Stony Point Density Summary for Sections D, E and F Revised September 27, 2016 Based on "Gross Leasable Area" Map Section Building Highwoods Stony Point One, 9030 Stony Point Parkway Highwoods Stony Point Two, 9020 Stony Point Parkway 114,770 SF 28,185 SF Subtotal of Existing Density Maximum Allowable Density under CUP for Section D 261,000 SF **Section E** 56,800 SF 29,200 SF Future 73,746 SF Ambulatory Surgery (Eye & Ear Center) Existing Ambulatory Surgery – Reserve 13.254 SF Future Highwoods Stony Point Three, 8730 Stony Point Parkway Existing Highwoods Stony Point Four, 8720 Stony Point Parkway 94,730 SF Subtotal of Existing and Reserve Density Rev. 7/12/99 Maximum Allowable Density under CUP for Section E Highwoods Building A, 9101 Stony Point Drive * Proposed Virginia Urology, 9105 Stony Point Drive ** 55,000 SF Existing Highwoods Building C, 9109 Stony Point Drive 59,841 SF Approved 37,690 SF 234,000 SF Subtotal of Existing and Approved Density Maximum Allowable Density under CUP for Section F Maximum Density under CUP for Sections D, E and F 835,000 SF * Parcel F Master Plan approval by City of Richmond in 2002 included three buildings totaling 199,000 SF. The "Gross Leasable Area" of proposed Building A is 2,690 SF under the original Master Plan approval of 199,000 SF ** The existing Urology Building is being carried at the original approved SF. This building may be adjusted in the future to more accurately comply with definition of "Gross Leasable Area".

SITE STATISTICS

ADDRESS: 9101 STONY POINT DRIVE, RICHMOND, VA 23235

Total maximum Density allowed for all three Map Sections under the amended CUP is 835,000 SF

Total Existing, Reserve, and Approved Density to Date is 835,000 SF (239,904 SF + 361,096 SF + 234,000 SF).

MAP REFERENCE NUMBER: C0010757065

PLANNED LAND USE: ECONOMIC OPPORTUNITY AREA

COMMUNITY UNIT PLAN (ORDINANCE NO. 2015-23-40) ZONING CODE: R-2 (CUP)

ENTERPRISE ZONE: EAST

TRAFFIC ZONE: 1142

PROPOSED USE: OFFICE

WATER: PUBLIC, CITY OF RICHMOND

SANITARY SEWER: PUBLIC, CITY OF RICHMOND

DRAINAGE: PRIVATE SITE

ACREAGE: 19.8 AC. (ENTIRE SITE)

DISTURBED ACREAGE: 5.35 AC.

PROJECT LOCATION: SE QUADRANT, STONY POINT PKWY & CHIPPENHAM PKWY

BUILDING FLOOR AREA: REFER TO EXHIBIT D ABOVE

FLOOR AREA RATIO: (55,000 SF + 60,693 SF + 90,000 SF) / 43560 SF) = 4.77 AC

4.70 AC / 19.8 AC = 0.238 0.238 < 0.27

MAP F REQUIRED PARKING SUMMARY (4 SPACES PER 1,000 SF):

PROPOSED BUILDING A: 81,469 SF / 250 = 326 SPACES

(INCLUDES 9 HANDICAPPED ACCESSIBLE SPACES)

EXISTING BUILDING B: 55,000 SF / 250 = 220 SPACES (INCLUDES 28 HANDICAPPED ACCESSIBLE SPACES)

EXISTING BUILDING C: 59,841 / 250 = 240 SPACES (INCLUDES 7 HANDICAPPED ACCESSIBLE SPACES)

TOTAL PARKING REQUIRED = 786 SPACES

TOTAL PARKING PROVIDED = 1,058 SPACES PHASE I: 275 EXISTING

PHASE II: 306 EXISTING

MINIMUM GROSS OPEN SPACE (% OF SITE): 85% REQUIRED (BASED ON BUILDINGS A, B, & C)

92% PROVIDED (18.2 AC)

BUILDING A + BUILDING B + BUILDING C FOOTPRINT = 1.6 AC (19.8 AC - 1.6 AC = 18.2 AC)

46% PROVIDED (9.1 AC) (BASED ON BUILDINGS A, B, & C) GREEN SPACE + UNDISTURBED LAND = 9.1 AC

MINIMUM LIVABILITY SPACE (% OF SITE):

MINIMUM UNDISTURBED LAND (% OF SITE) WITHIN MAP SECTIONS D, E, AND F: (BASED ON BUILDINGS A. B. & C)

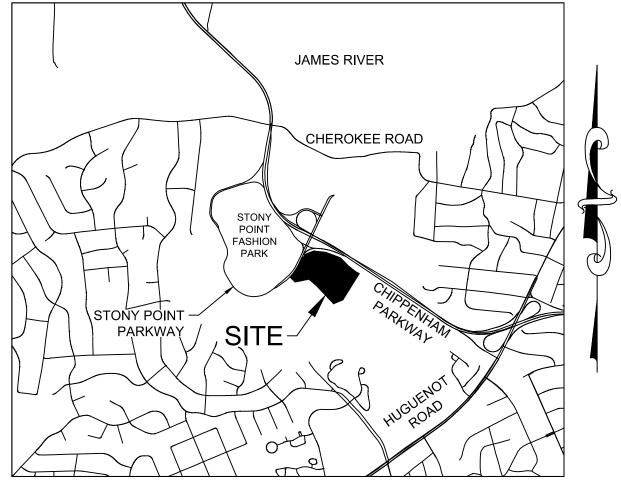
(5.2 AC + 3.9 AC = 9.1 AC)20% REQUIRED 20% PROVIDED (3.9 AC)

45% REQUIRED

STONY POINT MAP 'F' Rev. 7/12/99 PHASE III - BUILDING A



CITY OF RICHMOND, VIRGINIA



VICINITY MAP

SCALE: 1'' = 2,000'

SEPTEMBER 27, 2016

OWNER / DEVELOPER:

HIGHWOODS REALTY LIMITED PARTNERSHIP 4501

HIGHWOODS PARKWAY

SUITE 400

ARCHITECT:

BASKERVILL

SUITE 200

101 S. 15TH ST.

RICHMOND, VA 23219

PHONE: (804) 343-1010

FAX: (804) 343-0909

GLEN ALLEN, VIRGINIA 23060

CONTACT: JAY WOODBURN, AIA

CONTACT: HANK ROBERTSON

PHONE: (804) 747-7800 FAX: (804) 346-4265

EMAIL: HANK.ROBERTSON@HIGHWOODS.COM

CIVIL ENGINEER:

TIMMONS GROUP

1001 BOULDERS PARKWAY

SUITE 300

RICHMOND, VIRGINIA 23225

CONTACT: RYAN RITTERSKAMP, P.E. PHONE: (804) 200-6416

FAX: (804) 560-1016

EMAIL: RYAN.RITTERSKAMP@TIMMONS.COM

A2.1 DK

Sheet List Table Sheet **Sheet Title** Number COVER SHEET C0.0 EXISTING CONDITIONS AND DEMOLITION PLAN OVERALL PLAN C2.0 EROSION & SEDIMENT CONTROL PLAN C2.1 EROSION AND SEDIMENT CONTROL NOTES AND DETAILS C2.2 EROSION AND SEDIMENT CONTROL NOTES AND DETAILS C2.3 EROSION AND SEDIMENT CONTROL NOTES AND DETAILS C2.4 EROSION AND SEDIMENT CONTROL NOTES AND DETAILS C3.0 LAYOUT PLAN SITE NOTES AND DETAILS SITE NOTES AND DETAILS UTILITY PLAN **UTILITY PROFILES** UTILITY NOTES AND DETAILS UTILITY NOTES AND DETAILS C5.0 GRADING & DRAINAGE PLAN C5.1 STORM SEWER PROFILES & CALCULATIONS C5.2 STORM CALCULATIONS & DRAINAGE AREA MAP L1.0 LANDSCAPE PLAN LANDSCAPE NOTES AND DETAILS LUMEN PLAN LUMEN PLAN NOTES & DETAILS ARCHITECTURAL 1ST FLOOR - PLAN ARCHITECTURAL 2ND FLOOR - PLAN ARCHITECTURAL 3RD FLOOR - PLAN A2.1 ARCHITECTURAL EXTERIOR ELEVATIONS ARCHITECTURAL EXTERIOR ELEVATIONS A1.1 DK PARKING DECK 1ST LEVEL CONSTRUCTION & PARKING PLAN A1.2 DK PARKING DECK 2ND LEVEL CONSTRUCTION & PARKING PLAN

PARKING DECK EXTERIOR ELEVATIONS & DETAILS

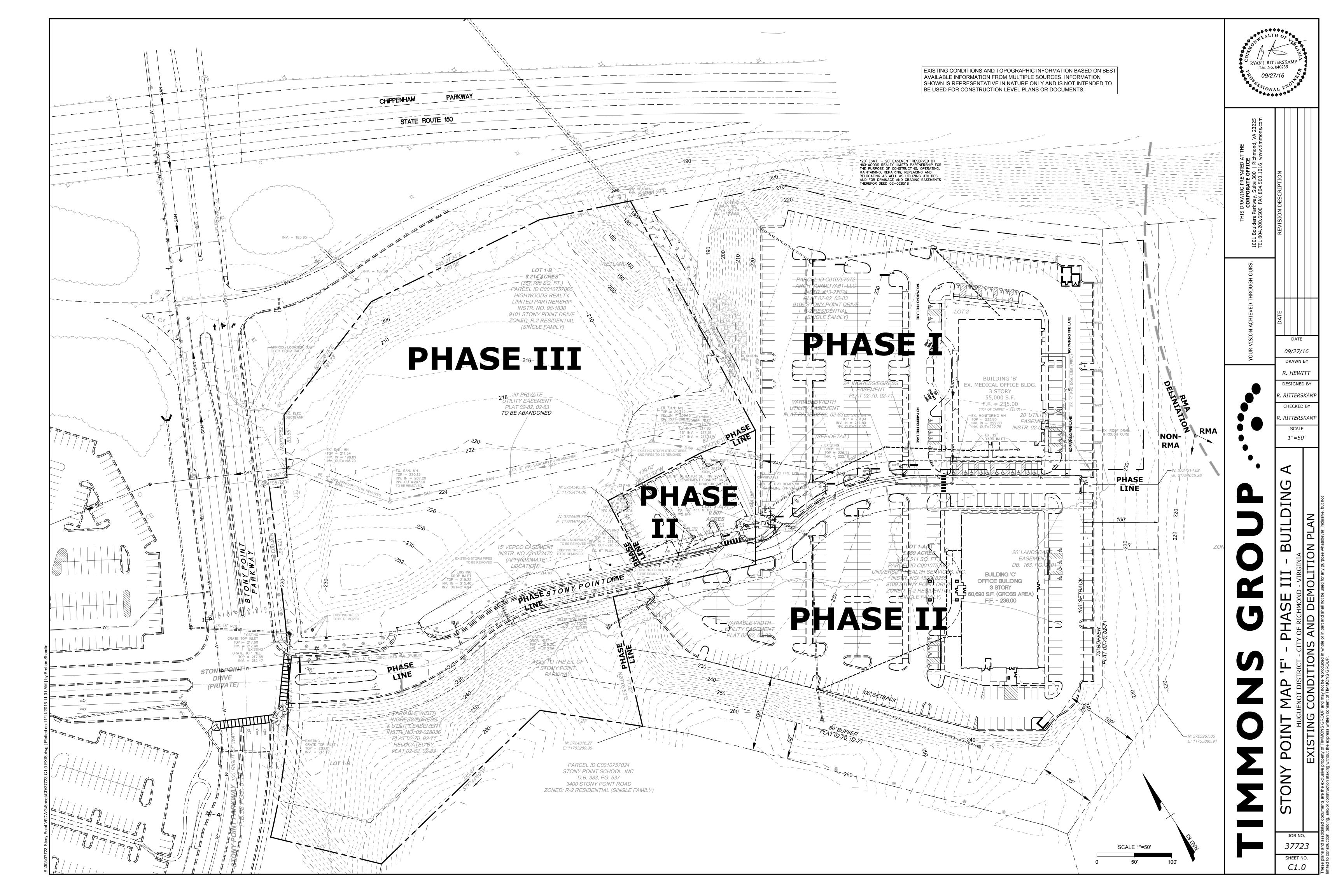
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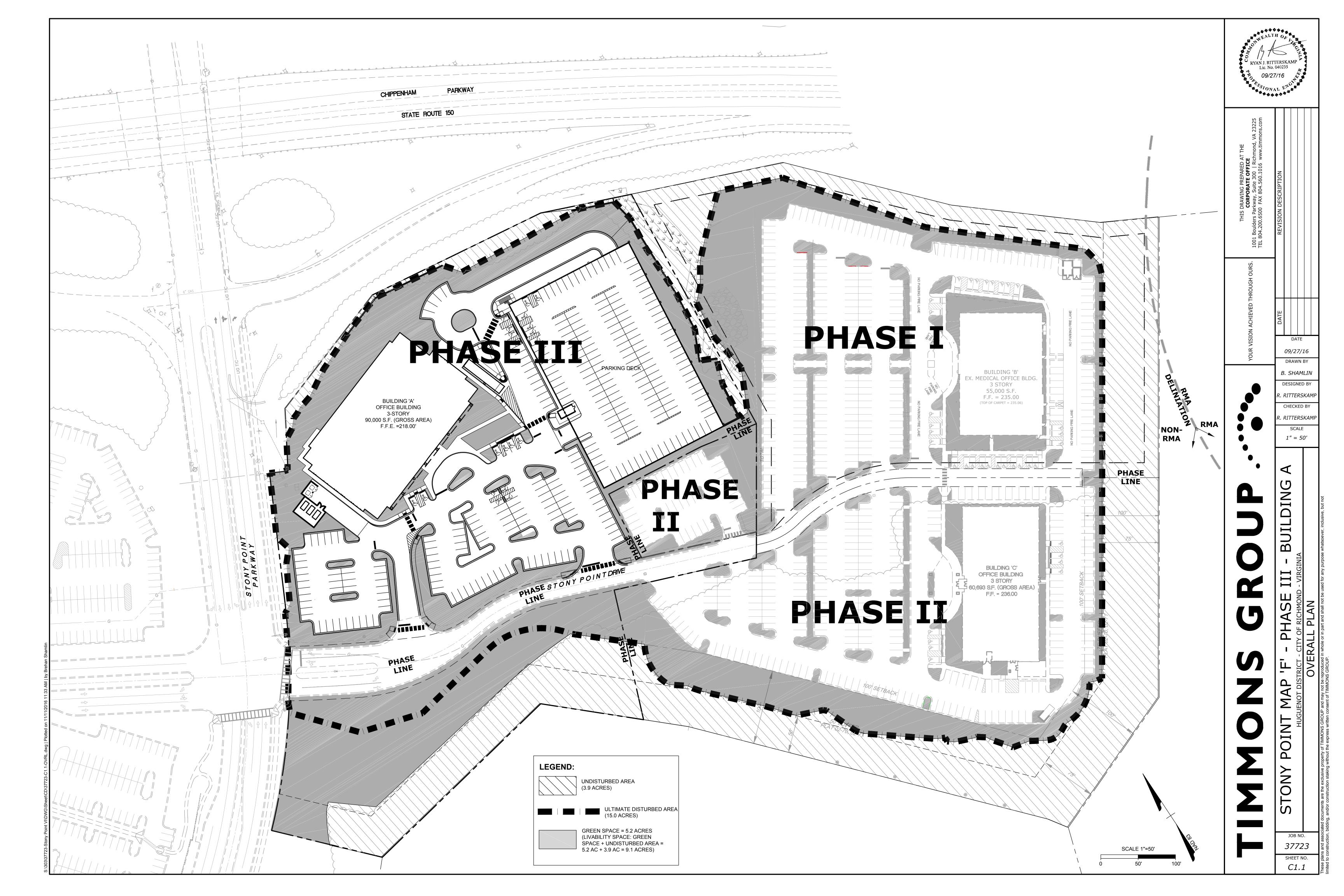
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AS SHOWN

