6. PROVIDE POSITIVE DRAINAGE AWAY FROM FACE OF BUILDINGS AT ALL LOCATIONS.
- 7. PITCH EVENLY BETWEEN CONTOUR LINES AND BETWEEN SPOT GRADES. SPOT GRADE
- 8. THE CONTRACTOR SHALL BLEND NEW GRADING SMOOTHLY INTO EXISTING GRADING AT
- LIMITS OF GRADING.
- 9. WHERE NEW PAVING MEETS EXISTING PAVING, MEET LINE AND GRADE OF EXISTING PAVING WITH SMOOTH TRANSITION BETWEEN EXISTING AND NEW SURFACES.
- 10. THE CONTRACTOR SHALL VERIFY EXISTING GRADES IN THE FIELD AND REPORT ANY DISCREPANCIES IMMEDIATELY TO THE ARCHITECT OR OWNER'S REPRESENTATIVE PRIOR TO STARTING WORK.
- 11. PITCH TOPS OF ALL WALLS AT ONE-EIGHTH INCH (1/8") PER FOOT FROM BACK OF WALL TO FACE OF WALL.
- 12. SURPLUS MATERIALS SHALL BE REMOVED FROM THE SITE UNLESS DIRECTED BY THE OWNER OR OWNER'S REPRESENTATIVE TO REMAIN. REFER TO EARTHWORK SPECIFICATIONS.
- 13. ANY AREAS OUTSIDE OF THE LIMIT OF WORK THAT ARE DISTURBED SHALL BE RESTORED BY THE CONTRACTOR TO THE PRE-CONSTRUCTION CONDITION/GRADE AT NO COST TO THE
- 14. EXCAVATION REQUIRED WITHIN PROXIMITY OF EXISTING UTILITY LINES SHALL BE DONE BY HAND. CONTRACTOR SHALL REPAIR ANY DAMAGE TO EXISTING UTILITY LINES OR STRUCTURES INCURRED DURING CONSTRUCTION OPERATIONS AT NO ADDITIONAL COST TO

DEMOLITION NOTES

- 1. SITE PREPARATION AND DEMOLITION SHALL INCLUDE THOSE AREAS WITHIN THE LIMITS OF DISTURBANCE LINE AS SHOWN ON THE CONTRACT DOCUMENTS.
- 2. ANY AREA OUTSIDE OF THE LIMITS OF DISTURBANCE THAT IS DISTURBED SHALL BE RESTORED TO ITS ORIGINAL CONDITION AT NO ADDITIONAL COST TO THE OWNER.
- 3. CONSULT ALL OF THE DRAWINGS AND SPECIFICATIONS FOR COORDINATION REQUIREMENTS BEFORE COMMENCING DEMOLITION.
- 4. THE CONTRACTOR SHALL COORDINATE SITE DEMOLITION EFFORTS WITH ALL TRADES THAT MAY BE AFFECTED BY THE WORK.

ALL ITEMS REQUIRING REMOVAL SHALL BE REMOVED TO FULL DEPTH TO INCLUDE BASE

- MATERIAL AND FOOTINGS OR FOUNDATIONS AS REQUIRED TO FACILITATE CONSTRUCTION, AND LEGALLY DISPOSED OF OFFSITE BY CONTRACTOR.
- 3. UTILITY PIPES DESIGNATED TO BE ABANDONED IN PLACE SHALL BE PLUGGED AT THEIR ENDS WITH WATERTIGHT BRICK MASONRY OR CEMENT MORTAR WITH A MINIMUM THICKNESS OF 8 INCHES.
- 7. UTILITY PIPES DESIGNATED TO BE REMOVED SHALL CONSIST OF THE COMPLETE REMOVAL AND DISPOSAL OF THE ENTIRE LENGTH OF PIPE AND BACKFILL AND 95% COMPACTION OF THE VOID WITH ORDINARY BORROW. WHEN THE VOID IS WITHIN THE FOOTPRINT OF THE NEW BUILDING, GRAVEL BORROW SHALL BE USED TO BACKFILL THE VOID.
- 8. UTILITY STRUCTURES DESIGNATED TO BE ABANDONED IN PLACE SHALL HAVE THEIR CAST IRON CASTINGS REMOVED AND DISPOSED, INLET AND OUTLET PIPES PLUGGED, THE BOTTOM OF THE STRUCTURES SHALL BE BROKEN, THE VOID OF THE STRUCTURES SHALL BE BACKFILLED AND COMPACTED TO 95% WITH ORDINARY BORROW OR FLOWABLE FILL, AND THE TOP OF THE STRUCTURE SHALL BE REMOVED SO THAT IT IS AT LEAST 36 INCHES BELOW FINISH GRADE.
- 9. UTILITY STRUCTURES DESIGNATED TO BE REMOVED SHALL CONSIST OF THE REMOVAL AND DISPOSAL OF CAST IRON CASTINGS, PLUGGING OF INLET AND OUTLET PIPES, REMOVAL OF THE STRUCTURE, AND BACKFILL AND 95% COMPACTION OF THE VOID WITH ORDINARY BORROW. WHEN HE VOID IS WITHIN THE FOOTPRINT OF THE NEW BUILDING, GRAVEL BORROW SHALL BE USED TO BACKFILL THE VOID.
- 10. ALL DEBRIS GENERATED DURING SITE PREPARATION ACTIVITIES SHALL BE LEGALLY DISPOSED OF OFFSITE.
- 11. AT ALL LOCATIONS WHERE EXISTING CURBING, CONCRETE PAVEMENT OR BITUMINOUS CONCRETE ROADWAY ABUTS NEW CONSTRUCTION, THE EDGE OF THE EXISTING CURB OR PAVEMENT SHALL BE SAW CUT TO A CLEAN, SMOOTH EDGE.
- 12. THE CONTRACTOR SHALL COORDINATE WITH THE ENGINEER PRIOR TO ANY EXPANSION OF THE LIMITS OF DISTURBANCE. IF IT IS DEEMED NECESSARY TO EXTEND THE DESIGNATED LIMITS OF DISTURBANCE TO ACCOMPLISH ROUGH GRADING, EROSION CONTROL, AND SITE WORK AS REQUIRED BY THESE DRAWINGS AND SPECIFICATIONS. ADDITIONAL PERMITTING WILL BE REQUIRED FOR ANY EXPANSION OF THE LIMITS OF DISTURBANCE.
- 13. THE CONTRACTOR SHALL REMOVE FROM THE SITE ALL RUBBISH AND DEBRIS FOUND THEREON. STORAGE OF SUCH MATERIALS ON THE PROJECT SITE WILL NOT BE PERMITTED. THE CONTRACTOR SHALL LEAVE THE SITE IN SAFE, CLEAN, AND LEVEL CONDITION UPON COMPLETION OF THE SITE DEMOLITION WORK.
- 14. REMOVE AND STOCKPILE ALL EXISTING SITE LIGHTS, BENCHES, TRASH RECEPTACLES, TRAFFIC SIGNS, GRANITE CURB, AND OTHER SITE IMPROVEMENTS WITHIN LIMIT OF WORK LINE UNLESS OTHERWISE NOTED.
- 15. ALL EXISTING TREES AND SHRUBS TO REMAIN SHALL BE PROTECTED AND MAINTAINED THROUGHOUT THE TIME OF CONSTRUCTION, AS SPECIFIED AND DIRECTED BY THE EROSION AND SEDIMENT CONTROL PLANS.
- 16. BEFORE ANY TREES OR SHRUBS ARE REMOVED, THE CONTRACTOR SHALL ARRANGE A CONFERENCE ON THE SITE WITH THE OWNER OR OWNER'S REPRESENTATIVE TO IDENTIFY TREES AND SHRUBS THAT ARE TO BE REMOVED, AS WELL AS THOSE WHICH ARE TO BE PROTECTED. DO NOT COMMENCE CLEARING OPERATIONS WITHOUT A CLEAR UNDERSTANDING OF EXISTING CONDITIONS TO BE PRESERVED.
- 17. THE CONTRACTOR SHALL COORDINATE WITH THE ENGINEER PRIOR TO REMOVAL FROM THE AREA OF CONSTRUCTION PAVEMENT, CONCRETE, CURBING, POLES AND FOUNDATIONS, ISLANDS, TREE BERMS AND OTHER FEATURES WITHIN THE LIMITS OF CONSTRUCTION THAT ARE NOT INDICATED TO BE REMOVED ON SHEET C-1.0 AS REQUIRED

TO ACCOMMODATE NEW CONSTRUCTION.

EROSION AND SEDIMENT CONTROL NOTES:

- 1. PRIOR TO STARTING ANY OTHER WORK ON THE SITE, THE CONTRACTOR SHALL NOTIFY APPROPRIATE AGENCIES AND SHALL INSTALL EROSION CONTROL MEASURES AS SHOWN ON THE PLANS AND AS IDENTIFIED IN FEDERAL, STATE, AND LOCAL APPROVAL DOCUMENTS PERTAINING TO THIS PROJECT.
- 2. SEEDING OPERATIONS SHALL BE INITIATED WITHIN 7 DAYS AFTER REACHING FINAL GRADE OR UPON SUSPENSION OF GRADING OPERATIONS FOR ANTICIPATED DURATION OF GREATER THAN 14 DAYS OR UPON COMPLETION OF GRADING OPERATIONS FOR A SPECIFIC AREA.
- 3. CONTRACTOR SHALL INSPECT AND MAINTAIN EROSION CONTROL MEASURES AND REMOVE SEDIMENT THEREFROM ON A WEEKLY BASIS AND WITHIN TWELVE HOURS AFTER EACH STORM EVENT. CONTRACTOR SHALL DISPOSE OF ANY SEDIMENTS REMOVED FROM EROSION CONTROL MEASURES IN AN UPLAND AREA SUCH THAT THEY DO NOT ENCUMBER OTHER DRAINAGE STRUCTURES AND PROTECTED AREAS.
- 4. THE CONTRACTOR IS RESPONSIBLE FOR THE DAILY REMOVAL OF SEDIMENT THAT HAS BEEN TRANSPORTED ONTO A PAVED OR PUBLIC ROAD SURFACE.
- THE CONTRACTOR SHALL BE RESPONSIBLE FOR PREVENTING SURFACE AND AIR MOVEMENT OF DUST FROM EXPOSED SOILS WHICH MAY PRESENT HEALTH HAZARDS, TRAFFIC SAFETY PROBLEMS, OR HARM ANIMAL OR PLANT LIFE.
- 6. DISTURBANCE IS LOCATED ENTIRELY WITHIN THE RICHMOND COMBINED SEWER OVERFLOW (CSO).SEE HTTP://WWW.DEQ.VIRGINIA.GOV/PROGRAM/WATER/STORMWATERMANAGEMENT/VSPPERMITS.ASPX FOR INFORMATION
- 7. CONTRACTOR SHALL BE FULLY RESPONSIBLE TO CONTROL CONSTRUCTION SUCH THAT SEDIMENTATION SHALL NOT AFFECT REGULATORY PROTECTED AREAS WHETHER SUCH SEDIMENTATION IS CAUSED BY WATER, WIND, OR DIRECT DEPOSIT.
- CONTRACTOR SHALL SEQUENCE CONSTRUCTION SUCH THAT EARTH MATERIALS ARE EXPOSED FOR THE MINIMUM LENGTH OF TIME BEFORE THEY ARE COVERED, SEEDED, OR OTHERWISE STABILIZED TO PREVENT EROSION.
- 9. UPON COMPLETION OF CONSTRUCTION AND ESTABLISHMENT OF PERMANENT GROUND COVER, CONTRACTOR SHALL REMOVE AND DISPOSE OF EROSION CONTROL MEASURES AND CLEAN SEDIMENT AND DEBRIS FROM ALL DRAINAGE AND SEWER SYSTEMS.

LAYOUT AND MATERIALS NOTES:

DOCKS, BOLLARDS, ETC.

- DIMENSIONS ARE FROM THE FACE OF CURB, FACE OF BUILDING, FACE OF WALL, AND CENTER LINE OF PAVEMENT MARKINGS, UNLESS OTHERWISE NOTED.
- 2. CURB RADII ARE 5 FEET TO FACE OF CURB UNLESS OTHERWISE NOTED.
- 3. CURBING SHALL BE CG-2 WITHIN THE SITE UNLESS OTHERWISE INDICATED ON THE PLANS.
- 4. SEE ARCHITECTURAL DRAWINGS FOR EXACT BUILDING DIMENSIONS AND DETAILS CONTIGUOUS TO THE BUILDING, INCLUDING SIDEWALKS, RAMPS, BUILDING ENTRANCES, STAIRWAYS, UTILITY PENETRATIONS, CONCRETE DOOR PADS, COMPACTOR PAD, LOADING
- 5. PROPOSED BONDS AND ANY EXISTING PROPERTY LINE MONUMENTS DISTURBED DURING CONSTRUCTION SHALL BE SET OR RESET BY A PROFESSIONAL LAND SURVEYOR.
- 6. PRIOR TO START OF CONSTRUCTION, CONTRACTOR SHALL VERIFY EXISTING PAVEMENT ELEVATIONS AT INTERFACE WITH PROPOSED PAVEMENTS, AND EXISTING GROUND ELEVATIONS ADJACENT TO DRAINAGE OUTLETS TO ASSURE PROPER TRANSITIONS BETWEEN EXISTING AND PROPOSED FACILITIES.
- 7. SYMBOLS AND LEGENDS OF PROJECT FEATURES ARE GRAPHIC REPRESENTATIONS AND ARE NOT NECESSARILY SCALED TO THEIR ACTUAL DIMENSIONS OR LOCATIONS ON THE DRAWINGS. THE CONTRACTOR SHALL REFER TO THE DETAIL SHEET DIMENSIONS, MANUFACTURERS' LITERATURE, SHOP DRAWINGS AND FIELD MEASUREMENTS OF SUPPLIED PRODUCTS FOR LAYOUT OF THE PROJECT FEATURES.
- 8. CONTRACTOR SHALL NOT RELY SOLELY ON ELECTRONIC VERSIONS OF THE PLANS, SPECIFICATIONS, AND DATA FILES THAT ARE OBTAINED FROM THE DESIGNERS, BUT SHALL VERIFY LOCATION OF PROJECT FEATURES IN ACCORDANCE WITH THE PAPER COPIES OF THE PLANS AND SPECIFICATIONS THAT ARE SUPPLIED AS A PART OF THE CONTRACT DOCUMENTS.

EXISTING LEGEND

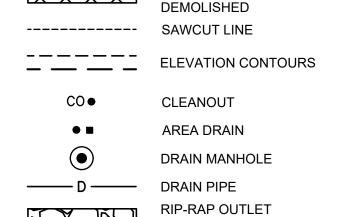
	EXISTING ELEVATION CONTOURS		
		AD	AREA DRAIN
	EDGE OF LANDSCAPED AREA	ВС	BOTTOM OF CURB ELEVATION
	EXISTING ASPHALT	BS	BOTTOM OF STAIR ELEVATION
	EXISTING / (GITI/LET	BW	BOTTOM OF WALL ELEVATION
4	EXISTING CONCRETE	СВ	CATCH BASIN
	SURVEY CONTROL POINT	CO	CLEANOUT
		CPP	CORRUGATED POLYETHYLENE PIPE
000	CLEAN OUT	DMH	DRAIN MANHOLE
	UTILITY VAULT/HANDHOLE	FFE	FINISHED FLOOR ELEVATION
RRALI	DACKETRALL LICOR	INV	INVERT ELEVATION
) DDALL	BASKETBALL HOOP	LF	LINEAR FEET
O	DECIDUOUS TREE	LOD	LIMIT OF DISTURBANCE
53	EVERGREEN TREE	MG	MATCH EXISTING GRADE
Security		PERF	PERFORATED
•	FENCE GATE POST	PVC	POLYVINYL CHLORIDE PIPE
x ————	FENCE	RD	ROOF DRAIN
	0.01	RIM	RIM ELEVATION
_	SIGN	TC	TOP OF CURB ELEVATION

LIMITS OF DISTURBANCE (0.92 AC)

EXISTING SITE FEATURE TO BE

PROPOSED LEGEND

XXXX



PROTECTION

PROPERTY LINE

ABBREVIATIONS

TOP OF WALL ELEVATION

TYP TYPICAL

TARRET, FLR.3 NEW YORK, NY 10013 212.616.0420

OWNER/CLIENT ALLIANCE FOR THE CHESAPEAKE BAPPOJECT MANAGER NEAL FRIEDMAN
CIVIL ENGINEERING NITSCH ENGINEERING

SURVEY NYFELER SURVEY

CONSTRUCTION

REV. **07.17.2025 PERMIT**

KEY PLAN:NTS

MA PROJECT NO. 2402

AMELIA STREET SCHOOL ACCESSIBILITY TRAIL

CITY OF RICHMOND, VA

1821 AMELIA STREET

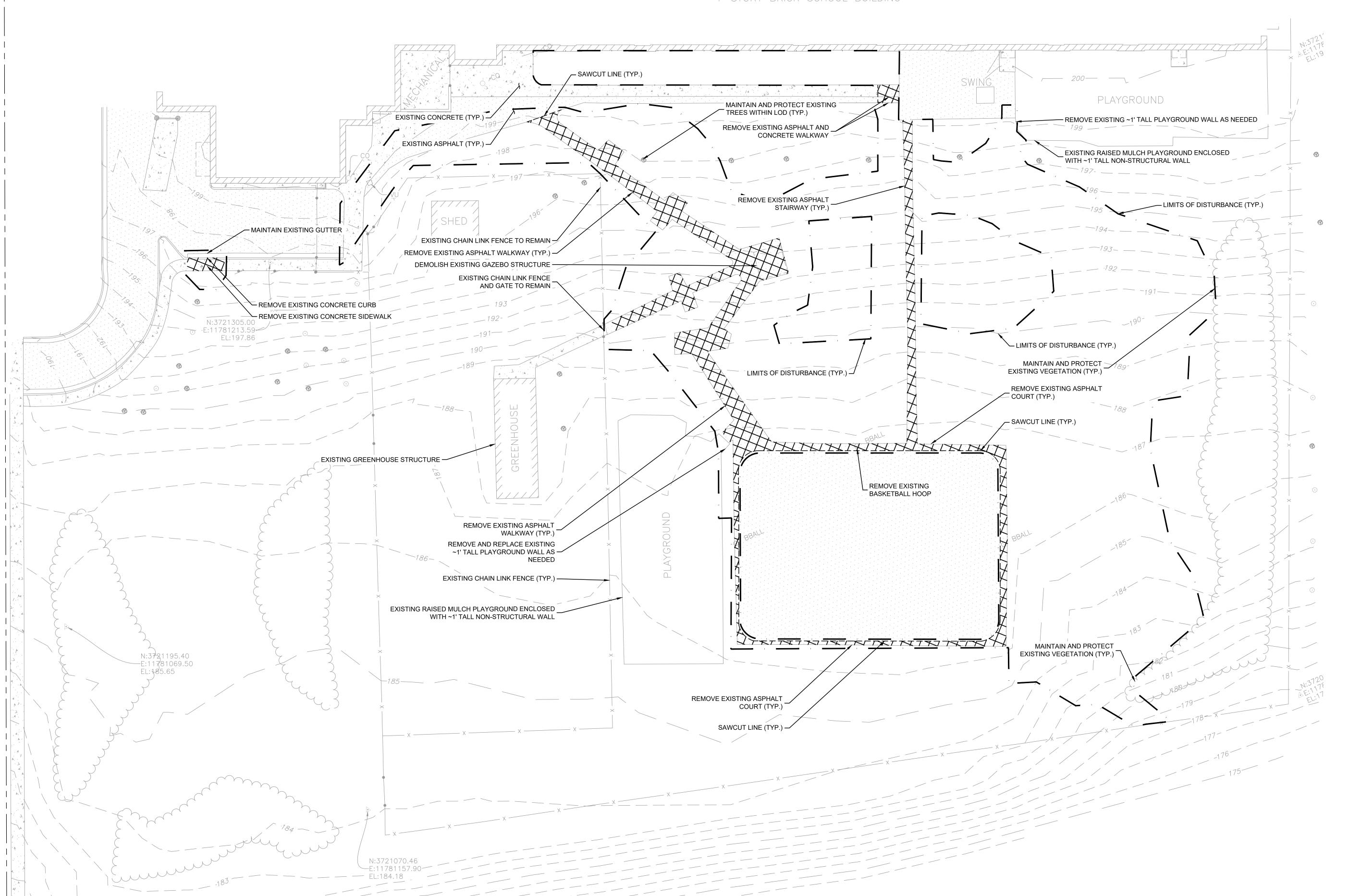
NOTES, LEGEND & ABBREVIATIONS

_ 01 ____

C-0.0

DATE 03.04.2025 © MARVEL ARCHITECTS, PLLC 2025

AMELIA STREET SCHOOL 1-STORY BRICK SCHOOL BUILDING







OWNER/CLIENT ALLIANCE FOR THE CHESAPEAKE BAY

PROJECT MANAGER **NEAL FRIEDMAN**

CIVIL ENGINEERING NITSCH ENGINEERING

SURVEY **Nyfeler Survey**

NOTFORCTION

REV. **07.17.2025 PERMIT**

KEY PLAN:NTS

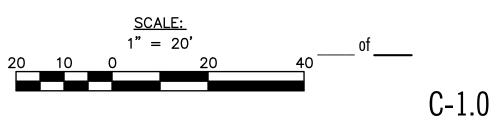
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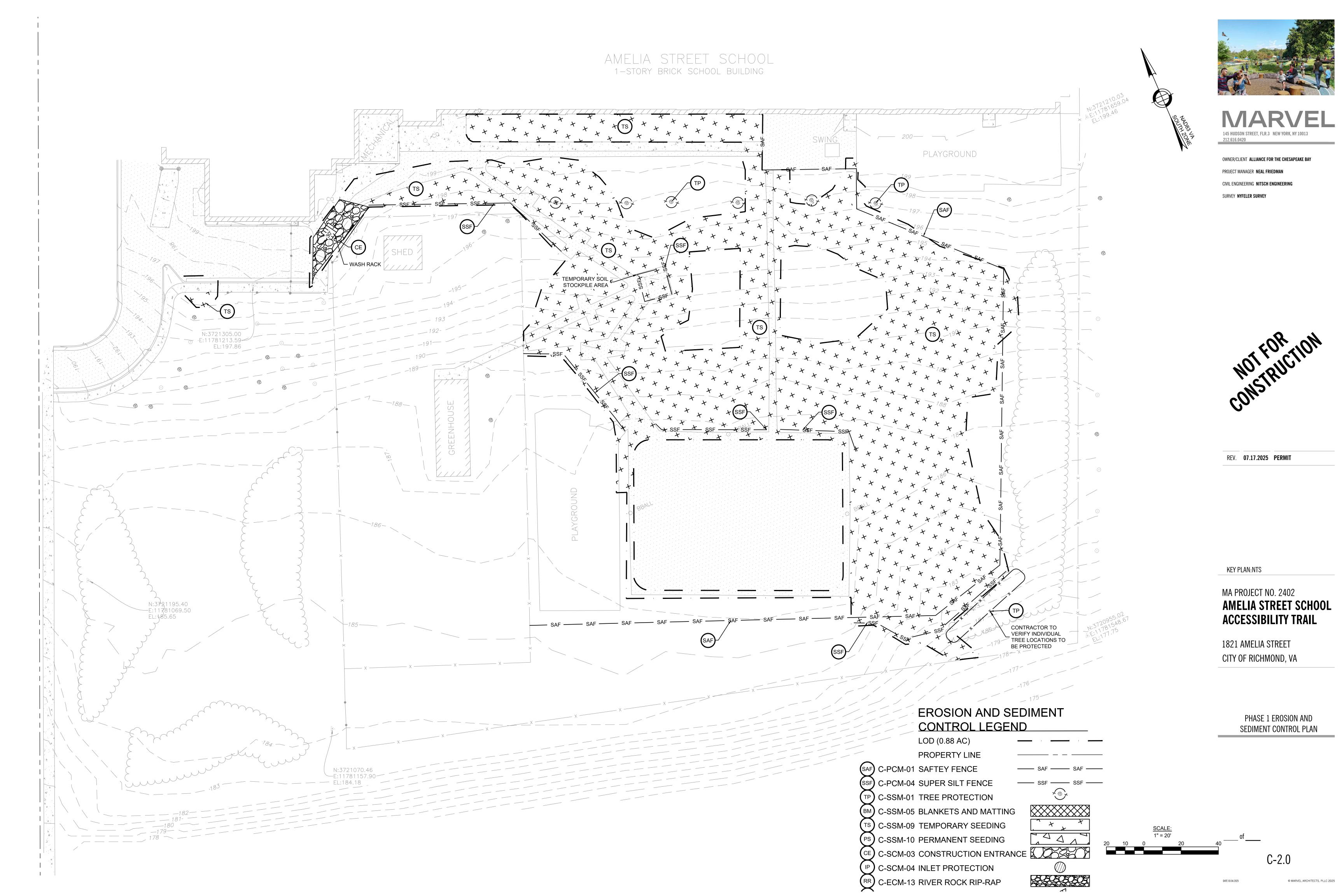
AMELIA STREET SCHOOL

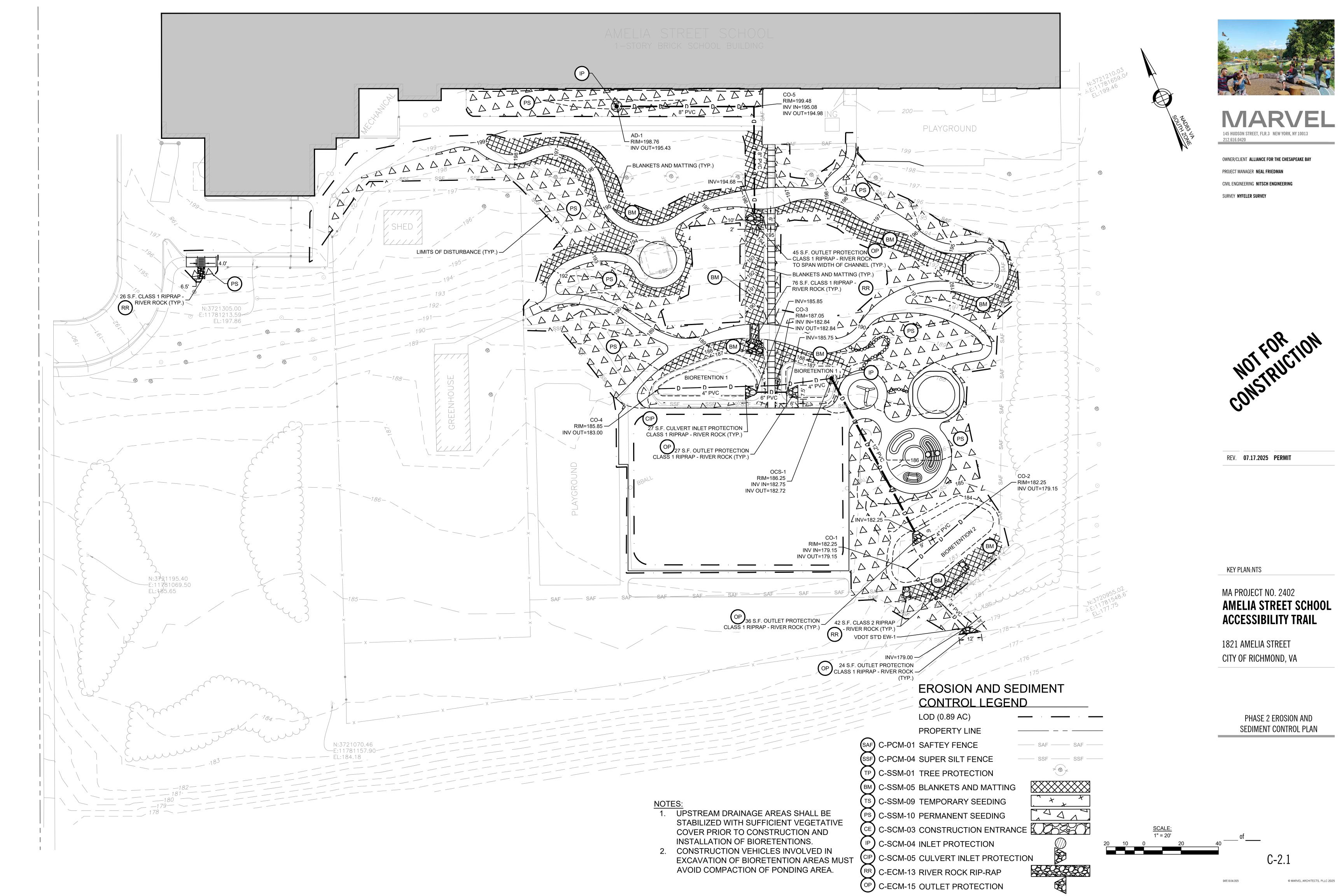
ACCESSIBILITY TRAIL

1821 AMELIA STREET CITY OF RICHMOND, VA

EXISTING CONDITIONS AND DEMOLITION PLAN







9VAC25-875-560. EROSION AND SEDIMENT CONTROL CRITERIA, ΓECHNIQUES, AND METHODS: MINIMUM STANDARDS

AN EROSION AND SEDIMENT CONTROL PLAN CONSISTENT WITH THE FOLLOWING CRITERIA, TECHNIQUES, AND METHODS SHALL BE SUBMITTED TO THE VESMP AUTHORITY OR VESCP AUTHORITY FOR REVIEW AND APPROVAL:

1. PERMANENT OR TEMPORARY SOIL STABILIZATION SHALL BE APPLIED TO DENUDED AREAS WITHIN SEVEN DAYS AFTER FINAL GRADE IS REACHED ON ANY PORTION OF THE SITE. TEMPORARY SOIL STABILIZATION SHALL BE APPLIED WITHIN SEVEN DAYS TO DENUDED AREAS THAT MAY NOT BE AT FINAL GRADE BUT WILL REMAIN DORMANT FOR LONGER THAN 14 DAYS. PERMANENT STABILIZATION SHALL BE APPLIED TO AREAS THAT ARE TO BE LEFT DORMANT FOR MORE THAN ONE

2. DURING CONSTRUCTION OF THE PROJECT, SOIL STOCKPILES AND BORROW AREAS SHALL BE STABILIZED OR PROTECTED WITH SEDIMENT TRAPPING MEASURES. THE APPLICANT IS RESPONSIBLE FOR THE TEMPORARY PROTECTION AND PERMANENT STABILIZATION OF ALL SOIL STOCKPILES ON SITE AS WELL AS BORROW AREAS AND SOIL INTENTIONALLY TRANSPORTED FROM THE PROJECT SITE.

THAT IS UNIFORM. IS MATURE ENOUGH TO SURVIVE. AND WILL INHIBIT EROSION. 4. SEDIMENT BASINS AND TRAPS, PERIMETER DIKES, SEDIMENT BARRIERS, AND OTHER MEASURES INTENDED TO TRAP

STABILIZED. PERMANENT VEGETATION SHALL NOT BE CONSIDERED ESTABLISHED UNTIL A GROUND COVER IS ACHIEVED

3. A PERMANENT VEGETATIVE COVER SHALL BE ESTABLISHED ON DENUDED AREAS NOT OTHERWISE PERMANENTLY

SEDIMENT SHALL BE CONSTRUCTED AS A FIRST STEP IN ANY LAND-DISTURBING ACTIVITY AND SHALL BE MADE FUNCTIONAL BEFORE UPSLOPE LAND DISTURBANCE TAKES PLACE.

5. STABILIZATION MEASURES SHALL BE APPLIED TO EARTHEN STRUCTURES SUCH AS DAMS, DIKES, AND DIVERSIONS IMMEDIATELY AFTER INSTALLATION.

6. SEDIMENT TRAPS AND SEDIMENT BASINS SHALL BE DESIGNED AND CONSTRUCTED BASED UPON THE TOTAL DRAINAGE AREA TO BE SERVED BY THE TRAP OR BASIN.

A. THE MINIMUM STORAGE CAPACITY OF A SEDIMENT TRAP SHALL BE 134 CUBIC YARDS PER ACRE OF DRAINAGE AREA AND THE TRAP SHALL ONLY CONTROL DRAINAGE AREAS LESS THAN THREE ACRES.

B. SURFACE RUNOFF FROM DISTURBED AREAS THAT IS COMPRISED OF FLOW FROM DRAINAGE AREAS GREATER THAN OR EQUAL TO THREE ACRES SHALL BE CONTROLLED BY A SEDIMENT BASIN. THE MINIMUM STORAGE CAPACITY OF A SEDIMENT BASIN SHALL BE 134 CUBIC YARDS PER ACRE OF DRAINAGE AREA. THE OUTFALL SYSTEM SHALL. AT A MINIMUM, MAINTAIN THE STRUCTURAL INTEGRITY OF THE BASIN DURING A 25-YEAR STORM OF 24-HOUR DURATION RUNOFF COEFFICIENTS USED IN RUNOFF CALCULATIONS SHALL CORRESPOND TO A BARE EARTH CONDITION OR THOSE CONDITIONS EXPECTED TO EXIST WHILE THE SEDIMENT BASIN IS UTILIZED.

7. CUT AND FILL SLOPES SHALL BE DESIGNED AND CONSTRUCTED IN A MANNER THAT WILL MINIMIZE EROSION. SLOPES THAT ARE FOUND TO BE ERODING EXCESSIVELY WITHIN ONE YEAR OF PERMANENT STABILIZATION SHALL BE PROVIDED WITH ADDITIONAL SLOPE STABILIZING MEASURES UNTIL THE PROBLEM IS CORRECTED.

8. CONCENTRATED RUNOFF SHALL NOT FLOW DOWN CUT OR FILL SLOPES UNLESS CONTAINED WITHIN AN ADEQUATE TEMPORARY OR PERMANENT CHANNEL, FLUME, OR SLOPE DRAIN STRUCTURE.

9. WHENEVER WATER SEEPS FROM A SLOPE FACE, ADEQUATE DRAINAGE OR OTHER PROTECTION SHALL BE PROVIDED. 10. ALL STORM SEWER INLETS THAT ARE MADE OPERABLE DURING CONSTRUCTION SHALL BE PROTECTED SO THAT

SEDIMENT-LADEN WATER CANNOT ENTER THE CONVEYANCE SYSTEM WITHOUT FIRST BEING FILTERED OR OTHERWISE TREATED TO REMOVE SEDIMENT

11. BEFORE NEWLY CONSTRUCTED STORMWATER CONVEYANCE CHANNELS OR PIPES ARE MADE OPERATIONAL, ADEQUATE OUTLET PROTECTION AND ANY REQUIRED TEMPORARY OR PERMANENT CHANNEL LINING SHALL BE INSTALLED IN BOTH THE CONVEYANCE CHANNEL AND RECEIVING CHANNEL.

12. WHEN WORK IN A LIVE WATERCOURSE IS PERFORMED, PRECAUTIONS SHALL BE TAKEN TO MINIMIZE ENCROACHMENT, CONTROL SEDIMENT TRANSPORT, AND STABILIZE THE WORK AREA TO THE GREATEST EXTENT POSSIBLE DURING CONSTRUCTION. NONERODIBLE MATERIAL SHALL BE USED FOR THE CONSTRUCTION OF CAUSEWAYS AND COFFERDAMS. EARTHEN FILL MAY BE USED FOR THESE STRUCTURES IF ARMORED BY NONERODIBLE COVER MATERIALS.

13. WHEN A LIVE WATERCOURSE MUST BE CROSSED BY CONSTRUCTION VEHICLES MORE THAN TWICE IN ANY SIX-MONTH PERIOD, A TEMPORARY VEHICULAR STREAM CROSSING CONSTRUCTED OF NONERODIBLE MATERIAL SHALL BE PROVIDED.

14. ALL APPLICABLE FEDERAL, STATE, AND LOCAL REQUIREMENTS PERTAINING TO WORKING IN OR CROSSING LIVE

15. THE BED AND BANKS OF A WATERCOURSE SHALL BE STABILIZED IMMEDIATELY AFTER WORK IN THE WATERCOURSE IS

16. UNDERGROUND UTILITY LINES SHALL BE INSTALLED IN ACCORDANCE WITH THE FOLLOWING STANDARDS IN ADDITION TO OTHER APPLICABLE CRITERIA

A. NO MORE THAN 500 LINEAR FEET OF TRENCH MAY BE OPENED AT ONE TIME.

B. EXCAVATED MATERIAL SHALL BE PLACED ON THE UPHILL SIDE OF TRENCHES.

C. EFFLUENT FROM DEWATERING OPERATIONS SHALL BE FILTERED OR PASSED THROUGH AN APPROVED SEDIMENT TRAPPING DEVICE, OR BOTH AND DISCHARGED IN A MANNER THAT DOES NOT ADVERSELY AFFECT FLOWING STREAMS

D. MATERIAL USED FOR BACKFILLING TRENCHES SHALL BE PROPERLY COMPACTED IN ORDER TO MINIMIZE EROSION AND PROMOTE STABILIZATION.

E. RESTABILIZATION SHALL BE ACCOMPLISHED IN ACCORDANCE WITH THIS CHAPTER

F. APPLICABLE SAFETY REQUIREMENTS SHALL BE COMPLIED WITH.

17. WHERE CONSTRUCTION VEHICLE ACCESS ROUTES INTERSECT PAVED OR PUBLIC ROADS, PROVISIONS SHALL BE MADE TO MINIMIZE THE TRANSPORT OF SEDIMENT BY VEHICULAR TRACKING ONTO THE PAVED SURFACE. WHERE SEDIMENT IS TRANSPORTED ONTO A PAVED OR PUBLIC ROAD SURFACE. THE ROAD SURFACE SHALL BE CLEANED. THOROUGHLY AT THE END OF EACH DAY. SEDIMENT SHALL BE REMOVED FROM THE ROADS BY SHOVELING OR SWEEPING AND TRANSPORTED TO A SEDIMENT CONTROL DISPOSAL AREA. STREET WASHING SHALL BE ALLOWED ONLY AFTER SEDIMENT IS REMOVED IN THIS MANNER. THIS PROVISION SHALL APPLY TO INDIVIDUAL DEVELOPMENT LOTS AS WELL AS TO LARGER LAND-DISTURBING ACTIVITIES.

18. ALL TEMPORARY EROSION AND SEDIMENT CONTROL MEASURES SHALL BE REMOVED WITHIN 30 DAYS AFTER FINAL SITE STABILIZATION OR AFTER THE TEMPORARY MEASURES ARE NO LONGER NEEDED, UNLESS OTHERWISE AUTHORIZED BY THE VESCP OR VESMP AUTHORITY. TRAPPED SEDIMENT AND THE DISTURBED SOIL AREAS RESULTING FROM THE DISPOSITION OF TEMPORARY MEASURES SHALL BE PERMANENTLY STABILIZED TO PREVENT FURTHER EROSION AND

19. PROPERTIES AND WATERWAYS DOWNSTREAM FROM DEVELOPMENT SITES SHALL BE PROTECTED FROM SEDIMENT DEPOSITION, EROSION, AND DAMAGE DUE TO INCREASES IN VOLUME, VELOCITY, AND PEAK FLOW RATE OF STORMWATER RUNOFF FOR THE STATED FREQUENCY STORM OF 24-HOUR DURATION IN ACCORDANCE WITH THE FOLLOWING STANDARDS AND CRITERIA. STREAM RESTORATION AND RELOCATION PROJECTS THAT INCORPORATE NATURAL CHANNEL DESIGN CONCEPTS ARE NOT MANMADE CHANNELS AND SHALL BE EXEMPT FROM ANY FLOW RATE CAPACITY AND VELOCITY REQUIREMENTS FOR NATURAL OR MANMADE CHANNELS:

a. CONCENTRATED STORMWATER RUNOFF LEAVING A DEVELOPMENT SITE SHALL BE DISCHARGED DIRECTLY INTO AN ADEQUATE NATURAL OR MANMADE RECEIVING CHANNEL, PIPE, OR STORM SEWER SYSTEM. FOR THOSE SITES WHERE RUNOFF IS DISCHARGED INTO A PIPE OR PIPE SYSTEM, DOWNSTREAM STABILITY ANALYSES AT THE OUTFALL OF THE PIPE OR PIPE SYSTEM SHALL BE PERFORMED.

b. ADEQUACY OF ALL CHANNELS AND PIPES SHALL BE VERIFIED IN THE FOLLOWING MANNER:

(1) THE APPLICANT SHALL DEMONSTRATE THAT THE TOTAL DRAINAGE AREA TO THE POINT OF ANALYSIS WITHIN THE CHANNEL IS 100 TIMES GREATER THAN THE CONTRIBUTING DRAINAGE AREA OF THE PROJECT IN QUESTION; OR

(2) (A) NATURAL CHANNELS SHALL BE ANALYZED BY THE USE OF A TWO-YEAR STORM TO VERIFY THAT STORMWATER WILL NOT OVERTOP CHANNEL BANKS NOR CAUSE EROSION OF CHANNEL BED OR BANKS.