SHEET NO.

37723 C0.0







RYAN J. RITTERSKAMP Lic. No. 040235 09/27/16

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09/27/16 DRAWN BY

B. SHAMLIN DESIGNED BY R. RITTERSKAMP CHECKED BY

. RITTERSKAMP SCALE 1"=30'

37723 SHEET NO. C2.0

PROJECT DESCRIPTION

HF PROJECT IS PHASE III OF A THREE PHASE PROJECT. PHASE III INVOLVES THE CONSTRUCTION OF AN OFFICE BUILDING WITH ASSOCIATED DRIVES, PARKING DECK AND PARKING AREAS. PLEASE REFER TO SHEET C1.1 FOR PROJECT PHASE LINES. A TIMELINE FOR THIS PROJECT HAS NOT YET BEEN ESTABLISHED. UNLESS OTHERWISE NOTED, THIS NARRATIVE REFERS TO THE DISTURBED AREA WITHIN PHASE

THE EXISTING 19.80 ACRE SITE IS MASTER PLANNED FOR THREE PHASES OF DEVELOPMENT. PHASE I & II OF THE PROJECT (VIRGINIA UROLOGY CENTER 7 HIGHWOODS BUILDING C) ARE COMPLETELY CONSTRUCTED AND STABILIZED. AFTER PHASE I COMPLETION OF THE PROJECT, THE ENTIRE 19.80 ACRES WAS SUBDIVIDED INTO TWO PARCELS. THE PHASE I PARCEL MAKES UP 5.45 ± ACRES OF THE ORIGINAL 19.80 ACRES, LEAVING THE RESIDUAL 14.35 ACRE AREA FOR PHASES II AND PHASE III (THIS PLAN; 5.3 ACRES BEING DISTURBED DURING PHASE III). PHASE II CONSISTS OF 4.0 ACRES DISTURBED AREA. THE EXISTING TOPOGRAPHY IN THE PHASE III DISTURBED AREA RANGES FROM 1.5% TO 50% SLOPES AND THERE ARE MINIMAL IF ANY EXISTING DRAINAGE AND EROSION CONTROL ISSUES. THE MAJORITY OF THE EXISTING SITE DRAINS TO A SINGLE 36" CULVERT UNDER CHIPPENHAM PARKWAY. THE PROPOSED DEVELOPMENT DISCHARGES STORMWATER RUNOFF THROUGH STORM SEWER TO THIS SAME POINT. VEGETATION TO BE PRESERVED WILL BE THOSE OUTSIDE THE LIMITS OF DISTURBANCE. THE LIMITS OF DISTURBANCE SHALL BE MARKED PRIOR TO CONSTRUCTION IN A MANOR ACCEPTABLE TO THE CITY OF RICHMOND EROSION CONTROL INSPECTOR. THE PROPOSED LIMITS OF DISTURBANCE ARE SHOWN ON SHEET C2.0.

ADJACENT AREAS:

THE SITE IS BOUNDED ON THE NORTHEAST BY CHIPPENHAM PARKWAY (STATE ROUTE 150). ON THE NORTHWEST BY STONY POINT PARKWAY, TO THE SOUTH BY STONY POINT DRIVE AND TO THE EAST BY BUILDING B. STONY POINT FASHION PARK REGIONAL SHOPPING MALL IS LOCATED ACROSS STONY POINT PARKWAY. A SINGLE SEDIMENT TRAP AND SILT FENCE WILL BE USED ON THE DOWNHILL SIDE OF THE SITE TO CONTROL AND RETAIN SEDIMENT RUNOFF DURING CONSTRUCTION. THE SITE WILL BE FULLY STABILIZED AFTER COMPLETION OF THE PROJECT.