(b) ALL PREVIOUSLY CONSTRUCTED MANMADE CHANNELS SHALL BE ANALYZED BY THE USE OF A 10-YEAR STORM TO VERIFY THAT STORMWATER WILL NOT OVERTOP THE STORMWATER'S BANKS AND BY THE USE OF A TWO-YEAR STORM TO DEMONSTRATE THAT STORMWATER WILL NOT CAUSE EROSION OF CHANNEL BED OR BANKS; AND

(c) PIPES AND STORM SEWER SYSTEMS SHALL BE ANALYZED BY THE USE OF A 10-YEAR STORM TO VERIFY THAT STORMWATER WILL BE CONTAINED WITHIN THE PIPE OR SYSTEM.

IF EXISTING NATURAL RECEIVING CHANNELS OR PREVIOUSLY CONSTRUCTED MANMADE CHANNELS OR PIPES ARE NOT ADEQUATE. THE APPLICANT SHALL:

(1) IMPROVE THE CHANNELS TO A CONDITION WHERE A 10-YEAR STORM WILL NOT OVERTOP THE BANKS AND A TWO-YEAR STORM WILL NOT CAUSE EROSION TO THE CHANNEL, THE BED, OR THE BANKS;

(2) IMPROVE THE PIPE OR PIPE SYSTEM TO A CONDITION WHERE THE 10-YEAR STORM IS CONTAINED WITHIN THE

(3) DEVELOP A SITE DESIGN THAT WILL NOT CAUSE THE PREDEVELOPMENT PEAK RUNOFF RATE FROM A TWO-YEAR STORM TO INCREASE WHEN RUNOFF OUTFALLS INTO A NATURAL CHANNEL OR WILL NOT CAUSE THE PREDEVELOPMENT PEAK RUNOFF RATE FROM A 10-YEAR STORM TO INCREASE WHEN RUNOFF OUTFALLS INTO A MANMADE CHANNEL: OR

(4) PROVIDE A COMBINATION OF CHANNEL IMPROVEMENT, STORMWATER DETENTION, OR OTHER MEASURES THAT IS SATISFACTORY TO THE VESCP OR VESMP AUTHORITY TO PREVENT DOWNSTREAM EROSION.

d. THE APPLICANT SHALL PROVIDE EVIDENCE OF PERMISSION TO MAKE THE IMPROVEMENTS.

ALL HYDROLOGIC ANALYSES SHALL BE BASED ON THE EXISTING WATERSHED CHARACTERISTICS AND THE ULTIMATE DEVELOPMENT CONDITION OF THE SUBJECT PROJECT.

IF THE APPLICANT CHOOSES AN OPTION THAT INCLUDES STORMWATER DETENTION, THE APPLICANT SHALL OBTAIN APPROVAL FROM THE VESCP OR VESMP AUTHORITY FOR A PLAN FOR MAINTENANCE OF THE DETENTION FACILITIES. THE PLAN SHALL SET FORTH THE MAINTENANCE REQUIREMENTS OF THE FACILITY AND THE PERSON RESPONSIBLE FOR PERFORMING THE MAINTENANCE.

OUTFALL FROM A DETENTION FACILITY SHALL BE DISCHARGED TO A RECEIVING CHANNEL, AND ENERGY DISSIPATORS SHALL BE PLACED AT THE OUTFALL OF ALL DETENTION FACILITIES AS NECESSARY TO PROVIDE A STABILIZED TRANSITION FROM THE FACILITY TO THE RECEIVING CHANNEL

h. ALL ON-SITE CHANNELS MUST BE VERIFIED TO BE ADEQUATE

INCREASED VOLUMES OF SHEET FLOWS THAT MAY CAUSE FROSION OR SEDIMENTATION ON ADJACENT PROPERTY SHALL BE DIVERTED TO A STABLE OUTLET, ADEQUATE CHANNEL, PIPE, OR PIPE SYSTEM OR TO A DETENTION FACILITY.

IN APPLYING THESE STORMWATER MANAGEMENT CRITERIA INDIVIDUAL LOTS OR PARCELS IN A RESIDENTIAL COMMERCIAL, OR INDUSTRIAL DEVELOPMENT SHALL NOT BE CONSIDERED TO BE SEPARATE DEVELOPMENT PROJECTS. INSTEAD, THE DEVELOPMENT, AS A WHOLE, SHALL BE CONSIDERED TO BE A SINGLE DEVELOPMENT PROJECT. HYDROLOGIC PARAMETERS THAT REFLECT THE ULTIMATE DEVELOPMENT CONDITION SHALL BE USED IN ALL ENGINEERING CALCULATIONS.

k. ALL MEASURES USED TO PROTECT PROPERTIES AND WATERWAYS SHALL BE EMPLOYED IN A MANNER THAT MINIMIZES IMPACTS ON THE PHYSICAL, CHEMICAL, AND BIOLOGICAL INTEGRITY OF RIVERS, STREAMS, AND OTHER WATERS OF THE STATE.

ANY PLAN APPROVED PRIOR TO JULY 1, 2014, THAT PROVIDES FOR STORMWATER MANAGEMENT THAT ADDRESSES ANY FLOW RATE CAPACITY AND VELOCITY REQUIREMENTS FOR NATURAL OR MANMADE CHANNELS SHALL SATISFY THE FLOW RATE CAPACITY AND VELOCITY REQUIREMENTS FOR NATURAL OR MANMADE CHANNELS IF THE PRACTICES ARE DESIGNED TO (I) DETAIN THE WATER QUALITY VOLUME AND TO RELEASE IT OVER 48 HOURS; (II) DETAIN AND RELEASE OVER A 24-HOUR PERIOD THE EXPECTED RAINFALL RESULTING FROM THE ONE YEAR, 24-HOUR STORM; AND (III) REDUCE THE ALLOWABLE PEAK FLOW RATE RESULTING FROM THE 1.5-YEAR, TWO-YEAR, AND 10-YEAR 24-HOUR STORMS TO A LEVEL THAT IS LESS THAN OR EQUAL TO THE PEAK FLOW RATE FROM THE SITE ASSUMING THE SITE WAS IN A GOOD FORESTED CONDITION, ACHIEVED THROUGH MULTIPLICATION OF THE FORESTED PEAK FLOW RATE BY A REDUCTION FACTOR THAT IS EQUAL TO THE RUNOFF VOLUME FROM THE SITE WHEN THE SITE WAS IN A GOOD FORESTED CONDITION DIVIDED BY THE RUNOFF VOLUME FROM THE SITE IN THE SITE'S PROPOSED CONDITION, AND SHALL BE EXEMPT FROM ANY FLOW RATE CAPACITY AND VELOCITY REQUIREMENTS FOR NATURAL OR MANMADE CHANNELS AS DEFINED IN ANY REGULATIONS PROMULGATED PURSUANT TO § 62.1-44.15:28 OF THE CODE OF VIRGINIA (VESMA) OR § 62.1-44.15:54 OR 62.1-44.15:65 OF THE CODE OF VIRGINIA (ESCL).

FOR PLANS APPROVED ON AND AFTER JULY 1, 2014, THE FLOW RATE CAPACITY AND VELOCITY REQUIREMENTS OF § 62.1-44.15:52 A OF THE CODE OF VIRGINIA (ESCL) AND THIS SUBDIVISION 19 SHALL BE SATISFIED BY COMPLIANCE WITH WATER QUANTITY REQUIREMENTS IN THE VESMA AND ATTENDANT REGULATIONS, UNLESS SUCH LAND-DISTURBING ACTIVITIES (I) ARE IN ACCORDANCE WITH PROVISIONS FOR TIME LIMITS ON APPLICABILITY OF APPROVED DESIGN CRITERIA IN 9VAC25-875-480 OR GRANDFATHERING IN 9VAC25-875-490, IN WHICH CASE THE FLOW RATE CAPACITY AND VELOCITY REQUIREMENTS OF § 62.1-44.15:52 A OF THE CODE OF VIRGINIA (ESCL) SHALL APPLY; OR (II) ARE EXEMPT PURSUANT TO § 62.1-44.15:34 G 2 OF THE CODE OF VIRGINIA (VESMA).

n. COMPLIANCE WITH THE WATER QUANTITY MINIMUM STANDARDS SET OUT IN 9VAC25-875-600 SHALL BE DEEMED TO SATISFY THE REQUIREMENTS OF THIS SUBDIVISION 19.

CITY OF RICHMOND EROSION AND SEDIMENT CONTROL NOTES

- 1. PERMANENT OR TEMPORARY SOIL STABILIZATION SHALL BE APPLIED TO DENUDED AREAS WITHIN SEVEN DAYS AFTER FINAL GRADE IS REACHED ON ANY PORTION OF THE SITE. TEMPORARY SOIL STABILIZATION SHALL BE APPLIED WITHIN SEVEN DAYS TO DENUDED AREAS THAT MAY NOT BE AT FINAL GRADE BUT WILL REMAIN, DORMANT (UNDISTURBED) FOR LONGER THAN 14 DAYS. PERMANENT STABILIZATION SHALL BE APPLIED TO AREAS THAT ARE TO BE LEFT DORMANT FOR MORE THAN ONE YEAR.
- 2. EXCESS EXCAVATION DISPOSED OF OFF THE SITE SHALL BE DISPOSED OF IN ACCORDANCE WITH THE VIRGINIA EROSION AND SEDIMENT CONTROL HANDBOOK.
- 3. EROSION AND SEDIMENT CONTROLS SHALL BE INSTALLED IN ACCORDANCE WITH VIRGINIA EROSION AND SEDIMENT CONTROL HANDBOOK AND SHALL BE PLACED PRIOR TO OR AS THE FIRST STEP OF THE LAND DISTURBING ACTIVITY
- 4. EROSION AND SEDIMENT CONTROLS SHALL BE MAINTAINED SO THAT THE SEDIMENT CARRYING RUNOFF FROM THE SITE WILL NOT ENTER STORM DRAINAGE FACILITIES.
- 5. EROSION AND SEDIMENT CONTROLS SHALL BE MAINTAINED UNTIL THE DISTURBED AREA IS STABILIZED.
- 6. PROPERTIES ADJOINING THE SITE SHALL BE KEPT CLEAN OF MUD OR SILT CARRIED FROM THE SITE BY VEHICULAR TRAFFIC OR RUNOFF.
- 7 THE DISPOSAL OF WASTE MATERIALS REMOVED FROM FROSION AND SEDIMENT CONTROL FACILITIES AND THE DISPOSAL OF THESE FACILITIES SHALL BE IN ACCORDANCE WITH THE VIRGINIA EROSION AND SEDIMENT CONTROL
- 8. STABILIZATION MEASURES SHALL BE APPLIED TO EARTHEN STRUCTURES SUCH AS DAMS, DIKES AND DIVERSIONS IMMEDIATELY AFTER INSTALLATION.
- 9. DURING CONSTRUCTION OF THE PROJECT, SOIL STOCKPILES SHALL BE STABILIZED OR PROTECTED WITH SEDIMENT TRAPPING MEASURES. THE APPLICANT IS RESPONSIBLE FOR THE TEMPORARY PROTECTION AND PERMANENT STABILIZATION OF ALL SOIL STOCKPILES ON SITE AS WELL AS SOIL INTENTIONALLY TRANSPORTED FROM THE PROJECT SITE

GENERAL EROSION AND SEDIMENT CONTROL NOTES

ES-1: UNLESS OTHERWISE INDICATED, ALL VEGETATIVE AND STRUCTURAL EROSION AND SEDIMENT CONTROL PRACTICES WILL BE CONSTRUCTED AND MAINTAINED ACCORDING TO MINIMUM STANDARDS AND SPECIFICATIONS OF THE VIRGINIA STORMWATER MANAGEMENT HANDBOOK AND VIRGINIA REGULATIONS 9VAC25-840 FROSION AND SEDIMENT CONTROL REGULATIONS.

ES-2: THE PLAN APPROVING AUTHORITY MUST BE NOTIFIED ONE WEEK PRIOR TO THE PRE-CONSTRUCTION CONFERENCE. ONE WEEK PRIOR TO THE COMMENCEMENT OF LAND DISTURBING ACTIVITY, AND ONE WEEK PRIOR TO THE FINAL

ES-3: ALL EROSION AND SEDIMENT CONTROL MEASURES ARE TO BE PLACED PRIOR TO OR AS THE FIRST STEP IN

ES-5: PRIOR TO COMMENCING LAND DISTURBING ACTIVITIES IN AREAS OTHER THAN INDICATED ON THESE PLANS

ES-4: A COPY OF THE APPROVED EROSION AND SEDIMENT CONTROL PLAN SHALL BE MAINTAINED ON THE SITE AT ALL

(INLCUDING, BUT NOT LIMITED TO, OFF-SITE BORROW OR WASTE AREAS), THE CONTRACTOR SHALL SUBMIT A SUPPLEMENTARY EROSION CONTROL PLAN TO THE OWNER FOR REVIEW AND APPROVAL BY THE PLAN APPROVING

ES-6: THE CONTRACTOR IS RESPONSIBLE FOR INSTALLATION OF ANY ADDITIONAL EROSION CONTROL MEASURES NECESSARY TO PREVENT EROSION AND SEDIMENTATION AS DETERMINED BY THE PLAN APPROVING AUTHORITY. ES-7: ALL DISTURBED AREAS ARE TO DRAIN TO APPROVED SEDIMENT CONTROL MEASURES AT ALL TIMES DURING LAND DISTURBING ACTIVITIES AND DURING SITE DEVELOPMENT UNTIL FINAL STABILIZATION IS ACHIEVED.

ES-8: DURING DEWATERING OPERATIONS, WATER WILL BE PUMPED INTO AN APPROVED FILTERING DEVICE.

ES-9: THE CONTRACTOR SHALL INSPECT ALL EROSION CONTROL MEASURES PERIODICALLY AND AFTER EACH RUNOFF-PRODUCING RAINFALL EVENT. ANY NECESSARY REPAIRS OR CLEANUP TO MAINTAIN THE EFFECTIVENESS OF THE EROSION CONTROL DEVICES SHALL BE MADE IMMEDIATELY.

EROSION AND SEDIMENT CONTROL NARRATIVE:

THIS PROJECT INCLUDES THE CONSTRUCTION OF A NEW WALKWAY, GATHERING AND PLAY AREAS AND STORMWATER BEST MANAGEMENT PRACTICES. THE LIMITS OF DISTURBANCE IS 0.92 AC.

OPPOSITE S ALLEN AVE IS A SCHOOL.

THE SITE IS WITHIN THE GROUNDS OF A SCHOOLYARD, AND CONSISTS OF ASPHALT WALKWAYS, AN ASPHALT BASKETBALL COURT, A GAZEBO, A GREENHOUSE, TWO PLAYGROUNDS, AND LAWN AREA. THE SITE DRAINS VIA SHEET FLOW FROM THE NORTHEAST TO SOUTHWEST PROPERTY LINE ALONG COLORADO AVE, ULTIMATELY DRAINING INTO THE CITY OF RICHMOND COMBINED SEWER SYSTEM.

HE WESTERN PROPERTY LINE IS BOUND BY S MEADOW ST. THE NORTHERN PROPERTY LINE IS BOUND BY AMELIA ST. THE EASTERN PROPERTY LINE IS BOUND BY S ALLEN AVE. THE SOUTHERN PROPERTY LINE IS BOUND BY COLORADO AVE. THE NEIGHBORING PROPERTIES OPPOSITE THE RIGHT OF WAY ON THREE SIDES OF THE PROPERTY ARE RESIDENTIAL, AND

NO OFFSITE AREAS WILL BE DISTURBED AS PART OF CONSTRUCTION.

SOILS
THE UNITED STATES DEPARTMENT OF AGRICULTURE NATURAL RESOURCES CONSERVATION SERVICE SOILS MAP REPORTS THE SITE SOILS AS CONSISTING OF:

4D - APPLING WEDOWEE COMPLEX, 12 TO 20 PERCENT SLOPES, WELL DRAINED, HYDROLOGIC SOIL GROUP B 37B - TURBEVILLE-URBAN LAND COMPLEX, 2 TO 6 PERCENT SLOPES, WELL DRAINED, HYDROLOGIC SOIL GROUP B

HERE ARE NO CRITICAL EROSION AREAS ON THE SITE.

ACCORDING TO THE U.S. FEDERAL EMERGENCY MANAGEMENT (FEMA) FLOOD INSURANCE RATE MAP, THERE ARE NO FEMA FLOODPLAINS ON THE PROPOSED SITE.

THERE ARE NO WETLANDS LOCATED WITHIN THE LIMITS OF THIS SITE.

UNLESS OTHERWISE INDICATED, ALL VEGETATIVE AND STRUCTURAL EROSION AND SEDIMENT CONTROL PRACTICES SHALL BE CONSTRUCTED AND MAINTAINED ACCORDING TO MINIMUM STANDARDS AND SPECIFICATIONS OF THE CURRENT EDITION OF THE VIRGINIA STORMWATER MANAGEMENT HANDBOOK. THE MINIMUM STANDARDS OF THE VIRGINIA STORMWATER MANAGEMENT HANDBOOK SHALL BE ADHERED TO UNLESS OTHERWISE WAIVED OR APPROVED BY A VARIANCE BY LOCAL AUTHORITIES HAVING JURISDICTION.

STRUCTURAL PRACTICES

- TEMPORARY CONSTRUCTION ENTRANCE C-SCM-02 A TEMPORARY CONSTRUCTION ENTRANCE SHALL BE PROVIDED AT THE LOCATION INDICATED ON THE PLANS. IT IS IMPERATIVE THAT THIS MEASURE BE MAINTAINED THROUGHOUT CONSTRUCTION. ITS PURPOSE IS TO REDUCE THE AMOUNT OF MUD TRANSPORTED ONTO PAVED PUBLIC ROADS BY MOTOR VEHICLES OR RUNOFF
- SILT FENCE BARRIER C-PCM-04 SILT FENCE SEDIMENT BARRIERS SHALL BE INSTALLED DOWNSLOPE OF AREAS WITH MINIMAL GRADES TO FILTER SEDIMENT-LADEN RUNOFF FROM SHEET FLOW AS INDICATED. ITS PURPOSE IS TO INTERCEPT SMALL AMOUNTS OF SEDIMENT FROM DISTURBED AREAS AND PREVENT SEDIMENT FROM LEAVING
- THE SITE. STORM DRAIN INLET PROTECTION C-SCM-04 - STONE FILTERS SHALL BE PLACED AT THE INLET OF ALL DRAINAGE STRUCTURES AS INDICATED ON PLANS. ITS PURPOSE IS TO PREVENT SEDIMENT FROM ENTERING THE STORM DRAINAGE SYSTEM PRIOR TO PERMANENT STABILIZATION.
- CULVERT INLET PROTECTION C-SCM-05 CULVERT INLET PROTECTION SHALL BE USED TO PREVENT SEDIMENT FROM ENTERING, ACCUMULATING, AND BEING TRANSPORTED BY A CULVERT AND ASSOCIATED DRAINAGE SYSTEM BEFORE THE PERMANENT STABILIZATION OF A DISTURBED PROJECT AREA. OUTLET PROTECTION C-ECM-15 - STRUCTURALLY LINED APRONS OR OTHER ACCEPTABLE ENERGY DISSIPATING

DEVICES SHALL BE PLACED AT ALL OUTLETS OF PIPES OR PAVED CHANNEL SECTIONS.

TREE PRESERVATION AND PROTECTION C-SSM-01 - DESIGNATED TREES SHALL BE PROTECTED FROM CONSTRUCTION ACTIVITIES VIA FENCING AND ARMORING. THE PURPOSE IS TO ENSURE THE SURVIVAL OF DESIRABLE TREES AND PROTECT THEIR ROOT ZONES FROM CONSTRUCTION EQUIPMENT OR SOIL COMPACTION BY VEHICULAR TRAFFIC.

- TOPSOIL (TEMPORARY STOCKPILE) C-SSM-02 TOPSOIL SHALL BE STRIPPED FROM AREAS TO BE GRADED AND STOCKPILED FOR LATER SPREADING. STOCKPILE LOCATIONS SHALL BE LOCATED ONSITE AND SHALL BE STABILIZED WITH TEMPORARY SILT FENCE AND VEGETATION.
- SOIL STABILIZATION BLANKETS & MATTING C-SSM-05 A PROTECTIVE COVERING BLANKET OR SOIL STABILIZATION MAT SHALL BE INSTALLED ON PREPARED PLANTING AREAS OF CHANNELS TO PROTECT AND PROMOTE VEGETATION ESTABLISHMENT AND REINFORCE ESTABLISHED TURF.
- TEMPORARY SEEDING C-SSM-09 ALL DENUDED AREAS WHICH WILL BE LEFT DORMANT FOR MORE THAN 30 DAYS SHALL BE SEEDED WITH FAST GERMINATING TEMPORARY VEGETATION IMMEDIATELY FOLLOWING GRADING OF THOSE AREAS. SELECTION OF THE SEED MIXTURE SHALL DEPEND ON THE TIME OF YEAR IT IS APPLIED.
- PERMANENT SEEDING C-SSM-10 FOLLOWING GRADING ACTIVITIES, ESTABLISH PERENNIAL VEGETATIVE COVER BY PLANTING SEED TO REDUCE EROSION, STABILIZE DISTURBED AREAS, AND ENHANCE NATURAL BEAUTY.

MAINTENANCE OF EROSION AND SEDIMENT CONTROL FACILITIES ALL MAINTENANCE OF TEMPORARY AND PERMANENT EROSION AND SEDIMENT CONTROL FACILITIES SHALL BE CARRIED

OUT IN ACCORDANCE WITH THE VIRGINIA EROSION AND SEDIMENT CONTROL REGULATIONS (9VAC25-840-40). DURING THE PERIOD THAT THE PROJECT SITE IS UNDER CONSTRUCTION, THE CONTRACTOR WILL BE RESPONSIBLE FOR MAINTENANCE OF THE EROSION AND SEDIMENT CONTROL FACILITIES. THE CONTRACTOR WILL INSPECT EROSION AND SEDIMENT CONTROL FACILITIES ON A REGULAR BASIS, ESPECIALLY AFTER PERIODS OF HEAVY RAINFALL. ANY DAMAGE DISCOVERED WILL BE REPAIRED PROMPTLY BY THE CONTRACTOR. FURTHERMORE, A READILY AVAILABLE SUPPLY OF EROSION AND SEDIMENT CONTROL MATERIALS WILL BE MAINTAINED BY THE CONTRACTOR AT ALL TIMES. CONSTRUCTION ENTRANCE - THE ENTRANCE SHALL BE MAINTAINED IN A CONDITION WHICH WILL PREVENT

- TRACKING OR FLOW OF MUD ONTO PUBLIC RIGHTS OF WAY. THIS MAY REQUIRE PERIODIC TOP DRESSING WITH ADDITIONAL STONE OR THE WASHING AND REWORKING OF EXISTING STONE AS CONDITIONS DEMAND AND REPAIR AND/OR CLEANOUT OF ANY STRUCTURES USED TO TRAP SEDIMENT. SILT FENCE - SILT FENCES SHALL BE INSPECTED IMMEDIATELY AFTER EACH RAINFALL AND AT LEAST DAILY DURING
- PROLONGED RAINFALL. ANY REQUIRED REPAIRS SHALL BE MADE IMMEDIATELY. SEDIMENT DEPOSITS SHOULD BE REMOVED AFTER EACH STORM EVENT. THEY MUST BE REMOVED WHEN DEPOSITS REACH APPROXIMATELY ONE-HALF THE HEIGHT OF THE BARRIER. CLOSE ATTENTION SHALL BE PAID TO THE REPAIR OF DAMAGED SILT FENCE RESULTING FROM END RUNS AND UNDERCUTTING. INLET PROTECTION - THE STRUCTURE SHALL BE INSPECTED AFTER EACH RAIN AND REPAIRS MADE AS NECESSARY. SEDIMENT SHALL BE REMOVED AND THE TRAP RESTORED TO ITS ORIGINAL DIMENSIONS WHEN THE SEDIMENT HAS
- ACCUMULATED TO ONE HALF THE DESIGN DEPTH OF THE TRAP. REMOVED SEDIMENT SHALL BE DEPOSITED IN A SUITABLE AREA AND IN SUCH A MANNER THAT IT WILL NOT ERODE. ALL NON-PAVED AREAS DISTURBED BY CONSTRUCTION SHALL BE STABILIZED WITH PERMANENT SEEDING IMMEDIATELY FOLLOWING FINISHED GRADING. SEEDING SHALL BE IN ACCORDANCE WITH STD. & SPEC. C-SSM-10, PERMANENT SEEDING. SEED TYPE SHALL BE AS SPECIFIED FOR "GENERAL SLOPE (3H:1V OR LESS)" IN TABLE C-SSM-10-7

OF THE HANDBOOK FOR SLOPES LESS THAN 3:1. FOR SLOPES GREATER THAN 3:1, SEED TYPE SHALL BE AS SPECIFIED FOR "LOW-MAINTENANCE SLOPE (>3:1) OR INACCESSIBLE AREA" IN TABLE C-SSM-10-7 OF THE HANDBOOK. MULCH (STRAW OR FIBER) SHALL BE USED ON ALL SEEDED SURFACES. IN ALL SEEDING OPERATIONS SEED, FERTILIZER AND LIME SHALL BE APPLIED PRIOR TO MULCHING. SHOULD IT BE NECESSARY TO PUMP STORMWATER DURING CONSTRUCTION BECAUSE THE GRADING WILL NOT ALLOW

FOR GRAVITY FLOW, THEN THE PUMPED STORMWATER MUST BE FILTERED THROUGH A SILT SACK OR SIMILAR SEDIMENT TRAPPING DEVICE BEFORE DRAINING INTO THE ADJACENT STORM SEWER SYSTEM. STORMWATER RUNOFF CONSIDERATIONS

DURING DEMOLITION AND THROUGHOUT CONSTRUCTION, STORMWATER RUNOFF WILL SHEET FLOW TO EXISTING OR PROPOSED STORMWATER INLETS OR CONVEYANCES, WITH INLET PROTECTION, AND OTHER PERIMETER CONTROLS SUCH AS SILT FENCE. THROUGH THESE MEASURES, THE RUNOFF WILL BE FILTERED PRIOR TO RELEASE INTO THE EXISTING STORM SEWER SYSTEM.

SEQUENCE OF INSTALLATION PHASE I (SEE SHEET C-2.0)

- 1. A PRE-CONSTRUCTION MEETING IS REQUIRED WITH THE CITY OF RICHMOND E&S INSPECTOR, CONTRACTOR, OWNER, AND ENGINEER. THIS MEETING SHALL TAKE PLACE AT THE COUNTY OFFICE BUILDING. CLEARING LIMITS MUST BE FLAGGED PRIOR TO THE MEETING WITH ONE (1) WEEK OF NOTICE INSTALL CONSTRUCTION ENTRANCE. SHOULD THE TEMPORARY CONSTRUCTION ENTRANCE NOT BE MAINTAINED
- PROPERLY OR AN EXCESSIVE AMOUNT OF SOIL BE TRACKED ONTO THE PUBLIC ROADWAY, THEN A PAVED CONSTRUCTION ENTRANCE, WATER TANKER TRUCK WITH PRESSURE WASHERS, AND SETTING AREA MAY BE REQUIRED BY THE EROSION CONTROL INSPECTOR. INSTALL PERIMETER SILT FENCE AND TREE PROTECTION ALONG LIMITS OF GRADING.
- AFTER ALL MEASURES ARE IN PLACE, CONTRACTOR TO COORDINATE INSPECTIONS AS REQUIRED BY THE CITY OF
- CLEAR THE PROJECT AREA AND BRING SITE UP TO GRADE.
- SEED ALL DENUDED AREAS PER VIRGINIA STORMWATER HANDBOOK CONSTRUCTION BMP STANDARDS.
- AS CONTRACTOR BRINGS SITE UP TO GRADE CONSTRUCTION ENTRANCE STABILIZATION MAY BE REQUIRED IF DEEMED NECESSARY
- MAINTAIN POSITIVE DRAINAGE TO PERIMETER CONTROLS. ENSURE THAT NO SECTION OF SILT FENCE IS OVERLOADED. SEED ALL DENUDED AREAS PER VESCH STANDARDS.
- STABILIZE ALL UPSTREAM AREAS PRIOR TO INSTALLATION AND CONNECTION OF DRAINAGE THROUGH BIORETENTION PRACTICES. FOLLOWING STABILIZATION AND CONNECTION OF ALL PERMANENT STORMWATER MEASURES, REMOVE PHASE 1

EROSION CONTROL MEASURES.

SEQUENCE OF CONSTRUCTION

EXECUTED BY THE CONTRACTOR

THE FOLLOWING OUTLINES THE GENERAL CONSTRUCTION SEQUENCE THAT WILL BE EMPLOYED DURING THE SITE CONSTRUCTION STAGE:

- 1. A PRE-CONSTRUCTION MEETING MUST TAKE PLACE PRIOR TO ANY LAND DISTURBING ACTIVITIES. THE OWNER'S REPRESENTATIVE, ENGINEER, EROSION CONTROL INSPECTOR, AND CONTRACTOR MUST BE PRESENT AT THIS MEETING. THE SITE WORK CONTRACTOR SHALL GIVE THE DEQ INSPECTOR TWO WORKING DAYS NOTIFICATION.
- A CERTIFIED RESPONSIBLE LAND DISTURBER (RLD) IS REQUIRED DURING ALL STAGES OF CONSTRUCTION, FROM THE INITIAL LAND DISTURBANCE THROUGH FINAL SITE STABILIZATION. THE NAME OF THE PROJECT RLD MUST BE PROVIDED BEFORE ANY LAND DISTURBANCE MAY BEGIN.
- THE SITE CONTRACTOR WILL BE RESPONSIBLE FOR SCHEDULING AND CONDUCTING ALL NECESSARY INSPECTIONS WITH THE APPROPRIATE LOCAL AND STATE OFFICIALS. COORDINATION WITH THE APPROPRIATE ENTITIES WILL BE
- 4. PRIOR TO STARTING ANY OTHER WORK ON THE SITE, THE CONTRACTOR SHALL NOTIFY APPROPRIATE AGENCIES AND SHALL INSTALL EROSION CONTROL MEASURES AS SHOWN ON THE PLANS AND AS IDENTIFIED IN FEDERAL, STATE, AND LOCAL APPROVAL DOCUMENTS PERTAINING TO THIS PROJECT. THE SITE WORK CONTRACTOR SHALL

GIVE THE EROSION AND SEDIMENT CONTROL INSPECTOR 1 WEEK NOTIFICATION PRIOR TO COMMENCING WORK.

- 5. DURING PHASE I EROSION AND SEDIMENT CONTROL WORK, THE CONTRACTOR SHALL INSTALL PERIMETER CONTROLS SUCH AS SILT FENCING, SAFETY FENCING, CONSTRUCTION ENTRANCES, LAY DOWN AREAS, AND INLET PROTECTION AS SHOWN ON THE FROSION CONTROL PLAN LAND DISTURBANCE WITHIN THE LIMITS OF DISTURBANCE MAY NOT OCCUR UNTIL THE INITIAL ESC MEASURES INSTALLATION HAS BEEN APPROVED BY THE ENVIRONMENTAL INSPECTOR.
- 6. CONTRACTOR SHALL INSPECT AND MAINTAIN EROSION CONTROL MEASURES, AND REMOVE SEDIMENT THEREFROM ON A WEEKLY BASIS AND WITHIN 12 HOURS AFTER EACH STORM EVENT AND DISPOSE OF THE SEDIMENTS IN AN UPLAND AREA SUCH THAT THEY DO NOT ENCUMBER OTHER DRAINAGE STRUCTURES AND PROTECTED AREAS. CONTRACTOR IS FULLY RESPONSIBLE TO CONTROL CONSTRUCTION SUCH THAT SEDIMENTATION SHALL NOT AFFECT REGULATORY PROTECTED AREAS, WHETHER SUCH SEDIMENTATION IS CAUSED BY WATER, WIND, OR
- 7. CONTRACTOR SHALL PROVIDE SURFACE DRAINAGE FROM GRADING OPERATIONS TO ADJACENT STORM SEWER STRUCTURES IF POSSIBLE. INLET PROTECTION SHALL BE PROVIDED FOR ALL DOWNSTREAM STRUCTURES AND INSPECTED AND MAINTAINED ON A WEEKLY BASIS.
- 8. DEMOLITION OF THE SITE MAY BEGIN AFTER PHASE 1 EROSION CONTROL MEASURES ARE INSTALLED AND APPROVED BY THE INSPECTOR
- 9. UPON COMPLETION OF SITE DEMOLITION, ROUGH GRADING OPERATIONS MAY COMMENCE. DEWATERING DEVICES TO BE INSTALLED AS NECESSARY TO REMOVE TRAPPED WATER FROM THE EXCAVATED AREA. REFER TO THE PUMPING OF STORMWATER NOTE ON WITH THE EROSION CONTROL DETAILS.
- 10. BEGIN INSTALLATION OF PROPOSED STORM SEWER PIPES AND INLETS. CONTRACTOR SHALL COORDINATE UTILITY CONNECTIONS WITH THE CITY OF RICHMOND DEPARTMENT OF PUBLIC UTILITIES.
- 11. INSTALL DRAINAGE STRUCTURES AS REQUIRED.
- 12. INSTALL SITE HARDSCAPE AND FINE GRADE SIDEWALKS
- 13. STABILIZE ALL AREAS AS REQUIRED IN PLANS AND SPECIFICATIONS.
- 14. INSTALL BIORETENTIONS ACCORDING TO PLANS AND SPECIFICATIONS ONCE ALL UPSTREAM DRAINAGE AREAS HAVE BEEN STABILIZED
- 15. APPLY TEMPORARY SEEDING WITHIN 7 DAYS TO DENUDED AREAS THAT MAY NOT BE AT FINAL GRADE BUT WILL REMAIN DORMANT FOR PERIODS LONGER THAN 14 DAYS. APPLY PERMANENT SEEDING TO AREAS THAT WILL REMAIN DORMANT FOR MORE THAN 1 YEAR.
- 16. ALL EROSION AND SEDIMENT CONTROL DEVICES SHALL REMAIN IN OPERATION AND MAINTAINED UNTIL CONSTRUCTION OPERATIONS ARE COMPLETE. TEMPORARY ESC MEASURES SHALL BE REMOVED WITHIN 30 DAYS AFTER THEY ARE NO LONGER NEEDED. UPON COMPLETION OF CONSTRUCTION AND ESTABLISHMENT OF PERMANENT GROUND COVER, CONTRACTOR SHALL REMOVE AND DISPOSE OF EROSION CONTROL MEASURES AND CLEAN SEDIMENT AND DEBRIS FROM SITE PAVEMENTS AND SEWER SYSTEMS



SOURCE: USDA NRSO





OWNER/CLIENT ALLIANCE FOR THE CHESAPEAKE BAY

PROJECT MANAGER NEAL FRIEDMAN

CIVIL ENGINEERING NITSCH ENGINEERIN

SURVEY NYFELER SURVEY

REV. **07.17.2025 PERMIT**

KEY PLAN:NTS

MA PROJECT NO. 2402 **AMELIA STREET SCHOOL ACCESSIBILITY TRAIL**

1821 AMELIA STREET

CITY OF RICHMOND. VA

DATE 03.04.2025

	Table C-SSM-09-2 Liming								
TS)		Liming Requirements for Temporary Sites							
	pH Test	Recommended Application of Agricultural Limestone							
	below 4.2	3 tons per acre							
	4.2 to 5.2	2 tons per acre							
	5.2 to 6	1 ton per acre							

Source: Va. DSWC

Table C-SSM-09-3 Plant Material for Temporary Seeding								
Acceptable	Temporary Seeding Plant Materials "Quick Reference"	for all Regions"						
Planting Dates	Species	Rate (pounds per acre)						
Sept. 1 – Feb. 15	50/50 Mix of annual ryegrass (Lolium multi-florum) & cereal (winter) rye (Secale cereale)	50 – 100						
Feb. 16 – Apr. 30*	Annual ryegrass (Lolium multi-florum)	60 – 100						
May 1* – Aug. 31	German millet (Setaria italica)	50						

* The shift date for annual rye to German millet should be April 15 for the Piedmont and Coastal Plain, rather than May 1.

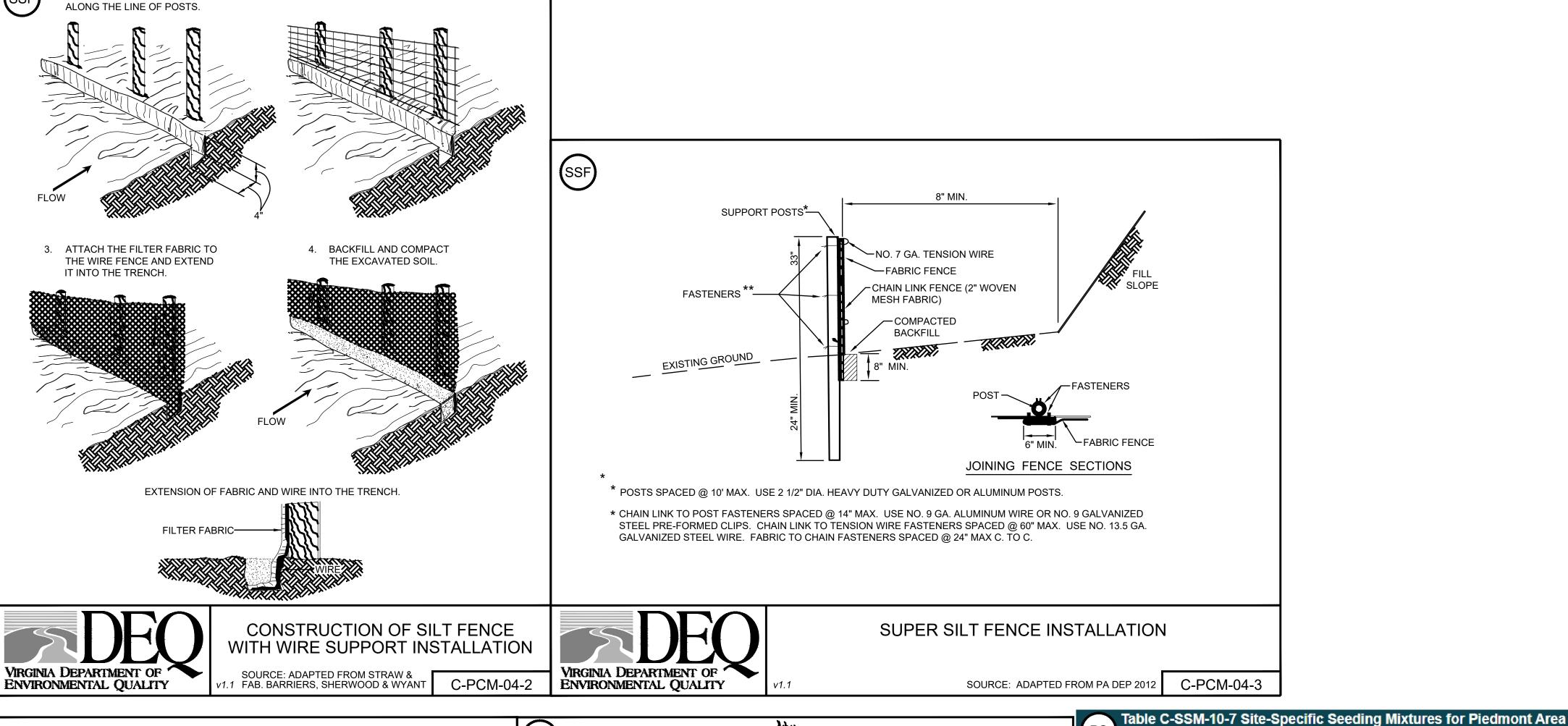
	Tal	ole C	SSN	1-09	4 Te	mpo	orar	y See	eding Applications
Temporary Seeding Plant Materials, Seeding Rates, and Dates									
		ding ate		North	a	5	Sout	h ^b	
Species	lbs./ acre	lbs./ 1,000 ft ²	_	_	8/15 - 11/1	_	_	9/1 - 11/15	Plant Characteristics
Oats (Avena sativa)	50 – 100	2.0	Х	-	_	Х	-	-	Use spring varieties (e.g., Noble).
Rye ^d (Secale cereale)	50 – 110	2.5	Х	-	х	Х	-	х	Use for late fall seedings, winter cover. Tolerates cold and low moisture.
German millet (Setaria italica)	50	1.0	-	X*	-	-	X*	-	Warm-season annual. Dies at first frost. May be added to summer mixes.
Annual ryegrass ^c (<i>Lolium multi-</i> <i>florum</i>)	60	1.5	X*	-	Х	X*	-	х	May be added in mixes. Will mow out of most stands.
Korean Lespedeza ^c (Lespedeza stipulacea)	25	1.5	х	х	-	х	x	-	Warm-season annual legume. Tolerates acid soils. May only be used in a mix with another grass (e.g., annual or cereal rye); it is not suitable as a pure seeding for this purpose.
 a. Northern Piedmont and Mountain regions. See Figure C-ENV-01-1 and Figure C-ENV-01-2. b. Southern Piedmont and Coastal Plain. 									