NO OFFSITE LAND DISTURBANCE IS PROPOSED WITH THIS PLAN. EXCESS MATERIALS THAT ARE NOT REUSED ONSITE WILL BE TRANSPORTED TO AN APPROVED DESIGNATED SITE (TO BE DETERMINED).

A SOIL SURVEY OF THE CITY OF RICHMOND WAS COMPLETED BY THE USDA IN 2009. SOILS MAPPING INFORMATION APPEARS ON SHEET C7.0. THE FOLLOWING FOUR SOIL TYPES ARE THE PREDOMINATE SOILS PRESENT ON THE SITE:

- APPLING SANDY LOAM, 12% TO 20% SLOPES (MAPPING UNIT 3D; HYDROLOGIC SOIL GROUP B)- THE EROSION HAZARD IS MODERATE (Kw=0.24 AND Kf=0.28 AT A DEPTH OF O"-10"). DEPTH TO A ROOT RESTRICTIVE LAYER IS GREATER THAN 60 INCHES. THE NATURAL DRAINAGE CLASS IS WELL DRAINED. WATER MOVEMENT IN THE MOST RESTRICTIVE LAYER IS MODERATELY HIGH. AVAILABLE WATER TO A DEPTH OF 60 INCHES IS MODERATE. SHRINK-SWELL POTENTIAL IS LOW. THIS SOIL IS NOT FLOODED. IT IS NOT PONDED. THERE IS NO ZONE OF WATER SATURATION WITHIN A DEPTH OF 72 INCHES. ORGANIC MATTER CONTENT IN THE SURFACE HORIZON IS ABOUT 1 PERCENT. THE ERODABILITY IS MODERATE.
- CECIL-URBAN LAND COMPLEX, 6% TO 12% SLOPES (MAPPING UNIT 8C; HYDROLOGIC GROUP B)—THE CECIL COMPONENT MAKES UP 70 PERCENT OF THE MAP UNIT. THE EROSION HAZARD IS MODERATE (Kw=0.28 AND Kf=0.28 AT A DEPTH OF O"-9"). DEPTH TO A ROOT RESTRICTIVE LAYER IS GREATER THAN 60 INCHES. THE NATURAL DRAINAGE CLASS IS WELL DRAINED. WATER MOVEMENT IN THE MOST RESTRICTIVE LAYER IS MODERATELY HIGH. AVAILABLE WATER TO A DEPTH OF 60 INCHES IS MODERATE. SHRINK-SWELL POTENTIAL IS LOW. THIS SOIL IS NOT FLOODED. IT IS NOT PONDED. THERE IS NO ZONE OF WATER SATURATION WITHIN A DEPTH OF 72 INCHES. ORGANIC MATTER CONTENT IN THE SURFACE HORIZON IS ABOUT 1 PERCENT. THE URBAN LAND COMPONENT MAKES UP 15 PERCENT OF THE MAP UNIT. THE URBAN LAND IS A MISCELLANEOUS AREA. THE ERODABILITY IS MODERATE.
- GROVER FINE SANDY LOAM. 20% TO 35% SLOPES (MAPPING UNIT 19E: HYDROLOGIC SOIL GROUP B)-THE EROSION HAZARD IS MODERATE (Kw=0.24 AND Kf=0.24 AT A DEPTH OF 0"-12"). DEPTH TO A ROOT RESTRICTIVE LAYER IS GREATER THAN 60 INCHES. THE NATURAL DRAINAGE CLASS IS WELL DRAINED. WATER MOVEMENT IN THE MOST RESTRICTIVE LAYER IS MODERATELY HIGH. AVAILABLE WATER TO A DEPTH OF 60 INCHES IS MODERATE. SHRINK-SWELL POTENTIAL IS LOW. THIS SOIL IS NOT FLOODED. IT IS NOT PONDED. THERE IS NO ZONE OF WATER SATURATION WITHIN A DEPTH OF 72 INCHES. ORGANIC MATTER CONTENT IN THE SURFACE HORIZON IS ABOUT 1 PERCENT. THE ERODABILITY IS
- GROVER-WATEREE-URBAN LAND COMPLEX, 6% TO 20% SLOPES (MAPPING UNIT 20C: HYDROLOGIC GROUP B)- THE GROVER COMPONENT MAKES UP 40 PERCENT OF THE MAP UNIT. THE EROSION HAZARD IS MODERATE (Kw=0.24 AND Kf=0.24 AT A DEPTH OF O"-12"). DEPTH TO A ROOT RESTRICTIVE LAYER IS GREATER THAN 60 INCHES. THE NATURAL DRAINAGE CLASS IS WELL DRAINED. WATER MOVEMENT IN THE MOST RESTRICTIVE LAYER IS MODERATELY HIGH. AVAILABLE WATER TO A DEPTH OF 60 INCHES IS MODERATE. SHRINK—SWELL POTENTIAL IS LOW. THIS SOIL IS NOT FLOODED. IT IS NOT PONDED. THERE IS NO ZONE OF WATER SATURATION WITHIN A DEPTH OF 72 INCHES. ORGANIC MATTER CONTENT IN THE SURFACE HORIZON IS ABOUT 1 PERCENT. THE WATEREE COMPONENT MAKES UP 35 PERCENT OF THE MAP UNIT. THE EROSION HAZARD IS MODERATE (Kw=0.24 AND Kf=0.24 AT A DEPTH OF 0"-9"). DEPTH TO A ROOT RESTRICTIVE LAYER, BEDROCK, PARALITHIC, IS 20 TO 40 INCHES. THE NATURAL DRAINAGE CLASS IS WELL DRAINED. WATER MOVEMENT IN THE MOST RESTRICTIVE LAYER IS HIGH. AVAILABLE WATER TO A DEPTH OF 60 INCHES IS VERY LOW. SHRINK-SWELL POTENTIAL IS LOW. THIS SOIL IS NOT FLOODED. IT IS NOT PONDED. THERE IS NO ZONE OF WATER SATURATION WITHIN A DEPTH OF 72 INCHES. ORGANIC MATTER CONTENT IN THE SURFACE HORIZON IS ABOUT 1 PERCENT. THE URBAN LAND COMPONENT MAKES UP 15 PERCENT OF THE MAP UNIT. THE URBAN LAND IS A MISCELLANEOUS AREA. THE ERODABILITY IS MODERATE.

THE PROPOSED LIMITS OF DISTURBANCE ARE SHOWN ON SHEET C2.0. PERIMETER EROSION CONTROL MEASURES WILL CAPTURE SEDIMENT RUNOFF FROM ALL DISTURBED AREAS.