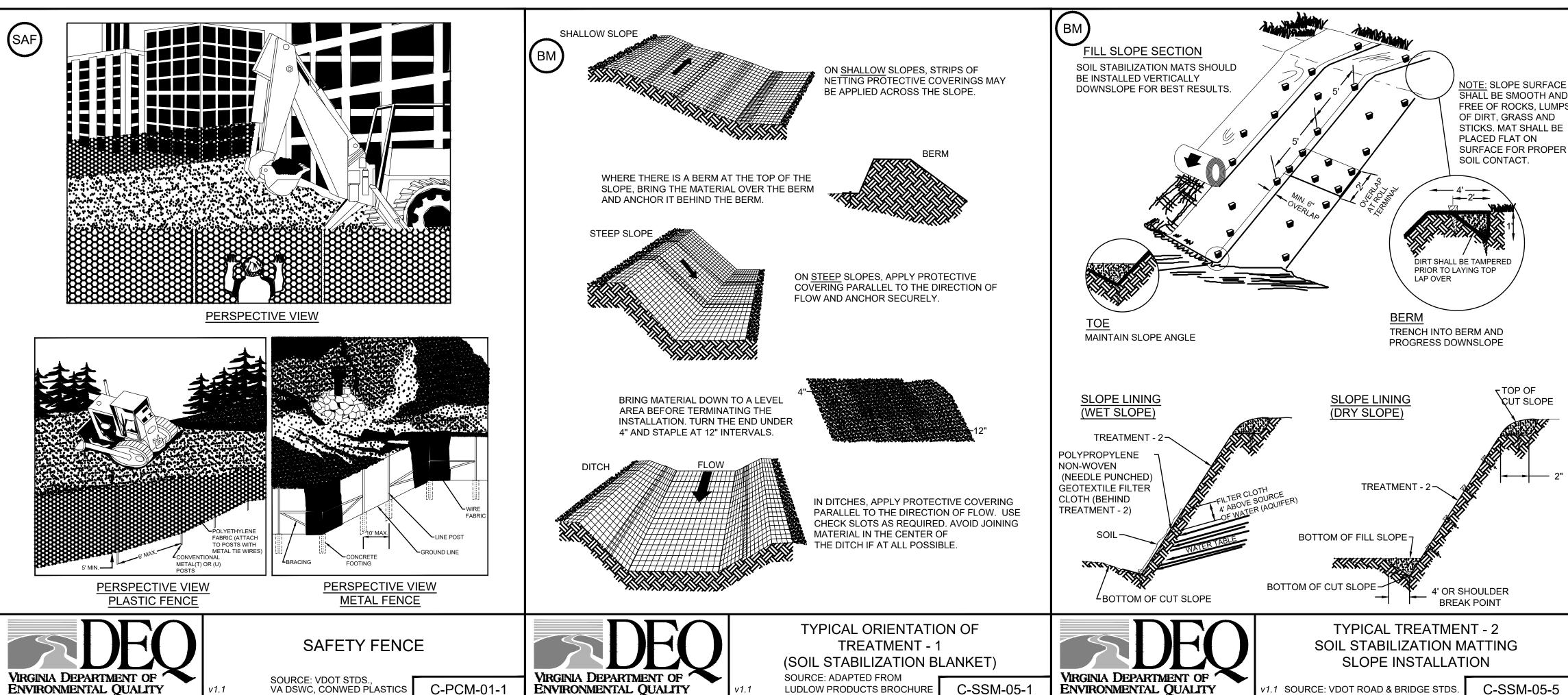
* The shift date for annual rye to German millet should be April 15 for the Piedmont and Coastal Plain, rather than May 1.

May be used as a cover crop with spring seeding.

 d. May be used as a cover crop with fall seeding. Xe.May not be planted between these dates.

-f May be planted between these dates.





SET POSTS AND EXCAVATE A 4"X4" TRENCH UPSLOPE

2. STAPLE WIRE FENCING TO THE



MARVEL

OWNER/CLIENT ALLIANCE FOR THE CHESAPEAKE BAY

PROJECT MANAGER **NEAL FRIEDMAN**

SURVEY **NYFELER SURVEY**

CIVIL ENGINEERING **NITSCH ENGINEERING**

(pounds per acre)

150 - 200

150 - 200

50 - 75

10 - 20

10 - 20

30 - 40

50 - 75

5 - 10

15 - 20

10 - 15

20-30

95-100%

0-5%

0-5%

100%

Site Condition

Commercial or Residential

High-Maintenance Lawn

General Slope (3H:1V or

Low-Maintenance Slope (>

3:1) or Inaccessible Area***

February 16 through April - annual/cereal rye

May 1 through August 15 – foxtail/German millet

August 16 through February 15 – annual/cereal rye

*** Increase seeding rate if KY-31 is used rather than VCIA/VDOT improved varieties.

revegetation professionals. All seed rates expressed as PLS (Pure Live Seed; see Table C-SSM-10-9).

Minimum-Care Lawn

Seed Mix

Turf-Type Tall Fescue

Improved Perennial Ryegrass

Kentucky Bluegrass

Improved (VCIA) Turf-Type Tall Fescue

Tall Fescue***

Red Top or Red/Hard Fescue

White Clover and/or Birdsfoot Trefoil**

Seasonal Nurse Crop*

Tall Fescue

Red Top and/or Hard Fescue

White Clover and/or Birdsfoot Trefoil**

Annual Lespedeza**

Seasonal Nurse/Cover Crop

** Use legume seed that is properly inoculated with specified Rhizobia. Legumes recommended unless periodic N fertilization is in-

tended. Weeping lovegrass may be added to any slope or low-maintenance mix during warmer seeding periods; add 10 to 20 lbs/acre

Bermudagrass can be added to substitute for Tall or Hard Fescue in the Low Maintenance mixes for the Southern Piedmont, particu-

larly on sandy soils or hot (S and W) facing slopes. May through October, use hulled seed. All other seeding periods, use un-hulled

Note: Seed mixes are suggested and subject to modification based on site-specific conditions by an agronomist or other qualified

Use seasonal nurse crop in accordance with seeding dates as stated below: February 16 through April annual rye

REV. **07.17.2025 PERMIT**

KEY PLAN:NTS

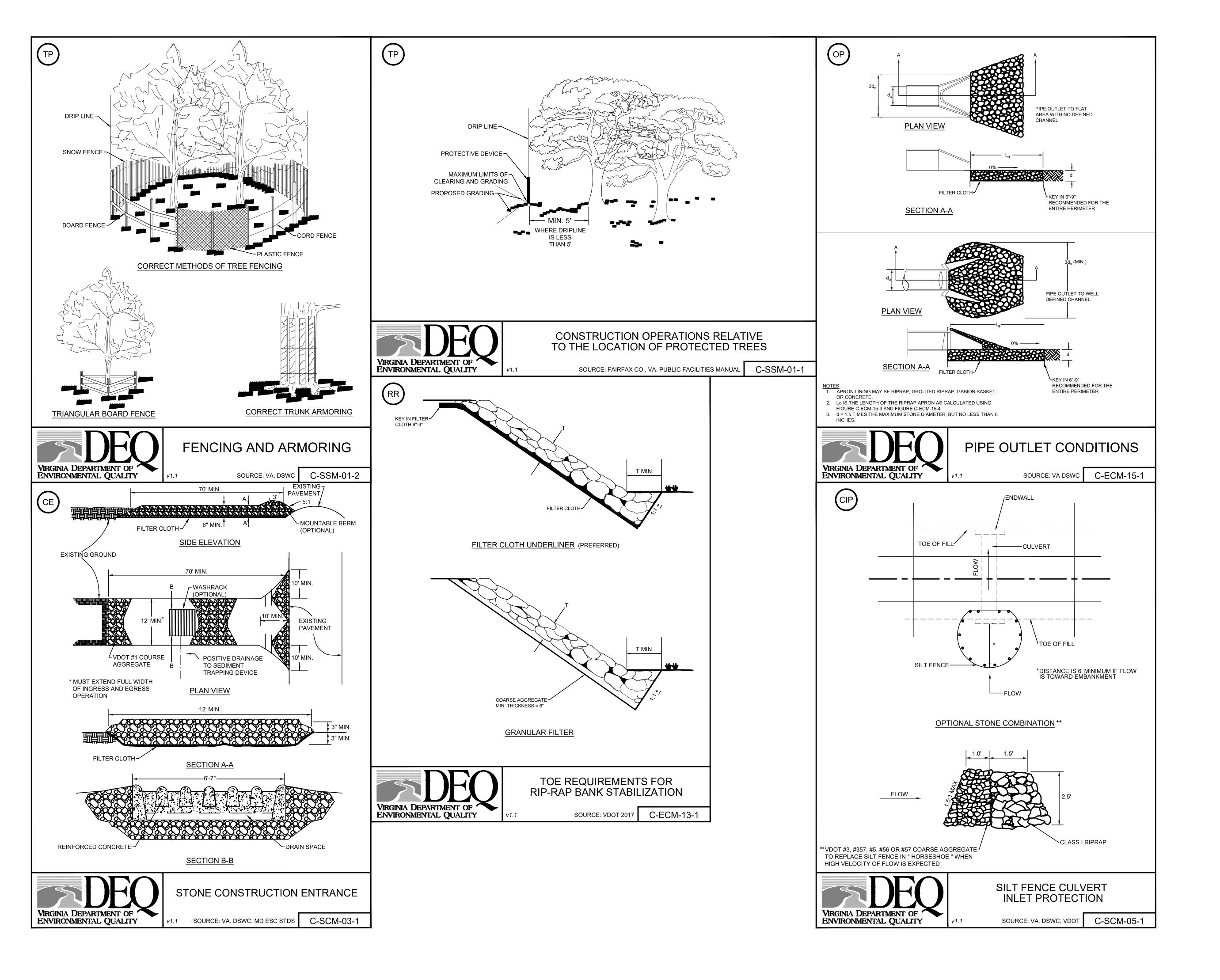
MA PROJECT NO. 2402 **AMELIA STREET SCHOOL**

ACCESSIBILITY TRAIL

1821 AMELIA STREET CITY OF RICHMOND, VA

> **EROSION AND SEDIMENT** CONTROL DETAILS

DATE 03.04.2025





OWNER/CLIENT ALLIANCE FOR THE CHESAPEAKE BAY

PROJECT MANAGER **NEAL FRIEDMAN**

CIVIL ENGINEERING NITSCH ENGINEERING

SURVEY **Nyfeler Survey**

CONSTRUCTION

REV. **07.17.2025 PERMIT**

KEY PLAN:NTS

MA PROJECT NO. 2402

AMELIA STREET SCHOOL

ACCESSIBILITY TRAIL

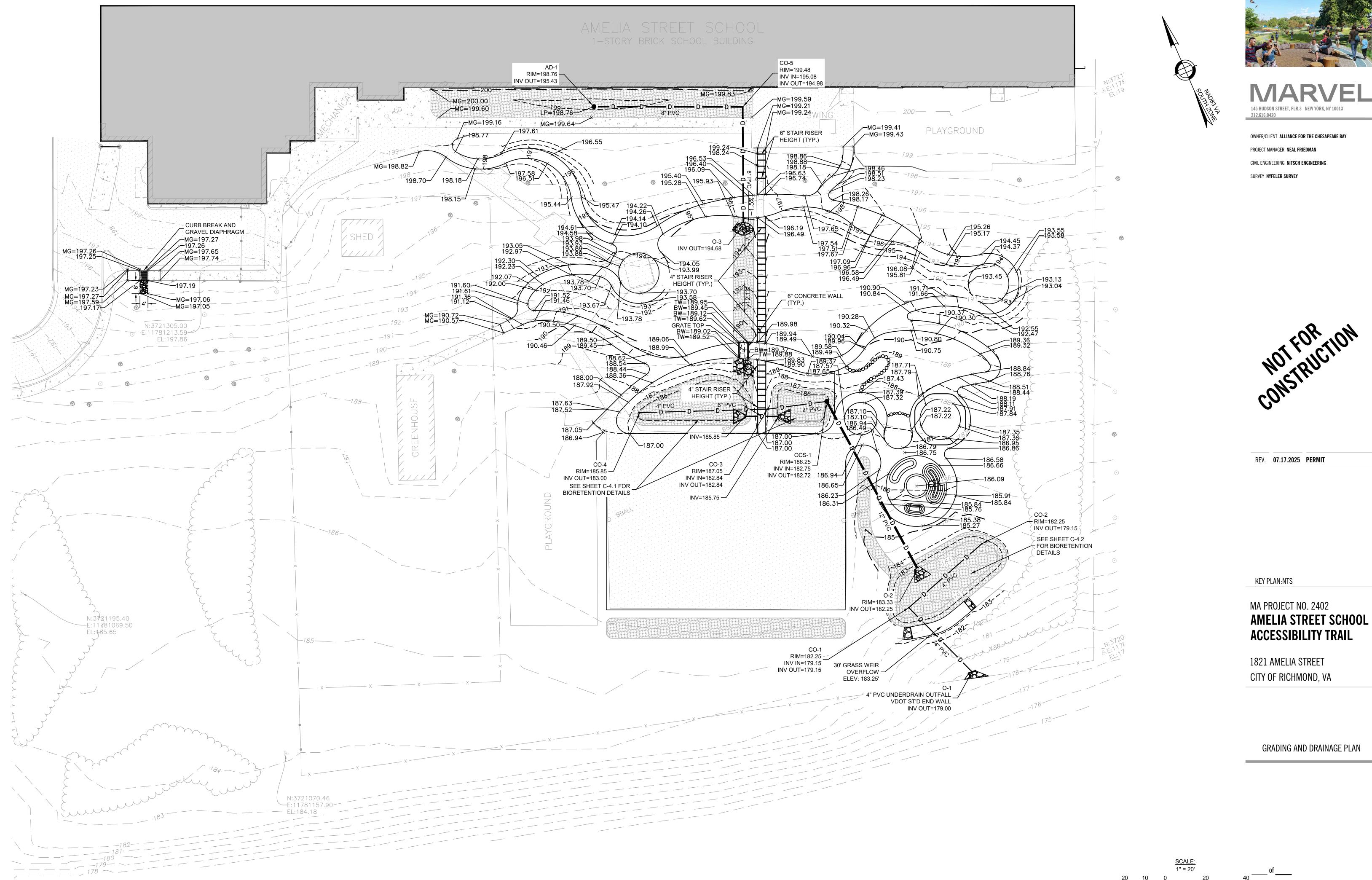
1821 AMELIA STREET CITY OF RICHMOND, VA

EROSION AND SEDIMENT CONTROL DETAILS

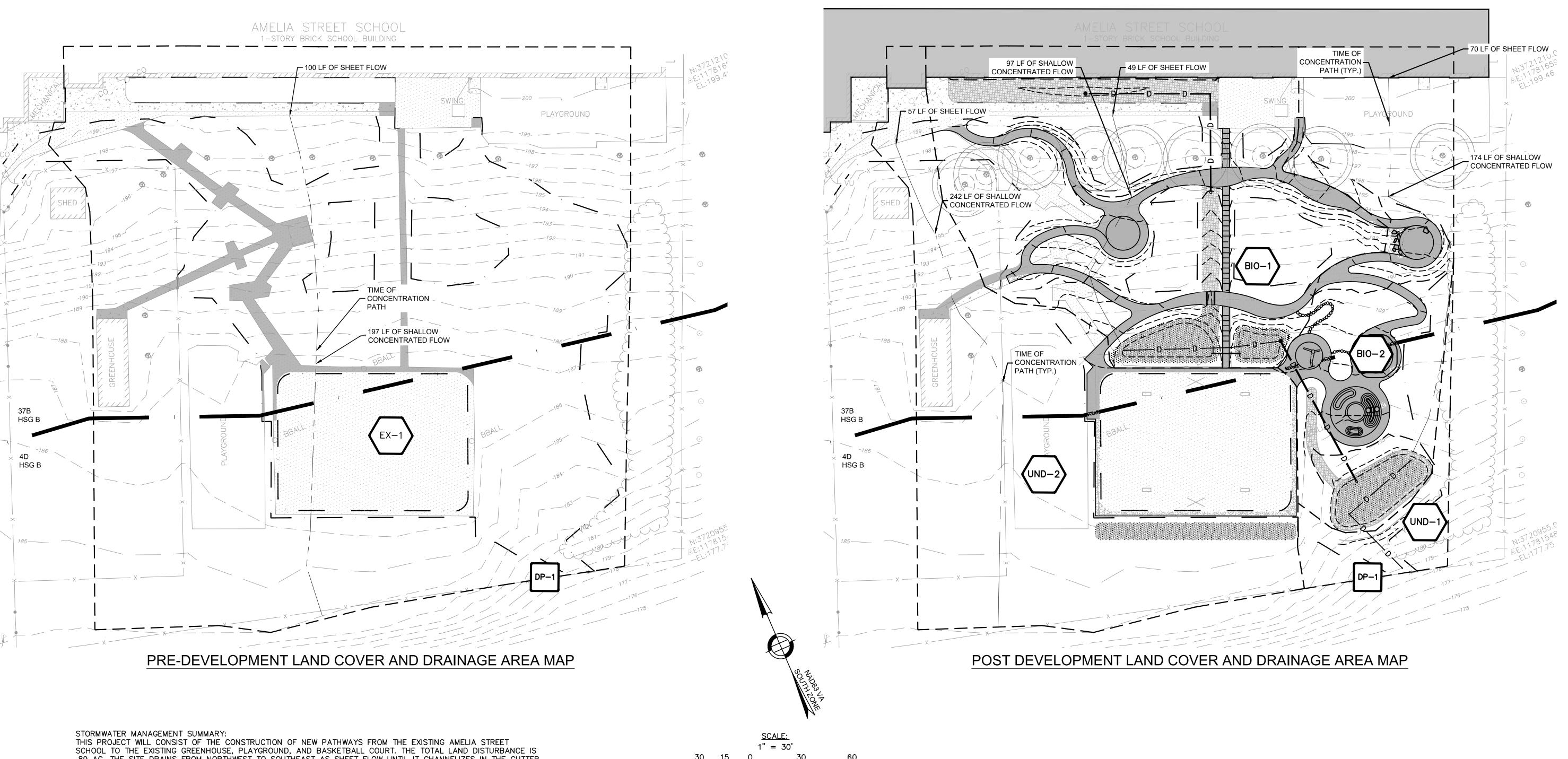
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DATE 03.04.2025

C-2

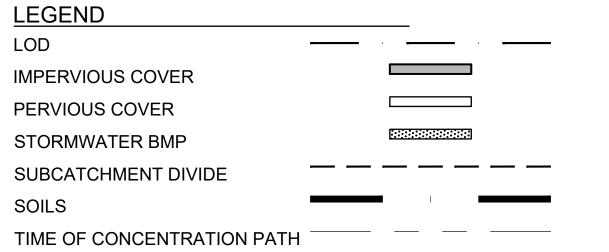






THIS PROJECT WILL CONSIST OF THE CONSTRUCTION OF NEW PATHWAYS FROM THE EXISTING AMELIA STREET SCHOOL TO THE EXISTING GREENHOUSE, PLAYGROUND, AND BASKETBALL COURT. THE TOTAL LAND DISTURBANCE IS .89 AC. THE SITE DRAINS FROM NORTHWEST TO SOUTHEAST AS SHEET FLOW UNTIL IT CHANNELIZES IN THE GUTTER ON COLORADO AVENUE AND ENTERS THE STORM DRAIN NETWORK. THE PROJECT IS LOCATED WITHIN THE CITY OF RICHMOND'S COMBINED SEWER OVERFLOW (CSO) SYSTEM. NO CHANGES TO THE SANITARY SEWER FLOW OF THE EXISTING SCHOOL ARE PROPOSED WITH THIS PROJECT.

THE EXISTING SITE IS MOSTLY MANAGED TURF. THE PROPOSED DEVELOPMENT WILL INCREASE THE IMPERVIOUS AREA BY 0.13 ACRES. BIORETENTION FACILITIES ARE PROVIDED ONSITE TO MANAGE THE INCREASE IN IMPERVIOUS AREA AND REDUCE 10—YR POST DEVELOPMENT PEAK FLOW RATES BELOW 10—YR PREDEVELOPMENT PEAK FLOW RATES.



DESIGN POINT

SUBCATCHMENT



145 HUDSON STREET, FLR.3 NEW YORK, NY 10013 212.616.0420

OWNER/CLIENT ALLIANCE FOR THE CHESAPEAKE BAY

PROJECT MANAGER **NEAL FRIEDMAN**

CIVIL ENGINEERING **NITSCH ENGINEERING**

SURVEY **NYFELER SURVEY**

NOTFORCION

REV. **07.17.2025 PERMIT**

KEY PLAN:NTS

MA PROJECT NO. 2402

AMELIA STREET SCHOOL

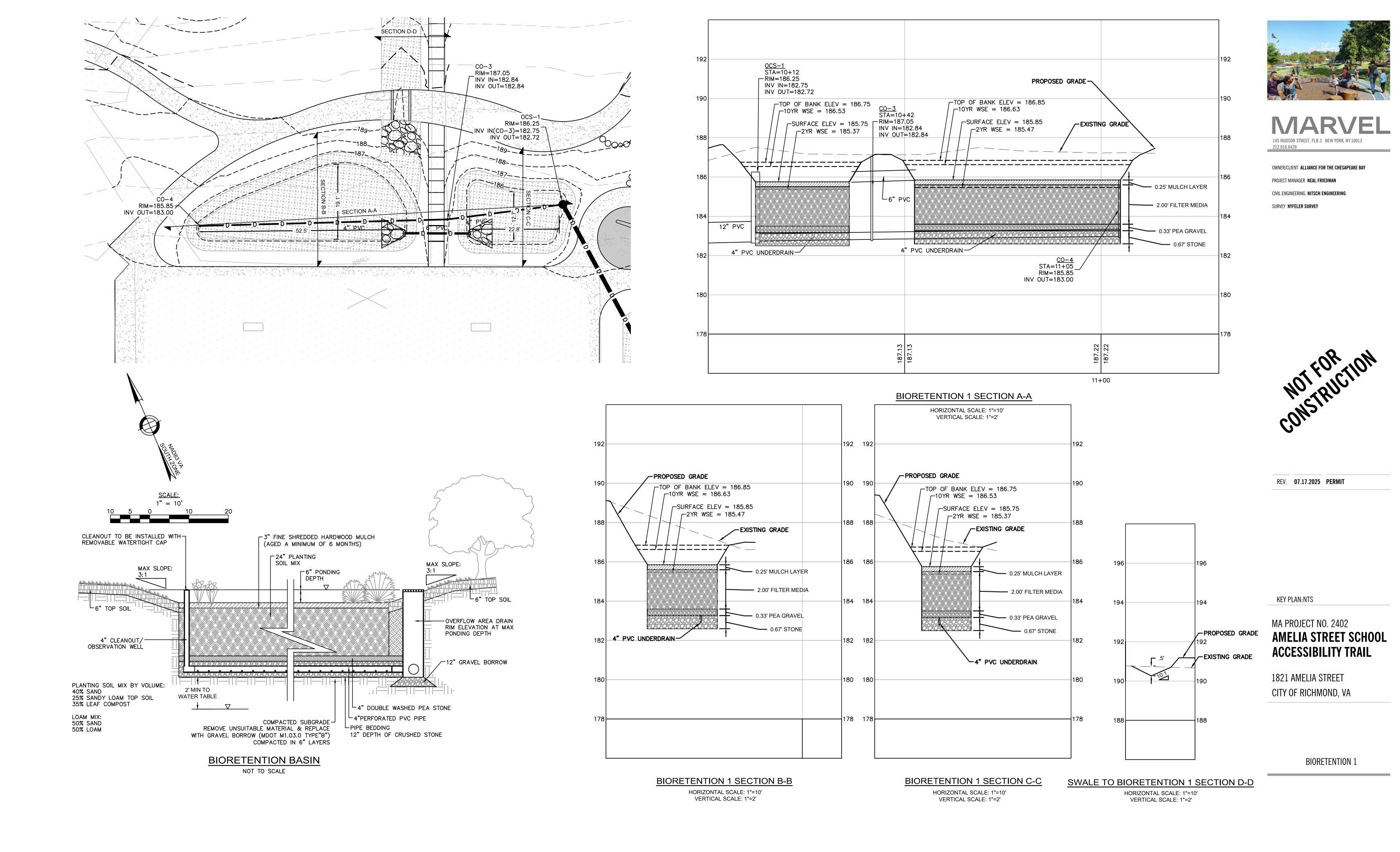
ACCESSIBILITY TRAIL

1821 AMELIA STREET CITY OF RICHMOND, VA

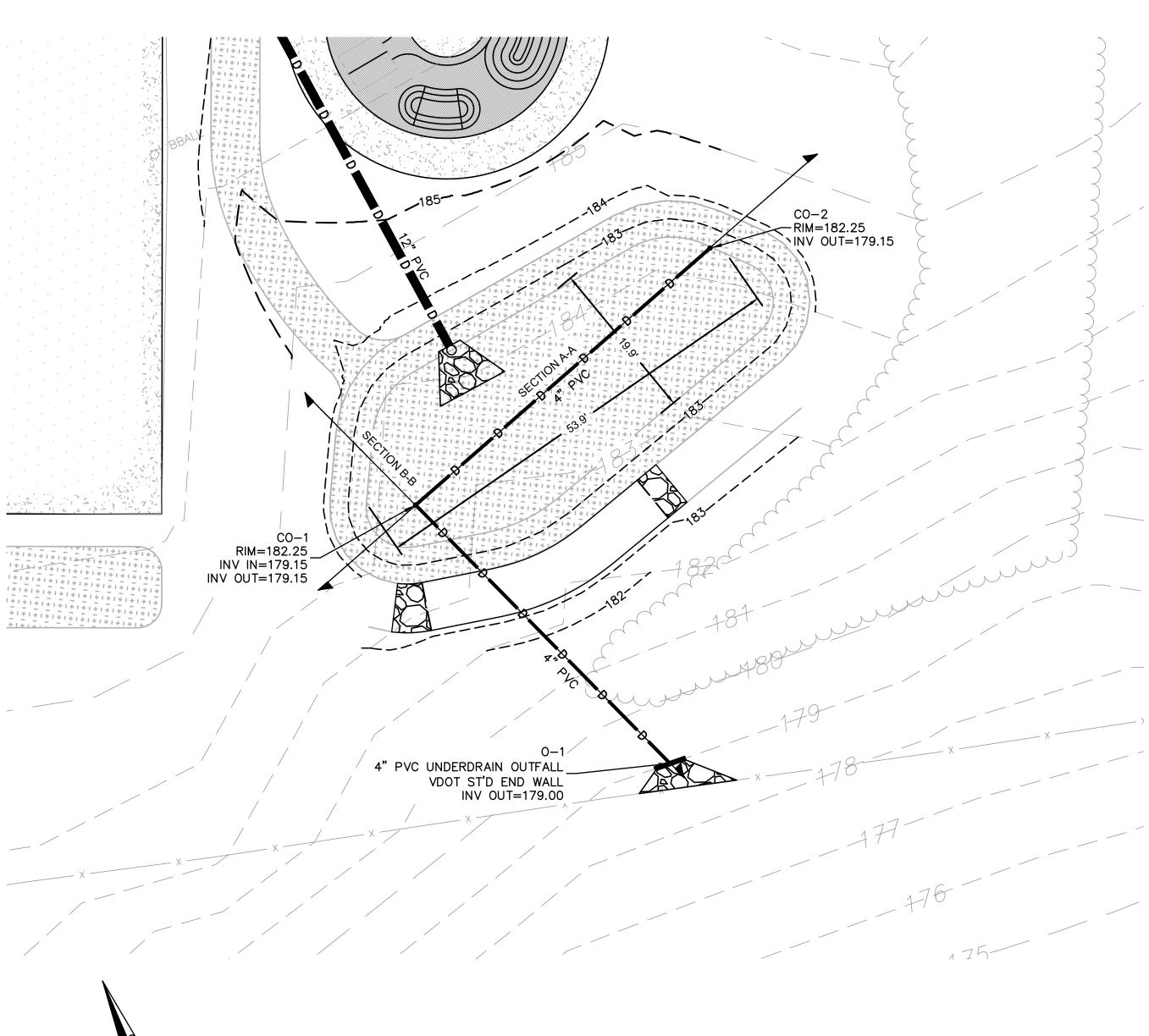
DRAINAGE AREA MAPS

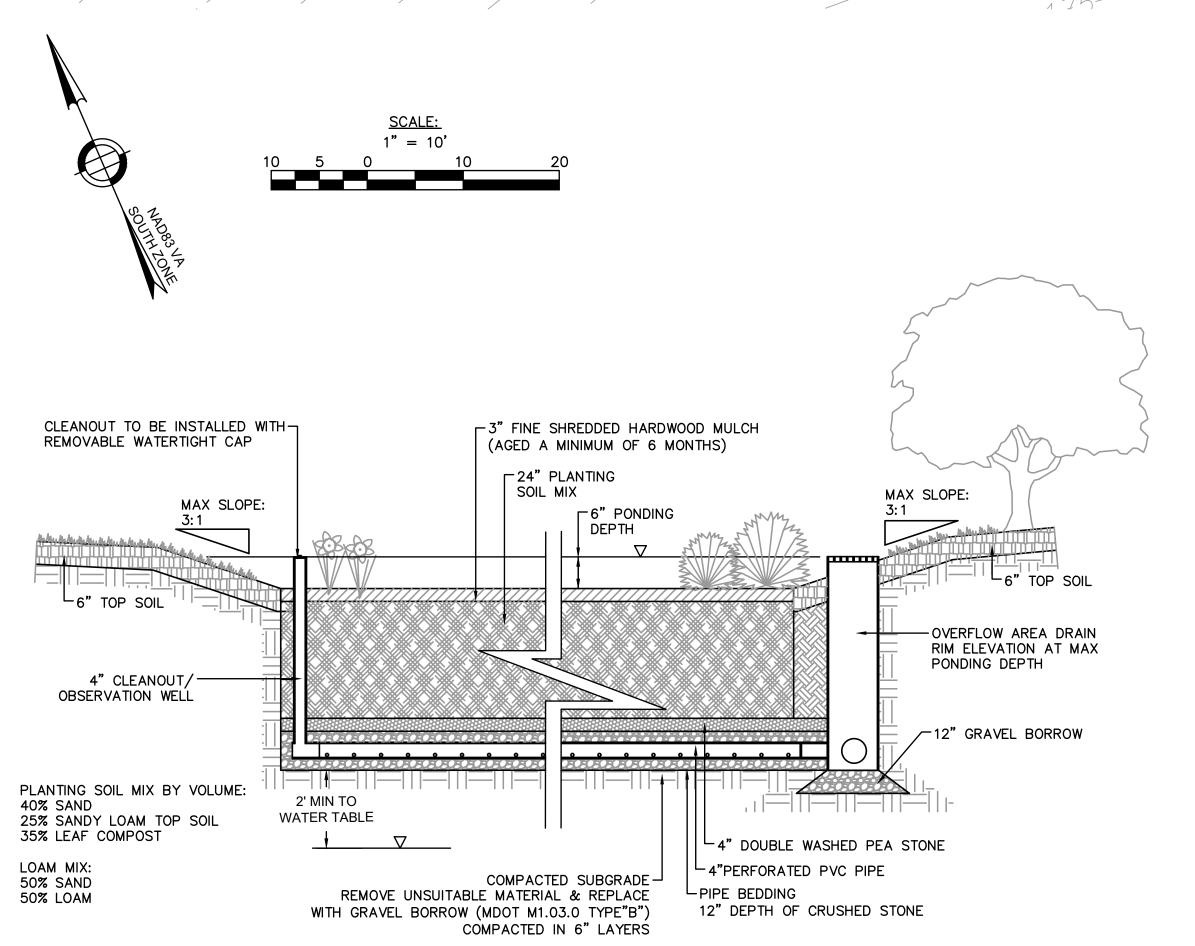
___ of ____

C-4

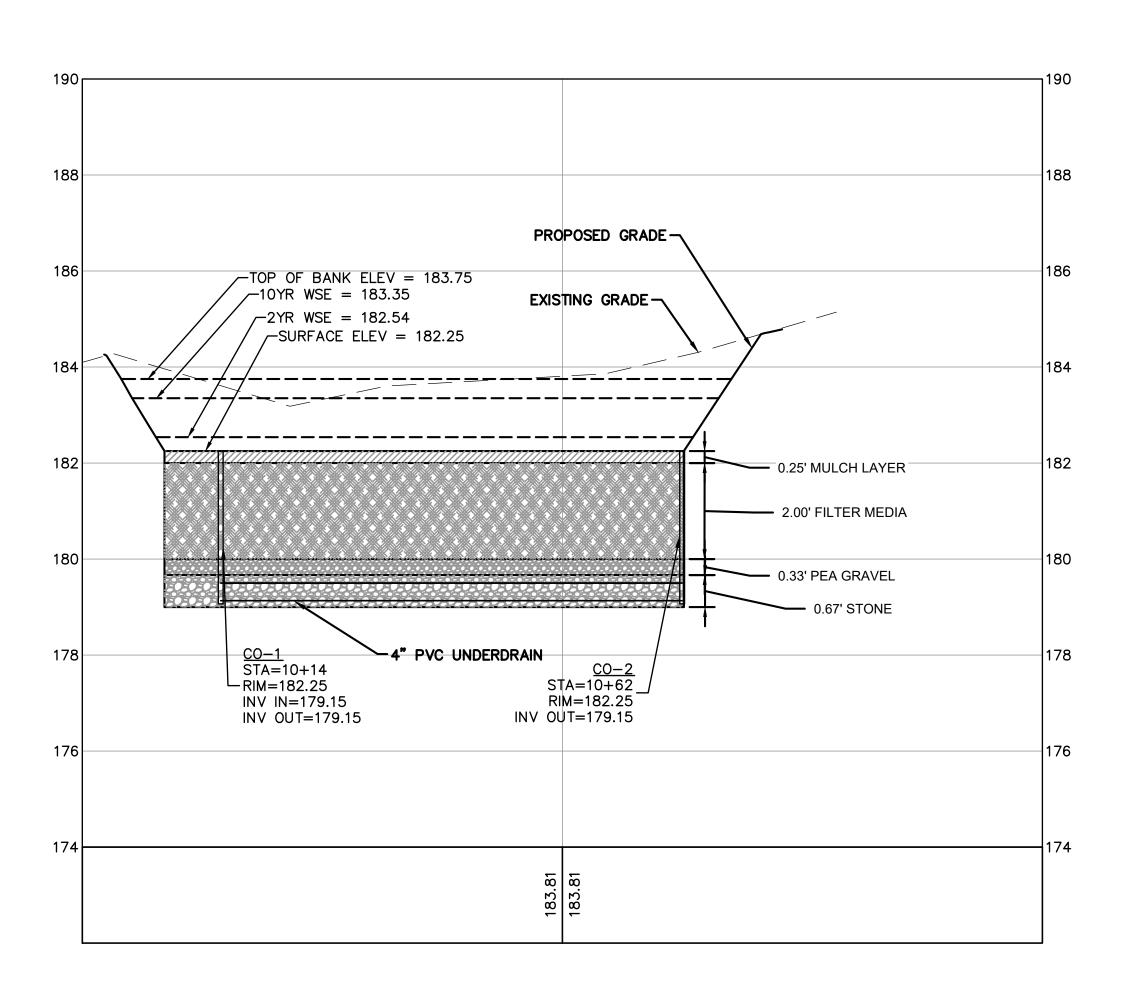


C-4.1



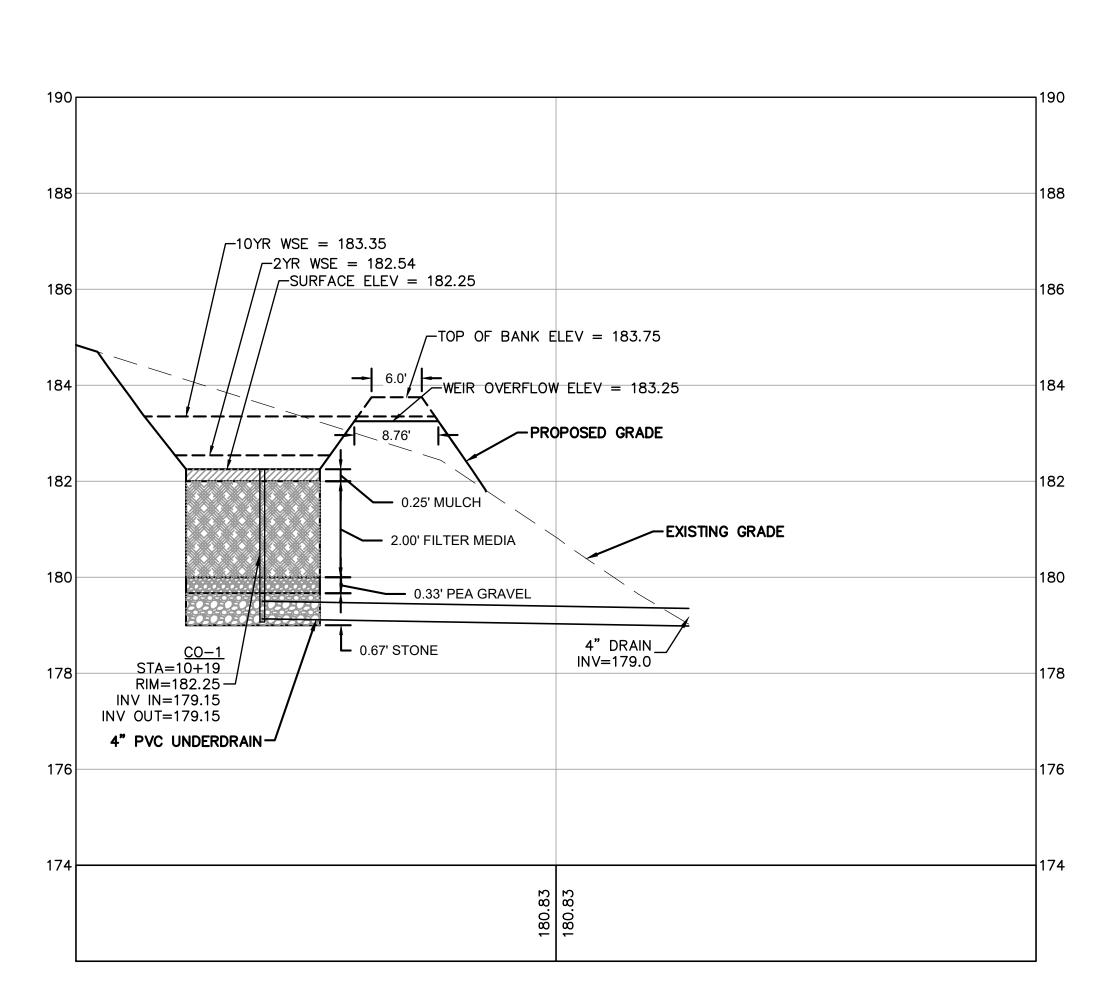


BIORETENTION BASIN NOT TO SCALE



BIORETENTION 2 SECTION A-A

HORIZONTAL SCALE: 1"=10' VERTICAL SCALE: 1"=2'



BIORETENTION 2 SECTION B-B

HORIZONTAL SCALE: 1"=10' VERTICAL SCALE: 1"=2'