CRITICAL AREAS:

ALL AREAS WITH EXISTING OR PROPOSED STEEP SLOPES (>20%) ARE CRITICAL. ALL EXISTING STEEP SLOPES WITHIN THE LIMITS OF DISTURBANCE HAVE BEEN HIGHLIGHTED ON SHEET C2.0. ALL PROPOSED STEEP SLOPES SHALL BE TREATED WITH BLANKET MATTING AND PERMANENT SEEDING TO ENSURE PROPER STABILIZATION. REFER TO SHEET C2.0 FOR AREAS WHERE BLANKET MATTING AND PERMANENT SEEDING ARE REQUIRED FOR PERMANENT STABILIZATION. TEMPORARY SLOPE DRAINS SHALL BE INSTALLED AS SHOWN ON SHEET C2.0 TO DIVERT STORMWATER RUNOFF AWAY FROM PROPOSED STEEP SLOPES UNTIL THEY ARE FULLY STABILIZED.

EROSION & SEDIMENT CONTROL METHODS:

THE CONTRACTOR SHALL CONSTRUCT AND MAINTAIN ALL MEASURES NECESSARY TO PREVENT SOIL FROM ERODING OR BEING TRACKED ONTO ADJACENT PROPERTY. STREETS. DRAINAGE SYSTEMS. AND WATERWAYS. UNLESS OTHERWISE INDICATED. ALL VEGETATIVE AND STRUCTURAL EROSIONS AND SEDIMENTS CONTROL PRACTICES SHALL BE CONSTRUCTED AND MAINTAINED ACCORDING TO MINIMUM STANDARDS AND SPECIFICATIONS OF THE "VIRGINIA EROSION AND SEDIMENT CONTROL HANDBOOK, 1992 EDITION". THE MINIMUM STANDARDS OF THE VIRGINIA EROSION AND SEDIMENT CONTROL REGULATIONS SHALL BE ADHERED TO UNLESS OTHERWISE WAIVED OR

APPROVED BY A VARIANCE FROM THE CITY OF RICHMOND.

PHASED CONSTRUCTION - CONSTRUCTION SHALL BE PHASED TO MINIMIZE THE EXTENT OF DISTURBED AREA AT ANY GIVEN TIME.

TEMPORARY CONSTRUCTION ENTRANCE (3.02) - A TEMPORARY CONSTRUCTION ENTRANCE SHALL BE INSTALLED AS SHOWN ON THE PLAN

SILT FENCE BARRIER (3.05) - SILT FENCE SEDIMENT BARRIERS SHALL BE INSTALLED DOWNSLOPE OF DISTURBED AREAS TO FILTER SEDIMENT—LADEN RUNOFF TO FROM SHEET FLOWS AND SHALLOW CONCENTRATED FLOWS. ADDITIONALLY, SILT FENCE SHALL BE INSTALLED AROUND ALL STOCKPILES.

INLET PROTECTION (3.07)— INLET PROTECTION SHALL BE INSTALLED TO PREVENT SEDIMENT FROM ENTERING STORM DRAINS OR STRUCTURES.

PERMANENT SEEDING (3.32) - ALL DISTURBED AREAS NOT OTHERWISE STABILIZED SHALL BE STABILIZED WITH PERMANENT SEEDING IMMEDIATELY FOLLOWING FINISH GRADING. SEEDING SHALL BE IN CONFORMANCE WITH STANDARD AND SPECIFICATION 3.32 OF THE HANDBOOK.

BLANKET MATTING (3.36) — ALL DISTURBED CRITICAL AREAS SHALL BE STABILIZED WITH BLANKET MATTING AND PERMANENT SEEDING (3.32) IMMEDIATELY FOLLOWING FINISH GRADING. BLANKET MATTING SHALL BE INSTALLED AS SHOWN ON THE EROSION AND SEDIMENT CONTROL PLAN.

STORMWATER RUNOFF CONSIDERATIONS

STORMWATER RUNOFF IS BEING DISCHARGED INTO AN EXISTING DRAINAGE SYSTEM ONSITE. THE EXISTING SYSTEM HAS BEEN DESIGNED TO ACCEPT ADDITIONAL FLOW FROM THE PROPOSED DEVELOPMENT. AN EXISTING SWM BASIN LOCATED ON THE NORTH SIDE OF CHIPPENHAM PARKWAY WAS DESIGNED TO HANDLE THE STORMWATER MANAGEMENT NEEDS OF THIS DEVELOPMENT. THE OVERALL STORMWATER MANAGEMENT PLAN IS SHOWN ON SHEET C11.3.

- 1. CONSTRUCTION SHALL BE PHASED TO REDUCE THE OVERALL AREA THAT IS NOT STABILIZED AT A GIVEN TIME DURING CONSTRUCTION.
- 2. CONSTRUCTION WILL BE SEQUENCED SO THAT GRADING OPERATIONS CAN BEGIN AND END AS QUICKLY AS POSSIBLE.
- 3. EROSION AND SEDIMENT CONTROL MEASURES WILL BE INSTALLED AS A FIRST STEP IN ANY LAND DISTURBING OPERATIONS.
- 4. TEMPORARY SEEDING OR OTHER STABILIZATION WILL FOLLOW IMMEDIATELY AFTER GRADING.
- 5. AREAS THAT ARE NOT TO BE DISTURBED WILL BE CLEARLY MARKED BY FLAGS, SIGNS, ETC.
- 6. THE CRLD SHALL BE RESPONSIBLE FOR THE INSTALLATION AND MAINTENANCE OF ALL EROSION AND SEDIMENT CONTROL MEASURES.
- 7. AFTER ADEQUATE STABILIZATION IS ACHIEVED, THE TEMPORARY EROSION AND SEDIMENT CONTROL MEASURED WILL BE CLEANED UP AND REMOVED.

<u>MAINTENANCE OF CONTROLS:</u>

ALL EROSION AND SEDIMENT CONTROL MEASURES WILL BE CHECKED DAILY AND AFTER EACH SIGNIFICANT RAINFALL. THE FOLLOWING ITEMS SHALL BE CHECKED IN PARTICULAR:

- 1. THE SILT FENCE BARRIERS WILL BE CHECKED REGULARLY FOR UNDERMINING OR DETERIORATION OF THE FABRIC. SEDIMENT SHALL BE REMOVED WHEN THE LEVEL OF SEDIMENT DEPOSITION REACHES HALF WAY TO THE TOP OF THE BARRIER.
- 2. INLET PROTECTION DEVICES SHALL BE CHECKED REGULARLY AND SHALL BE CLEANED OR REPLACED IF CLOGGING OR EXCESSIVE PONDING IS OBSERVED.
- 3. THE SEEDED AREAS WILL BE CHECKED REGULARLY TO ENSURE THAT A GOOD STAND OF GRASS IS MAINTAINED. AREAS SHALL BE FERTILIZED AND RESEEDED AS NEEDED.
- 4. THE CHANNEL DOWNSTREAM OF THE CULVERT INLET PROTECTION SEDIMENT TRAP SHALL BE CHECKED FOR SEDIMENT BUILDUP.

- 1. THE CONTRACTOR SHALL CONSTRUCT AND MAINTAIN ALL APPROVED MEASURES AS SHOWN ON THE DRAWINGS. ANY ADDITIONAL MEASURES REQUIRED BY THE CITY OF RICHMOND DUE TO FIELD CONDITIONS SHALL BECOME PART OF THE EROSION AND SEDIMENT CONTROL PLAN FOR THE PROPERTY. ALL FIELD CHANGES MUST BE APPROVED BY THE CITY'S INSPECTOR PRIOR TO INSTALLATION.
- 2. THE CONTRACTOR SHALL CONSTRUCT AND MAINTAIN ALL MEASURES TO PREVENT SOIL FROM ERODING OR BEING TRACKED ONTO ADJACENT PROPERTY, STREETS, DRAINAGE SYSTEM AND WATERWAYS. THE CONTRACTOR SHALL ALSO PROVIDE SATISFACTORY MEASURES FOR PREVENTING SOIL FROM ACCUMULATING ON THE WASHRACKS IF NECESSARY. ALL DEVICES SHALL BE CLEANED OF MUD, DEBRIS AND OTHER ERODED MATERIAL DURING THE OPERATION. INSPECTION OF DEVICES SHALL BE A DAILY ROUTINE AND REQUIRED AFTER EVERY RAINFALL EVENT.
- 3. THE CONTRACTOR SHALL MONITOR AND TAKE PRECAUTIONS TO CONTROL DUST, INCLUDING BUT NOT LIMITED TO USING WATER OR CHEMICALS, LIMITING THE NUMBER OF VEHICLES ALLOWED ONSITE, MINIMIZING THE OPERATING SPEED OF ALL VEHICLES, ETC. THE CONTRACTOR WILL BE RESPONSIBLE FOR THE DAILY SWEEPING OF PUBLIC RIGHT-OF-WAY SHOULD SEDIMENT ACCUMULATE ON PAVED SURFACES.
- 4. UNLESS OTHERWISE INDICATED, ALL VEGETATIVE AND STRUCTURAL EROSION AND SEDIMENT CONTROL PRACTICES WILL BE CONSTRUCTED AND MAINTAINED ACCORDING TO MINIMUM STANDARDS AND SPECIFICATIONS OF THE <u>VIRGINIA</u> EROSION AND SEDIMENT CONTROL HANDBOOK AND VIRGINIA REGULATIONS VR 625-02-00 EROSION AND SEDIMENT CONTROL REGULATIONS.
- 5. THE CITY OF RICHMOND ENVIRONMENTAL OFFICE SHALL BE NOTIFIED 24 HOURS IN ADVANCE OF ANY PLANS TO BEGIN CLEARING AND GRADING OPERATIONS.
- 6. ALL EROSION AND SEDIMENT CONTROL MEASURES ARE TO BE PLACED PRIOR TO OR AS THE FIRST STEP IN CLEARING.
- 7. A COPY OF THE APPROVED EROSION AND SEDIMENT CONTROL PLAN SHALL BE MAINTAINED ON THE SITE AT ALL TIMES.
- 8. PRIOR TO COMMENCING LAND DISTURBING ACTIVITIES IN AREAS OTHER THAN INDICATED ON THESE PLANS (INCLUDING, BUT NOT LIMITED TO, OFF-SITE BORROW OR WASTE AREAS), THE CONTRACTOR SHALL SUBMIT A SUPPLEMENTARY EROSION CONTROL PLAN TO THE OWNER FOR REVIEW AND APPROVAL BY THE ENVIRONMENTAL ENGINEERING DEPARTMENT.
- 9. ALL DISTURBED AREAS ARE TO DRAIN TO APPROVED SEDIMENT CONTROL MEASURES AT ALL TIMES DURING LAND DISTURBING ACTIVITIES AND DURING SITE DEVELOPMENT UNTIL FINAL STABILIZATION IS ACHIEVED.
- 10.DURING DEWATERING OPERATIONS, WATER WILL BE PUMPED INTO AN APPROVED FILTERING DEVICE.
- 11.UNDERGROUND UTILITY LINES SHALL BE INSTALLED IN ACCORDANCE WITH THE FOLLOWING STANDARDS IN ADDITION TO OTHER APPLICABLE CRITERIA: A. NO MORE THAN 500 LINER FEET OF TRENCH MAY BE OPENED AT
 - B. EXCAVATED MATERIAL SHALL BE PLACED ON THE UPHILL SIDE OF

- EFFLUENT FROM DEWATERING OPERATIONS SHALL BE FILTERED OR PASSED THROUGH AN APPROVED SEDIMENT TRAPPING DEVICE, OR BOTH, AND DISCHARGED IN A MANNER THAT DOES NOT ADVERSELY AFFECT FLOWING STREAMS OR OFF-SITE PROPERTY.
- RESTABILIZATION SHALL BE ACCOMPLISHED IN ACCORDANCE WITH THESE REGULATIONS.
- E. APPLICABLE SAFETY REGULATIONS SHALL BE COMPLIED WITH.
- 12.ALL DISTURBED AREAS TO BE SEEDED WITHIN SEVEN (7) DAYS AFTER ROUGH GRADING IS COMPLETED, WITH EROSION CONTROL SEEDING WHICH IS NOT FINAL SEEDING. ANY AREA WHICH WILL BE DENUDED FOR THIRTY (30) DAYS MUST BE SEEDED OR TEMPORARILY STABILIZED. (PERMANENT SEEDING SHALL BE INCLUDED AS PART OF THE LANDSCAPE CONTRACTORS RESPONSIBILITIES).
- 13.THE ENVIRONMENTAL DEPARTMENT OF THE CITY OF RICHMOND AND OTHER INTERESTED CITY AGENCIES SHALL MAKE A CONTINUING REVIEW AND EVALUATION OF THE METHOD USED AND THE OVERALL EFFECTIVENESS OF THE EROSION AND SEDIMENT CONTROL PROGRAM. AN APPROVED EROSION AND SEDIMENT CONTROL PLAN MAY BE AMENDED BY THE PLAN APPROVING AUTHORITY IF ON-SITE INSPECTIONS INDICATE THAT CONTROLLING EROSION AND SEDIMENTATION OR, IF BECAUSE OF CHANGED CIRCUMSTANCE, THE APPROVED PLAN CANNOT BE CARRIED OUT.
- 14.EROSION CONTROL STRUCTURES SHALL REMAIN IN PLACE UNTIL GRASS HAS BEEN ESTABLISHED ON THE EXPOSED SOIL SURFACES.