MARVEL

OWNER/CLIENT ALLIANCE FOR THE CHESAPEAKE BAY

PROJECT MANAGER **NEAL FRIEDMAN**

CIVIL ENGINEERING NITSCH ENGINEERING

SURVEY **NYFELER SURVEY**

REV. **07.17.2025 PERMIT**

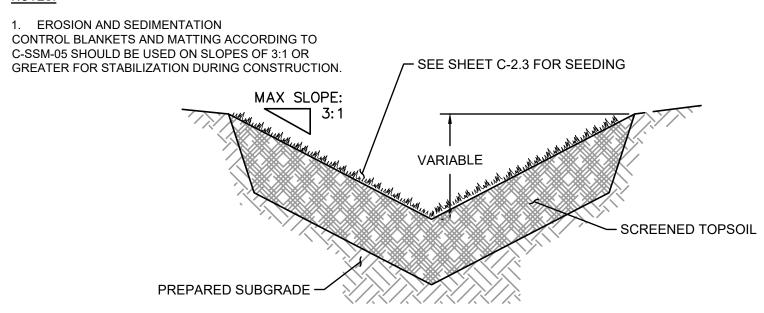
KEY PLAN:NTS

MA PROJECT NO. 2402 **AMELIA STREET SCHOOL ACCESSIBILITY TRAIL**

1821 AMELIA STREET CITY OF RICHMOND, VA

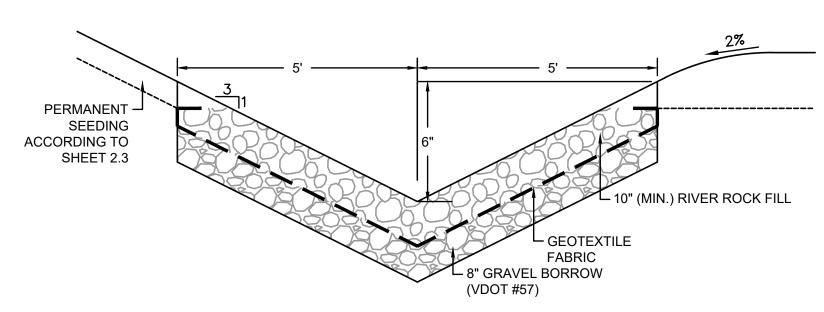
BIORETENTION 2

DATE 03.04.2025 © MARVEL ARCHITECTS, PLLC 2025 NOTES:



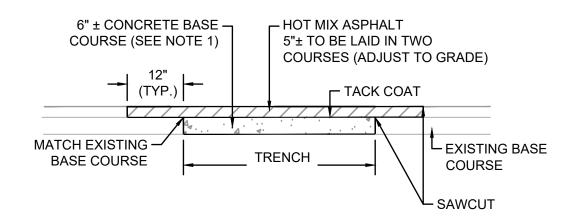
GRASS SWALE SECTION

NOT TO SCALE



MODIFIED ROCK FILLED DITCH DETAIL

NOT TO SCALE

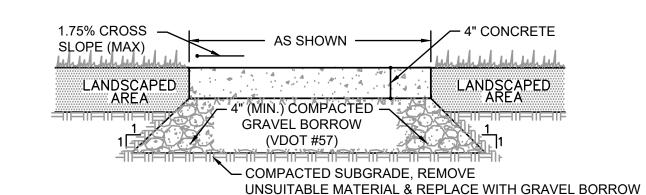


NOTES:

1. PROPOSED CONCRETE BASE COURSE SHALL BE LEVEL WITH
THE TOP OF THE EXISTING BASE COURSE

PAVEMENT RESTORATION OVER TRENCH DETAIL

NOT TO SCALE



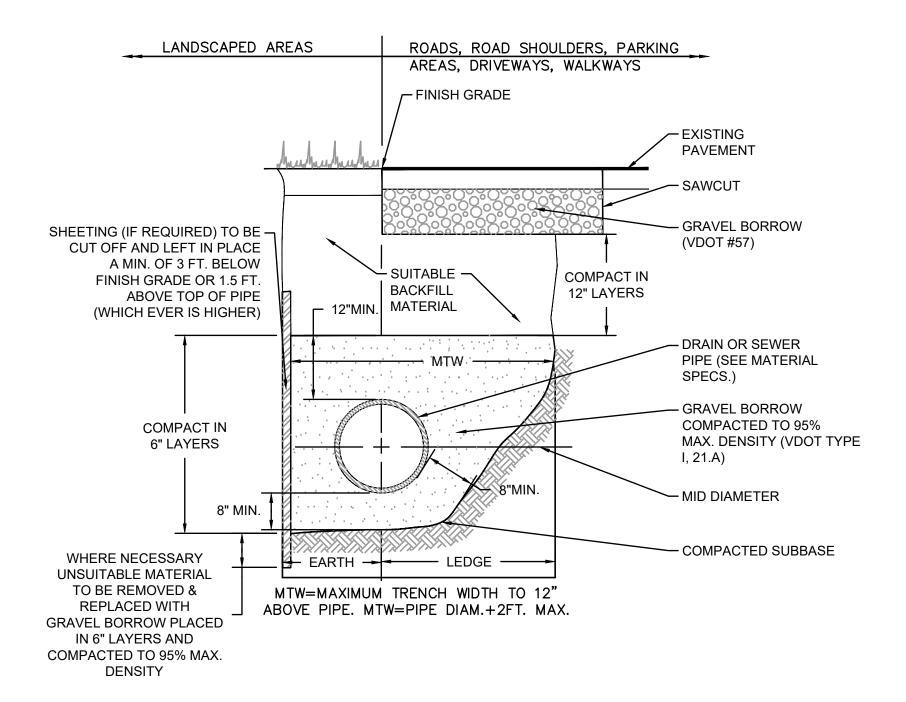
NOTES: (VDOT TYPE I, 21A) COMPACTED IN 6" LAYERS

1. CONTRACTOR SHALL INSTALL AN EXPANSION JOINT AT ALL BUILDINGS AND ALL SIDEWALK INTERSECTIONS OR AT START OF CURVED SECTIONS AND SHALL INSTALL LATERAL CONTROL JOINTS AS REQUIRED (8'-0" MAX. SPACING). SLOPE SIDEWALK AS SHOWN ON PLANS OR

ROADWAY SECTIONS (MINIMUM SLOPE = 1%)
2. CONCRETE SHALL BE 4000 PSI.

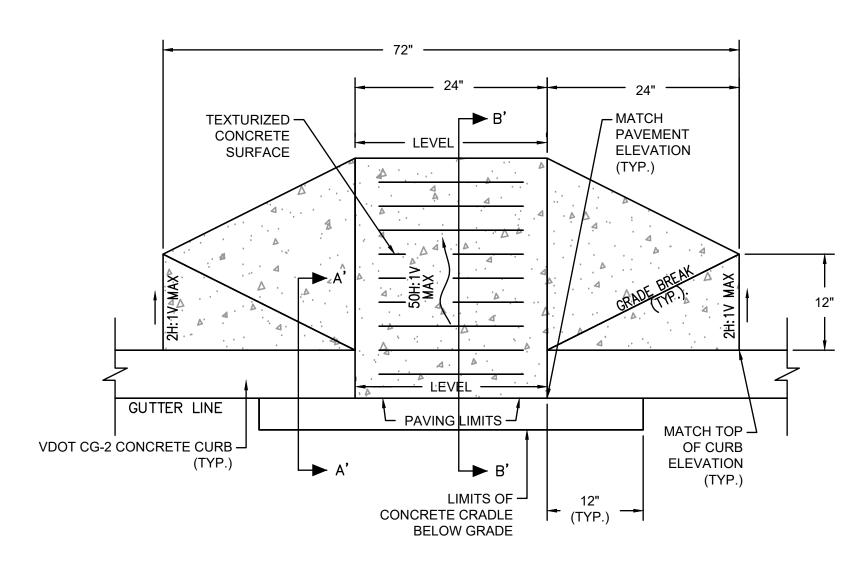
CONCRETE WALKWAY DETAIL

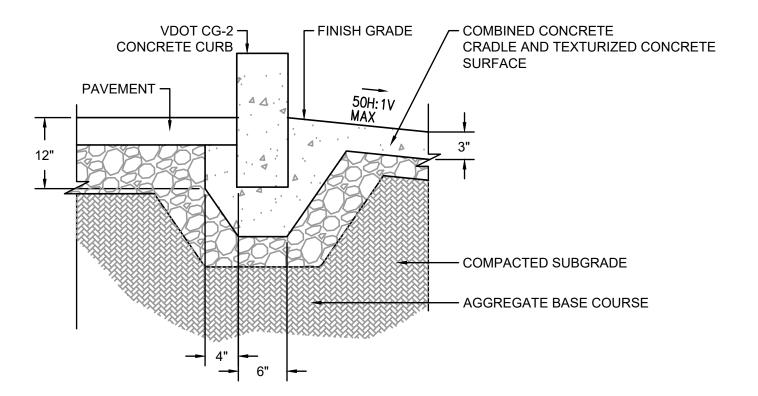
NOT TO SCALE



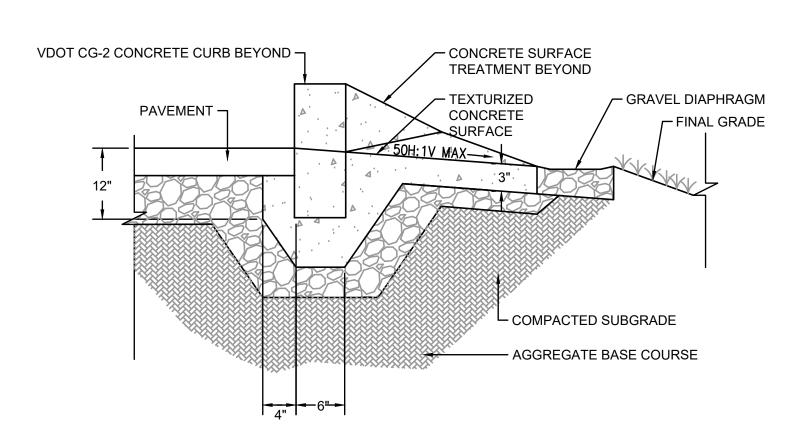
STANDARD TRENCH DETAIL FOR UTILITY PIPE

NOT TO SCALE





SECTION A'-A'



SECTION B'-B'

CURB BREAK AND ENERGY DISSIPATION

NOT TO SCALE





OWNER/CLIENT ALLIANCE FOR THE CHESAPEAKE BAY

PROJECT MANAGER **NEAL FRIEDMAN**

CIVIL ENGINEERING NITSCH ENGINEERING

SURVEY **NYFELER SURVEY**

CONSTRUCTION

REV. **07.17.2025 PERMIT**

KEY PLAN:NTS

MA PROJECT NO. 2402

AMELIA STREET SCHOOL

ACCESSIBILITY TRAIL

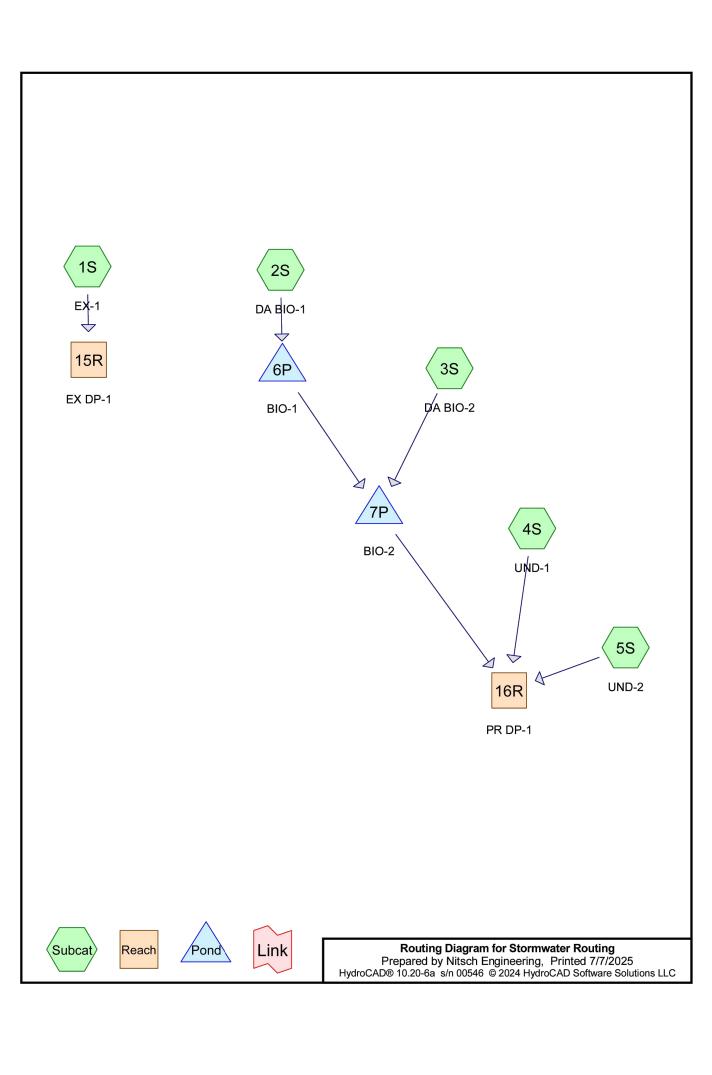
1821 AMELIA STREET CITY OF RICHMOND, VA

DETAILS

___ 0† _____

C-5.0

DATE 03.04.2025 © MARVEL ARCHITECTS, PLLC 2025



Type II 24-hr 10-yr Rainfall=5.09", P2=3.35" Stormwater Routing Printed 7/7/2025 Prepared by Nitsch Engineering HydroCAD® 10.20-6a s/n 00546 © 2024 HydroCAD Software Solutions LLC Page 12

Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment 1S: EX-1 Runoff Area=2.423 ac 30.87% Impervious Runoff Depth>2.07" Flow Length=297' Tc=9.6 min CN=72 Runoff=8.45 cfs 0.419 af

Runoff Area=0.871 ac 31.34% Impervious Runoff Depth>2.16" Subcatchment 2S: DA BIO-1 Tc=5.0 min CN=73 Runoff=3.69 cfs 0.157 af

Subcatchment 3S: DA BIO-2

Pond 6P: BIO-1

Tc=5.0 min CN=77 Runoff=2.45 cfs 0.105 af Runoff Area=0.093 ac 0.00% Impervious Runoff Depth>1.27" Subcatchment 4S: UND-1

Runoff Area=0.506 ac 42.69% Impervious Runoff Depth>2.49"

Flow Length=299' Tc=7.2 min CN=61 Runoff=0.22 cfs 0.010 af Runoff Area=0.950 ac 40.63% Impervious Runoff Depth>2.41" Subcatchment 5S: UND-2

Tc=5.0 min CN=76 Runoff=4.45 cfs 0.190 af Reach 15R: EX DP-1 Inflow=8.45 cfs 0.419 af Outflow=8.45 cfs 0.419 af

Inflow=5.18 cfs 0.409 af Reach 16R: PR DP-1 Outflow=5.18 cfs 0.409 af Peak Elev=186.53' Storage=2.023 cf Inflow=3.69 cfs 0.157 af

Outflow=2.17 cfs 0.153 af Peak Elev=183.35' Storage=2.499 cf Inflow=3.66 cfs 0.258 af Pond 7P: BIO-2 Primary=0.58 cfs 0.209 af Secondary=2.34 cfs 0.044 af Outflow=2.91 cfs 0.253 af

> Total Runoff Area = 4.843 ac Runoff Volume = 0.881 af Average Runoff Depth = 2.18" 66.49% Pervious = 3.220 ac 33.51% Impervious = 1.623 ac

Ρ	tormwatepared legistroce	by Nitso	h Engi		24 HydroCA	Type II :	10-yr Rainfall=5.09", P2=3.35" Printed 7/7/2025 Page 15
	Area (ad	c) CN	Desc	cription			
*	0.38	36 98					
*	0.56	61					
	0.95	0 76	Weig	ghted Aver	age		
	0.56	64	59.3	7% Pervio	us Area		
	0.38	86	40.6	3% Imperv	ious Area		
	Tc Lo	ength (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description	

Direct Entry,

Summary for Reach 15R: EX DP-1

2.423 ac, 30.87% Impervious, Inflow Depth > 2.07" for 10-yr event 8.45 cfs @ 12.01 hrs. Volume= 0.419 af 8.45 cfs @ 12.01 hrs, Volume= 0.419 af, Atten= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Summary for Reach 16R: PR DP-1

2.420 ac, 36.16% Impervious, Inflow Depth > 2.03" for 10-yr event 5.18 cfs @ 11.96 hrs, Volume= 0.409 af Outflow = 5.18 cfs @ 11.96 hrs, Volume= 0.409 af, Atten= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Summary for Pond 6P: BIO-1

0.871 ac, 31.34% Impervious, Inflow Depth > 2.16" for 10-yr event 3.69 cfs @ 11.96 hrs, Volume= 0.157 af 0.153 af, Atten= 41%, Lag= 5.5 min Outflow = 2.17 cfs @ 12.05 hrs, Volume= Primary = 2.17 cfs @ 12.05 hrs, Volume= 0.153 af Routed to Pond 7P: BIO-2

Routing by Dyn-Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 186.53' @ 12.05 hrs Surf.Area= 1,511 sf Storage= 2,023 cf

Plug-Flow detention time= 30.1 min calculated for 0.153 af (97% of inflow)

Center-of-Mass det. time= 20.4 min (811.7 - 791.3)

Volume	Invert	Avail.Storage	Storage Description
#1	182.49'	2,310 cf	Custom Stage Data (Prismatic) Listed below

Stormwater Routing	Type II 24-hr	10-yr Rainfall=5.09", P2=3.35"
Prepared by Nitsch Engineering		Printed 7/7/2025
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Elevation (feet)	Surf.Area (sq-ft)	Voids (%)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
182.49	1,015	0.0	0	0
182.50	1,015	40.0	4	4
183.50	1,015	40.0	406	410
183.51	1,015	25.0	3	413
185.75	1,015	25.0	568	981
185.76	1,015	100.0	10	991
186.75	1,649	100.0	1,319	2,310

Device	Routing	Invert	Outlet Devices
#1	Primary	182.67'	12.0" Round CMP_Round 12"
			L= 87.1' CPP, projecting, no headwall, Ke= 0.900
			Inlet / Outlet Invert= 182.67' / 182.20' S= 0.0054 '/' Cc= 0.900
			n= 0.010 PVC, smooth interior, Flow Area= 0.79 sf
#2	Device 1	182.77'	4.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#3	Device 1	186.27'	12.0" Horiz. Orifice/Grate C= 0.600
			Limited to weir flow at low heads

Primary OutFlow Max=2.13 cfs @ 12.05 hrs HW=186.53' TW=183.34' (Dynamic Tailwater) 1=CMP_Round 12" (Passes 2.13 cfs of 5.33 cfs potential flow) 2=Orifice/Grate (Orifice Controls 0.75 cfs @ 8.60 fps)

-3=Orifice/Grate (Weir Controls 1.38 cfs @ 1.67 fps)

Summary for Pond 7P: BIO-2

Inflow Area = 1.377 ac, 35.51% Impervious, Inflow Depth > 2.25" for 10-yr event Inflow = 3.66 cfs @ 12.00 hrs, Volume= 0.258 af Outflow = 2.91 cfs @ 12.08 hrs, Volume= 0.253 af, Atten= 20%, Lag= 4.7 min Primary = 0.58 cfs @ 12.08 hrs, Volume= 0.209 af Routed to Reach 16R: PR DP-1

Secondary = 2.34 cfs @ 12.08 hrs, Volume= 0.044 af

Routing by Dyn-Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 183.35' @ 12.08 hrs Surf.Area= 1,522 sf Storage= 2,499 cf

Plug-Flow detention time= 41.3 min calculated for 0.252 af (98% of inflow) Center-of-Mass det. time= 34.6 min (834.9 - 800.3)

Invert Avail.Storage Storage Description 178 99'

10101110		•	Gtorage Decemption				
#1	178.99'	3,041 cf	Custom Stage D	Data (Prismatic) Listed below			
Elevation (feet)	Surf.Area (sq-ft)	Voids (%)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)			
178.99	1,031	0.0	0	0			
179.00	1,031	40.0	4	4			
180.00	1,031	40.0	412	417			
180.01	1,031	25.0	3	419			
182.25	1,031	25.0	577	996			
182.26	1,031	100.0	10	1,007			
183.75	1,700	100.0	2,035	3,041			

Type II 24-hr 10-yr Rainfall=5.09", P2=3.35" Stormwater Routing Prepared by Nitsch Engineering Printed 7/7/2025 HydroCAD® 10.20-6a s/n 00546 © 2024 HydroCAD Software Solutions LLC Page 13