SEQUENCE OF CONSTRUCTION:

<u>PHASE I:</u>

- 1. A PRE-CONSTRUCTION CONFERENCE IS MANDATORY BEFORE ANY WORK IS DONE AT THE SITE. ARRANGE A MEETING WITH THE OWNER, ENGINEER AND CITY E&S INSPECTOR. 48 HOUR NOTICE IS REQUIRED.
- 2. INSTALL PHASE I EROSION AND SEDIMENT CONTROL DEVICES ACCORDING TO THE APPROVED PLANS SET, ONLY DISTURBING AREAS REQUIRED TO INSTALL THE PHASE I MEASURES.
- 3. INSTALL CONSTRUCTION ENTRANCE, SILT FENCE, DIVERSION DIKES AND SEDIMENT BASIN AS SHOWN ON SHEETS C7.0 AND C7.1. ALL ESC MEASURES SHALL BE INSTALLED PER THE VESCH, 1992 EDITION AND PER DETAILS ON SHEETS C7.3-C7.7.

<u>PHASE II:</u>

- 4. AFTER APPROVAL FROM THE CITY ENVIRONMENTAL INSPECTOR, COMMENCE CLEARING, GRUBBING, AND GRADING OPERATIONS. ALL SLOPES SHALL BE STABILIZED IMMEDIATELY UPON INSTALLATION. EXCESS MATERIAL IS TO BE STORED IN AN APPROVED STOCKPILE LOCATIONS ONLY.
- 5. INSTALL THE STORMWATER COLLECTION SYSTEM. INSTALL INLET PROTECTION IMMEDIATELY UPON INSTALLATION OF INLETS ACCORDING TO THE APPROVED PLANS.
- 6. COMMENCE INSTALLATION OF THE UNDERGROUND UTILITIES ONCE PLANNED SUBGRADE ELEVATIONS ARE ACHIEVED.

8. INSTALL CURB AND GUTTER AND COMMENCE ASPHALT PAVING.

- 7. FINE GRADE ROADWAY AREAS AND PLACE SUBBASE AND AGGREGATE BASE.
- 9. TOPSOIL, SEED, AND MULCH THE DISTURBED AREAS. INSTALL LANDSCAPING.
- 10.INSTALL PAVEMENT MARKINGS AND SIGNAGE.
- 11. REMOVE ALL TEMPORARY EROSION AND SEDIMENT CONTROL MEASURES AFTER FINAL STABILIZATION OF THE SITE AND APPROVAL FROM THE CITY ENVIRONMENTAL INSPECTOR.

EROSION CONTROL ONLY— SEEDING

- 1. SEEDING SHALL BE DONE IN CONFORMANCE WITH SECTION 604.03 THROUGH 604.06 OF V.D.O.T. SPECIFICATIONS.
- 2. THE AREA TO BE SEEDED SHALL FIRST BE FERTILIZED WITH COMMERCIAL 10-10-10 FERTILIZER AT THE RATE OF 30 LBS. PER THOUSAND SQUARE FEET AND TREATED WITH AGRICULTURAL LIME AT THE RATE OF 100 LBS. PER THOUSAND SQUARE FEET. THESE SHALL BE UNIFORMLY WORKED INTO SURFACE TO A MINIMUM DEPTH OF ONE INCH.
- 3. SEEDING SHALL BE DONE ONLY BETWEEN THE DATES OF FEB. 15 AND APRIL 15 OR BETWEEN SEPT. 15 AND NOV. 15, EXCEPT AS MAY BE OTHERWISE DIRECTED BY THE ENGINEER.
- 4. SURFACE SHALL BE RAKED AND SMOOTHED TO ELIMINATE RIDGES AND DEPRESSIONS.
- 5. AFTER PRELIMINARY RAKING, THE SEED SHALL BE SOWN AT THE RATE OF FOUR LBS. PER THOUSAND SQUARE FEET AS FOLLOWS: 20% PERENNIAL RYE 35% KENTUCKY 31 FESCUE 30% CREEPING RED FESCUE 15% REDTOP (ALL PERCENTAGES ARE BY WEIGHT)
- 6. SURFACE SHALL THEN BE LIGHTLY RAKED IN ORDER TO COVER SEED NO DEEPER THAN 1/4 INCH AND THEN SPRINKLE WITH WATER. THE SEEDED SURFACE SHALL BE COVERED WITH STRAW OR HAY TO PREVENT EROSION AND TO PROTECT SEEDING. THE ENTIRE SEEDED SURFACE SHALL BE ROLLED WITH A CORRUGATED ROLLER AFTER SEEDING AND BEFORE COVERING WITH STRAW. CONTRACTOR SHALL PROTECT SEEDED SURFACES UNTIL A GOOD STAND OF GRASS IS OBTAINED.
- 7. THE "HYDRO-SEEDING" METHOD OF SEED APPLICATION MAY BE USED, PRO-VIDED THE SEED RATE PER SQUARE FOOT IS THE SAME AS HEREIN BEFORE SPECIFIED. THE MULCH RATE SHALL BE SUCH AS TO PROVIDE PROPER SEED PROTECTION AND PREVENT EROSION. IF, IN THE OPINION OF THE INSPECTOR OR ENGINEER, THE MULCH RATE USED (AS EVIDENCED BY SLOPES AFTER SPRAYING) IS NOT SUFFICIENT, THE CONTRACTOR SHALL BE REQUIRED TO IN-CREASE THE AMOUNT OF MULCH IN THE MIX. NO EXTRA WILL BE ALLOWED FOR ANY REQUESTED INCREASE.

PERMANENT SEEDING STD. & SPEC.

<u>3.32</u>

PLANT SELECTION

REFER TO TABLE 3.32-D FOR SEEDING MIXTURES FOR THE PIEDMONT AREA. <u>SEEDBED PREPARATION</u> 1. LIMING IN THE PIEDMONT REGION, APPLY 2 TONS/ACRE PULVERIZED

AGRICULTURAL GRADE LIMESTONE (90 LBS./1,000 SQ, FT,). 2. FERTILIZER SHALL BE APPLIED AS 1000 LBS./ACRE OF 10-20-10 (23 LBS./SQ. FT.) OR EQUIVALENT NUTRIENTS. LIME AND FERTILIZER SHALL BE INCORPORATED INTO THE TOP 4-6 INCHES OF THE SOIL BE DISCING OR OTHER MEANS.