Summary for Subcatchment 1S: EX-1

Runoff = 8.45 cfs @ 12.01 hrs, Volume= 0.419 af, Depth> 2.07" Routed to Reach 15R: EX DP-1

___Area (ac) CN Description

1.675 61 >75% Grass cover, Good, HSG B

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type II 24-hr 10-yr Rainfall=5.09", P2=3.35"

	0	748 9	8 Pave	ed parking.	HSG B	
-						,
		_	,	ghted Aver	•	
	1.	675	69.1	3% Pervio	us Area	
	0.	748	30.8	7% Impery	rious Area	
	•			. ,		
	Тс	Length	Slope	Velocity	Capacity	Description
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	•
	8.4	100	0.0700	0.20		Sheet Flow,
						Grass: Dense n= 0.240 P2= 3.35"
	0.2	52	0.1200	5.58		Shallow Concentrated Flow,
	0.2	32	0.1200	3.30		•
						Unpaved Kv= 16.1 fps
	0.7	84	0.0100	2.03		Shallow Concentrated Flow,
						Paved Kv= 20.3 fps
	0.3	61	0.0400	3.22		Shallow Concentrated Flow,
	0.0	0.	0.0.00	0.22		Unpaved Kv= 16.1 fps
-						Olipaved Rv- 10.1 lps
	9.6	297	Total			

Summary for Subcatchment 2S: DA BIO-1

Runoff = 3.69 cfs @ 11.96 hrs, Volume= 0.157 af, Depth> 2.16" Routed to Pond 6P : BIO-1

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type II 24-hr, 10-yr Rainfall=5.09", P2=3.35"

	Type II 24-nr 10-yr Raintail=5.09°, P2=3.35°								
	Area (ac)	CN	Desc	cription					
	0.273	98	Pave	Paved parking, HSG B					
_	0.598	0.598 61 >75% Grass cover, Good, HSG B							
	0.871	73	Weig	ghted Aver	age				
	0.598 68.66% Pervious Area								
	0.273		31.3	4% Imperv	ious Area				
			<u> </u>						
		•	Slope	Velocity	Capacity	Description			
-	(min) (f	eet)	(ft/ft)	(ft/sec)	(cfs)				

Direct Entry,

Type II 24-hr 10-yr Rainfall=5.09", P2=3.35" Stormwater Routing Prepared by Nitsch Engineering Printed 7/7/2025

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Devic	e Routing	Invert	Outlet Devices
#1	1 Primary	179.25'	4.0" Round 4" Underdrain
			L= 50.0' CPP, projecting, no headwall, Ke= 0.900
			Inlet / Outlet Invert= 179.25' / 179.00' S= 0.0050 '/' Cc= 0.900
			n= 0.010 PVC, smooth interior, Flow Area= 0.09 sf
#2	2 Secondary	183.25'	30.0' long + 3.0 '/' SideZ x 6.0' breadth Broad-Crested Rectangular Wei
	•		Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00
			2.50 3.00 3.50 4.00 4.50 5.00 5.50
			Coef. (English) 2.37 2.51 2.70 2.68 2.68 2.67 2.65 2.65 2.65
			2.65 2.66 2.66 2.67 2.69 2.72 2.76 2.83

Primary OutFlow Max=0.58 cfs @ 12.08 hrs HW=183.35' TW=0.00' (Dynamic Tailwater)
—1=4" Underdrain (Barrel Controls 0.58 cfs @ 6.61 fps)

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Secondary OutFlow Max=2.16 cfs @ 12.08 hrs HW=183.35' (Free Discharge)

2=Broad-Crested Rectangular Weir (Weir Controls 2.16 cfs @ 0.74 fps)

Type II 24-hr 10-yr Rainfall=5.09", P2=3.35" **Stormwater Routing** Printed 7/7/2025 Prepared by Nitsch Engineering HydroCAD® 10.20-6a s/n 00546 © 2024 HydroCAD Software Solutions LLC Page 14

Summary for Subcatchment 3S: DA BIO-2

Runoff = 2.45 cfs @ 11.96 hrs, Volume= 0.105 af, Depth> 2.49" Routed to Pond 7P : BIO-2

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type II 24-hr 10-yr Rainfall=5.09", P2=3.35"

		Area	(ac)	CN	Desc	cription			
7	*	0.	216	98					
	*	0.	290	61					
		0.	506	77	Weig	hted Ave	rage		
		0.	290		57.3°	1% Pervio	us Area		
		0.	216		42.69	9% Imperv	vious Area		
		Тс	Lengt	:h	Slope	Velocity	Capacity	Descriptio	n
		(min)	(fee	t)	(ft/ft)	(ft/sec)	(cfs)		
		5.0						Direct En	try,
						0	f o	4	40 - UND 4
						Summ	nary for S	ubcatchn	nent 4S: UND-1
	Ь.	cc	_		0 00 -f-		0 h \ /- h		0.040 -

Runoff = 0.22 cfs @ 12.00 hrs, Volume= 0.010 af, Depth> 1.27" Routed to Reach 16R: PR DP-1

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type II 24-hr 10-yr Rainfall=5.09", P2=3.35"

_	Area	(ac) C	N Desc	cription		
*	0.	.093 6	31			
_	0.	.093	100.	00% Pervi	ous Area	
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
	5.7	57	0.0600	0.17		Sheet Flow, Grass: Dense n= 0.240 P2= 3.35"
	0.1	50	0.1300	5.80		Shallow Concentrated Flow, Unpaved Kv= 16.1 fps
	1.4	192	0.0200	2.28		Shallow Concentrated Flow, Unpaved Kv= 16.1 fps
_	7.2	299	Total			

Summary for Subcatchment 5S: UND-2

Runoff = 4.45 cfs @ 11.96 hrs, Volume= 0.190 af, Depth> 2.41" Routed to Reach 16R: PR DP-1

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type II 24-hr 10-yr Rainfall=5.09", P2=3.35"





OWNER/CLIENT ALLIANCE FOR THE CHESAPEAKE BAY

PROJECT MANAGER **NEAL FRIEDMAN**

CIVIL ENGINEERING **NITSCH ENGINEERING**

SURVEY **NYFELER SURVEY**

REV. **07.17.2025 PERMIT**

KEY PLAN:NTS

MA PROJECT NO. 2402

AMELIA STREET SCHOOL ACCESSIBILITY TRAIL

1821 AMELIA STREET CITY OF RICHMOND, VA

CALCULATIONS

DATE 03.04.2025

	CO-5 T	O O-3 - 8" PIPE
Project Description		
Friction Method	Manning	
Cohra For	Formula	
Solve For	Normal Depth	
Input Data		
Roughness Coefficient	0.010	
Channel Slope	0.500 %	
Diameter	8.0 in	
Discharge	0.70 cfs	
Results		
Normal Depth	4.6 in	
Flow Area	0.2 ft ²	
Wetted Perimeter	1.1 ft	
Hydraulic Radius	2.2 in	
Top Width	0.66 ft	
Critical Depth	4.7 in	
Percent Full	57.6 %	
Critical Slope	0.458 %	
Velocity	3.36 ft/s	
Velocity Head	0.18 ft	
Specific Energy	0.56 ft	
Froude Number	1.055	
Maximum Discharge	1.19 cfs	
Discharge Full	1.11 cfs	
Slope Full	0.199 %	
Flow Type	Supercritical	
GVF Input Data		
Downstream Depth	0.0 in	
Length	0.0 ft	
Number Of Steps	0	
GVF Output Data		
Upstream Depth	0.0 in	
Profile Description	N/A	
Profile Headloss	0.00 ft	
Average End Depth Over Rise	0.0 %	
Normal Depth Over Rise	57.6 %	
Downstream Velocity	Infinity ft/s	
Hartmann Malaski	T . C . '1 . C . / -	

Infinity ft/s

4.6 in 4.7 in

0.500 %

0.458 %

Bentley Systems, Inc. Haestad Methods Solution Center 27 Siemon Company Drive Suite 200 W Watertown, CT 06795 USA +1-203-755-1666

FlowMaster [10.03.00.03] Page 5 of 5

FlowMaster [10.03.00.03] Page 2 of 5

Upstream Velocity

Normal Depth

Critical Depth

Channel Slope

Critical Slope

swale calc.fm8 7/8/2025

swale calc.fm8 7/8/2025

Project Description		
Friction Method	Manning Formula	
Solve For	Normal Depth	
Input Data		
Roughness Coefficient	0.050	
Channel Slope	12.000 %	
Left Side Slope	10.000 %	
Right Side Slope	10.000 %	
Discharge	0.70 cfs	
Results		
Normal Depth	2.2 in	
Flow Area	0.3 ft ²	
Wetted Perimeter	3.7 ft	
Hydraulic Radius	1.1 in	
Top Width	3.66 ft	
Critical Depth	2.4 in	
Critical Slope	7.927 %	
Velocity	2.09 ft/s	
Velocity Head	0.07 ft	
Specific Energy	0.25 ft	
Froude Number	1.218	
Flow Type	Supercritical	
GVF Input Data		
Downstream Depth	0.0 in	
Length	0.0 ft	
Number Of Steps	0	
GVF Output Data		
Upstream Depth	0.0 in	
Profile Description	N/A	
Profile Headloss	0.00 ft	
Downstream Velocity	Infinity ft/s	
Upstream Velocity	Infinity ft/s	
Normal Depth	2.2 in	
Critical Depth	2.4 in	
Channel Slope	12.000 %	
Critical Slope	7.927 %	
wale calc.fm8	Bentley Systems, Inc. Haestad Methods Solution Center 27 Siemon Company Drive Suite 200 W	FlowN [10.03.0 Page

Manning Formula 0.010 0.500 % 12.0 in 2.17 cfs 7.1 in 0.5 ft² 1.8 ft 3.3 in 0.98 ft 7.6 in 59.5 % 0.421 % 4.46 ft/s 0.31 ft 0.90 ft
7.1 in 0.5 ft² 1.8 ft 3.3 in 0.98 ft 7.6 in 59.5 % 0.421 % 4.46 ft/s 0.31 ft
7.1 in 0.5 ft² 1.8 ft 3.3 in 0.98 ft 7.6 in 59.5 % 0.421 % 4.46 ft/s 0.31 ft
0.010 0.500 % 12.0 in 2.17 cfs 7.1 in 0.5 ft ² 1.8 ft 3.3 in 0.98 ft 7.6 in 59.5 % 0.421 % 4.46 ft/s 0.31 ft
0.500 % 12.0 in 2.17 cfs 7.1 in 0.5 ft² 1.8 ft 3.3 in 0.98 ft 7.6 in 59.5 % 0.421 % 4.46 ft/s 0.31 ft
0.500 % 12.0 in 2.17 cfs 7.1 in 0.5 ft² 1.8 ft 3.3 in 0.98 ft 7.6 in 59.5 % 0.421 % 4.46 ft/s 0.31 ft
12.0 in 2.17 cfs 7.1 in 0.5 ft² 1.8 ft 3.3 in 0.98 ft 7.6 in 59.5 % 0.421 % 4.46 ft/s 0.31 ft
2.17 cfs 7.1 in 0.5 ft² 1.8 ft 3.3 in 0.98 ft 7.6 in 59.5 % 0.421 % 4.46 ft/s 0.31 ft
7.1 in 0.5 ft² 1.8 ft 3.3 in 0.98 ft 7.6 in 59.5 % 0.421 % 4.46 ft/s 0.31 ft
0.5 ft ² 1.8 ft 3.3 in 0.98 ft 7.6 in 59.5 % 0.421 % 4.46 ft/s 0.31 ft
0.5 ft ² 1.8 ft 3.3 in 0.98 ft 7.6 in 59.5 % 0.421 % 4.46 ft/s 0.31 ft
1.8 ft 3.3 in 0.98 ft 7.6 in 59.5 % 0.421 % 4.46 ft/s 0.31 ft
3.3 in 0.98 ft 7.6 in 59.5 % 0.421 % 4.46 ft/s 0.31 ft
0.98 ft 7.6 in 59.5 % 0.421 % 4.46 ft/s 0.31 ft
7.6 in 59.5 % 0.421 % 4.46 ft/s 0.31 ft
59.5 % 0.421 % 4.46 ft/s 0.31 ft
0.421 % 4.46 ft/s 0.31 ft
0.421 % 4.46 ft/s 0.31 ft
4.46 ft/s 0.31 ft
0.31 ft
0.50 10
1.116
3.52 cfs
3.27 cfs
0.220 %
Supercritical
0.0 in
0.0 ft
0
0.0 in
N/A
0.00 ft
0.0 %
59.5 %
Infinity ft/s
Infinity ft/s
7.1 in
-
7.1 in

OCS-1 TO O-2 - 12" PIPE

		E TO BIO 2
Project Description		
Friction Method	Manning Formula	
Solve For	Normal Depth	
Input Data		
Roughness Coefficient	0.050	
Channel Slope	12.000 %	
Left Side Slope	10.000 %	
Right Side Slope	10.000 %	
Discharge	0.53 cfs	
Results		
Normal Depth	2.0 in	
Flow Area	0.3 ft ²	
Wetted Perimeter	3.3 ft	
Hydraulic Radius	1.0 in	
Top Width	3.30 ft	
Critical Depth	2.1 in	
Critical Slope	8.224 %	
Velocity	1.94 ft/s	
Velocity Head	0.06 ft	
Specific Energy	0.22 ft	
Froude Number	1.193	
Flow Type	Supercritical	
GVF Input Data		
Downstream Depth	0.0 in	
Length	0.0 ft	
Number Of Steps	0	
GVF Output Data		
Upstream Depth	0.0 in	
Profile Description	N/A	
Profile Headloss	0.00 ft	
Downstream Velocity	Infinity ft/s	
Upstream Velocity	Infinity ft/s	
Normal Depth	2.0 in	
Critical Depth	2.1 in	
Channel Slope	12.000 %	
Critical Slope	8.224 %	

Bentley Systems, Inc. Haestad Methods Solution Center 27 Siemon Company Drive Suite 200 W Watertown, CT 06795 USA +1-203-755-1666

Normal Depth	3 F i	
	3.5 in	
Flow Area	0.1 ft ²	
Wetted Perimeter	0.8 ft	
Hydraulic Radius	1.2 in	
Top Width	0.22 ft	
Critical Depth	4.0 in	
Percent Full	87.2 %	
Critical Slope	5.074 %	
Velocity	7.18 ft/s	
Velocity Head	0.80 ft	
Specific Energy	1.09 ft	
Froude Number	2.105	
Maximum Discharge	0.60 cfs	
Discharge Full	0.55 cfs	
Slope Full	5.496 %	
Flow Type	Supercritical	
0)/51		
GVF Input Data		
Downstream Depth	0.0 in	
Downstream Depth	0.0 111	
Length	0.0 ft	
-		
Length	0.0 ft	
Length	0.0 ft	
Length Number Of Steps	0.0 ft	
Length Number Of Steps GVF Output Data	0.0 ft 0	
Length Number Of Steps GVF Output Data Upstream Depth	0.0 ft 0	
Length Number Of Steps GVF Output Data Upstream Depth Profile Description	0.0 ft 0 0.0 in N/A	
Length Number Of Steps GVF Output Data Upstream Depth Profile Description Profile Headloss	0.0 ft 0 0.0 in N/A 0.00 ft	
Length Number Of Steps GVF Output Data Upstream Depth Profile Description Profile Headloss Average End Depth Over Rise	0.0 ft 0 0.0 in N/A 0.00 ft 0.0 %	
Length Number Of Steps GVF Output Data Upstream Depth Profile Description Profile Headloss Average End Depth Over Rise Normal Depth Over Rise	0.0 ft 0 0.0 in N/A 0.00 ft 0.0 % 87.2 %	
Length Number Of Steps GVF Output Data Upstream Depth Profile Description Profile Headloss Average End Depth Over Rise Normal Depth Over Rise Downstream Velocity	0.0 ft 0 0.0 in N/A 0.00 ft 0.0 % 87.2 % Infinity ft/s	
Length Number Of Steps GVF Output Data Upstream Depth Profile Description Profile Headloss Average End Depth Over Rise Normal Depth Over Rise Downstream Velocity Upstream Velocity Normal Depth	0.0 ft 0 0.0 in N/A 0.00 ft 0.0 % 87.2 % Infinity ft/s Infinity ft/s	
Length Number Of Steps GVF Output Data Upstream Depth Profile Description Profile Headloss Average End Depth Over Rise Normal Depth Over Rise Downstream Velocity Upstream Velocity Normal Depth Critical Depth	0.0 ft 0 0.0 in N/A 0.00 ft 0.0 % 87.2 % Infinity ft/s Infinity ft/s 3.5 in	
Length Number Of Steps GVF Output Data Upstream Depth Profile Description Profile Headloss Average End Depth Over Rise Normal Depth Over Rise	0.0 ft 0 0.0 in N/A 0.00 ft 0.0 % 87.2 %	
Length Number Of Steps GVF Output Data Upstream Depth Profile Description Profile Headloss Average End Depth Over Rise Normal Depth Over Rise Downstream Velocity Upstream Velocity	0.0 ft 0 0.0 in N/A 0.00 ft 0.0 % 87.2 % Infinity ft/s Infinity ft/s	
Length Number Of Steps GVF Output Data Upstream Depth Profile Description Profile Headloss Average End Depth Over Rise Normal Depth Over Rise Downstream Velocity Upstream Velocity Normal Depth	0.0 ft 0 0.0 in N/A 0.00 ft 0.0 % 87.2 % Infinity ft/s Infinity ft/s 3.5 in	
Length Number Of Steps GVF Output Data Upstream Depth Profile Description Profile Headloss Average End Depth Over Rise Normal Depth Over Rise Downstream Velocity Upstream Velocity Normal Depth Critical Depth	0.0 ft 0 0.0 in N/A 0.00 ft 0.0 % 87.2 % Infinity ft/s Infinity ft/s 3.5 in 4.0 in	
Length Number Of Steps GVF Output Data Upstream Depth Profile Description Profile Headloss Average End Depth Over Rise Normal Depth Over Rise Downstream Velocity Upstream Velocity Normal Depth	0.0 ft 0 0.0 in N/A 0.00 ft 0.0 % 87.2 % Infinity ft/s Infinity ft/s 3.5 in 4.0 in	

CO-1 TO O-1 - 4" PIPE

Manning Formula

0.010 5.000 % 4.0 in 0.58 cfs

Normal Depth

Project Description

Roughness Coefficient Channel Slope Diameter Discharge Results

Friction Method

Solve For

Input Data

Table C-ECM-15-3 Permissible	Velocities for Earth Linings
Soil Types	Permissible Velocities (ft/s)
Fine Sand (non-colloidal)	2.5
Sandy Loam (non-colloidal)	2.5
Silt Loam (non-colloidal)	3.0
Ordinary Firm Loam	3.5
Fine Gravel	5.0
Stiff Clay (very colloidal)	5.0
Graded, Loam to Cobbles (non-colloidal)	5.0
Graded, Silt to Cobbles (colloidal)	5.5
Alluvial Silts (non-colloidal)	3.5
Alluvial Silts (colloidal)	5.0
Coarse Gravel (non-colloidal)	6.0
Cobbles and Shingles	5.5
Shales and Hard Plans	6.0

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Source: Schwab, G., et al., 1966	3						
Table C-ECM-09-2 Maximum Velocities for Grass-Lined Channels							
Cover Type	Slope (%)	Erosion Resistant Soils (feet/second)	Easily Eroded Soils (feet/second)				
Damendamena	0 – 5	6	4.5				
Bermudagrass	5 – 10	5	3.8				
Kentucky bluegrass	0 – 5	5	3.8				
Reed Canarygrass	5 – 10	4	3				
Tall fescue	> 10	3	2.3				
Cross legume misture	0 – 5	4	3				
Grass-legume mixture	5 – 10	3	2.3				
Red fescue	0 – 5	2.5	1.9				
Sources: Virginia Department of Environmental Quality 1992; Ree 1949; Temple et al. 1987; NOVA 2007							





OWNER/CLIENT ALLIANCE FOR THE CHESAPEAKE BAY

PROJECT MANAGER **NEAL FRIEDMAN**

CIVIL ENGINEERING NITSCH ENGINEERING

SURVEY **NYFELER SURVEY**

REV. **07.17.2025 PERMIT**

KEY PLAN:NTS

MA PROJECT NO. 2402 **AMELIA STREET SCHOOL ACCESSIBILITY TRAIL**

1821 AMELIA STREET CITY OF RICHMOND, VA

CALCULATIONS

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