1. CERTIFIED SEED WILL BE USED FOR ALL PERMANENT SEEDING WHENEVER POSSIBLE.

2. LEGUME SEED SHOULD BE INOCULATED WITH THE INOCULANT APPROPRIATE TO THE SPECIES. SEED OF THE LESPEDIZAS, THE CLOVERS AND THE CROWNVETCH SHOULD BE SCARIFIED TO PROMOTE UNIFORM GERMINATION 3. APPLY SEED UNIFORMLY WITH A BROADCAST SEEDER, DRILL, CULTI—PACKER SEEDER, OR HYDROSEEDER ON A FIRM, FRIABLE SEEDBED. SEEDING DEPTH SHOULD BE 1/4 TO 1/2 INCH. MULCHING ALL PERMANENT SEEDING MUST BE MULCHED IMMEDIATELY UPON COMPLETION OF SEED APPLICATION.

NEW SEEDINGS SHOULD BE PROVIDED WITH ADEQUATE MOISTURE.

<u>RE-SEEDING</u>

SITE

CONDITIONS

MAINTENANCE

MINIMUM

CARE

LAWN

SLOPE

(3:1 OR

LESS)

LOW-

MAINTENANCE

SLOPE

(STEEPER

THAN 3:1)

LAWNS

INSPECT SEEDED AREAS FOR FAILURE AND MAKE NECESSARY REPAIRS AND RE-SEEDINGS WITHIN THE SAME SEASON, IF POSSIBLE.

SEEDING MIXTURES

IMPROVED PERENNIAL RYEGRASS 0%-5%

TURF-TYPE TALL FESCUE OR 95%-100% 175-200

TURF-TYPE TALL FESCUE OR

KENTUCKY 31 FESCUE

KENTUCKY BLUEGRASS

KENTUCKY 31 FESCUE

KENTUCKY 31 FESCUE

SEASONAL NURSE CROP*

SEASONAL NURSE CROP *

RED TOP GRASS

RED TOP GRASS

CROWN VETCH**

FEBRUARY 16th THROUGH APRIL

MAY 1st THROUGH AUGUST 15th.

AUGUST 16th THROUGH OCTOBER

NOVEMBER THROUGH FEBRUARY 15th..

TABLE 3.32-D SITE SPECIFIC SEEDING MIXTURES FOR

PIEDMONT AREA

128 LBS.

20 LBS.

108 LBS.

20 LBS.

20 LBS.

* USE SEASONAL NURSE CROP IN ACCORDANCE WITH SEEDING DATES A STATED BELOW:

MIX DURING WARMER SEEDING PERIODS; ADD 10-20 LBS./ACRE IN MIXES

** SUBSTITUTE SERICEA LESPEDEZA FOR CROWNVETCH EAST OF FARMVILLE, VA. (MAY THROUGH

SEPTEMBER USE HULLED SERICEA, ALL OTHER PERIODS, USE UNHULLED SERICEA). IF FLATPEA IS

PROPERLY INOCULATED. WEEPING LOVEGRASS MAY BE ADDED TO ANY SLOPE OR LOW-MAINTENANCE

USED IN LIEU OF CROWNVETCH, INCREASE RATE TO 30 LBS./ACRE. ALL LEGUME SEED MUST BE

2 LBS.

2 LBS.

PER

ACRE

200-250

150

LBS.

LBS.

RATES *SEASONAL NURSE CROP - DATES

ŤΟ

5/1 | 8/16 |

4/30 | 8/15 | 10/31 | 2/15

ANNUAL FOXTAIL ANNUAL WINTER

RYE | MILLET | RYE | RYE

ANNUAL | FOXTAIL | ANNUAL | WINTER

RYE | MILLET | RYE | RYE

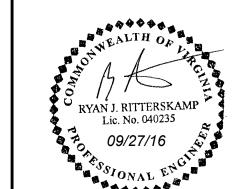
ANNUAL RYE

ANNUAL RYE

.WINTER RYE

FOXTAIL MILLET

11/1



ARRESTANT OF THE PROPERTY OF T

DATE 09/27/16 DRAWN BY

B. SHAMLIN DESIGNED BY RITTERSKAMI CHECKED BY

RITTERSKAM SCALE

Z

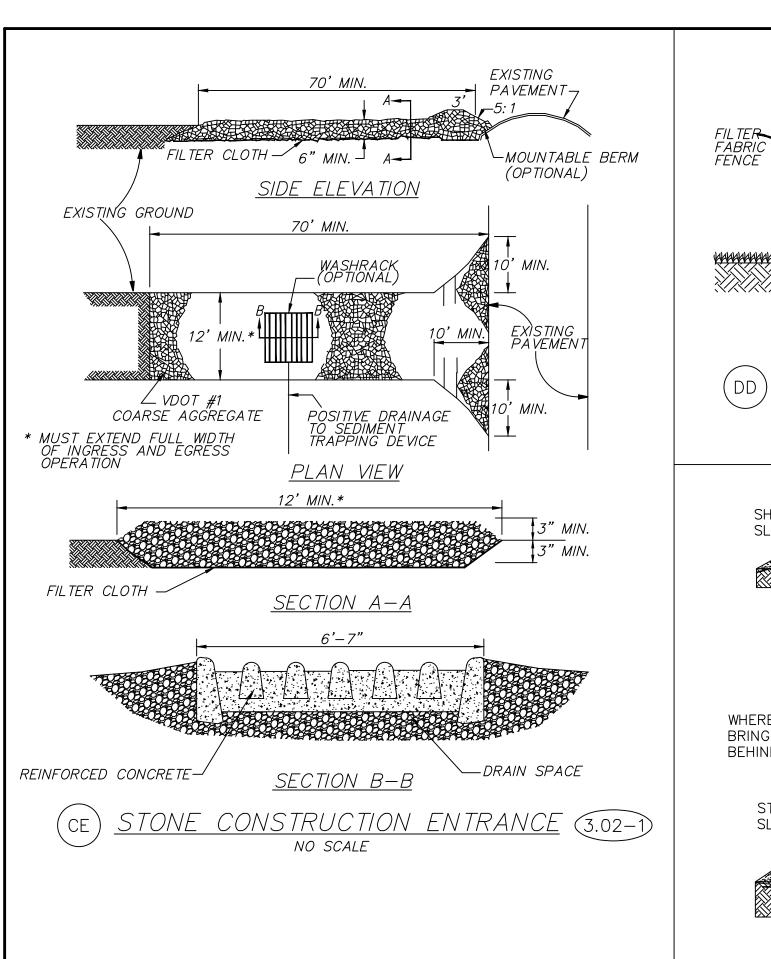
 $\mathbf{\Omega}$ 7

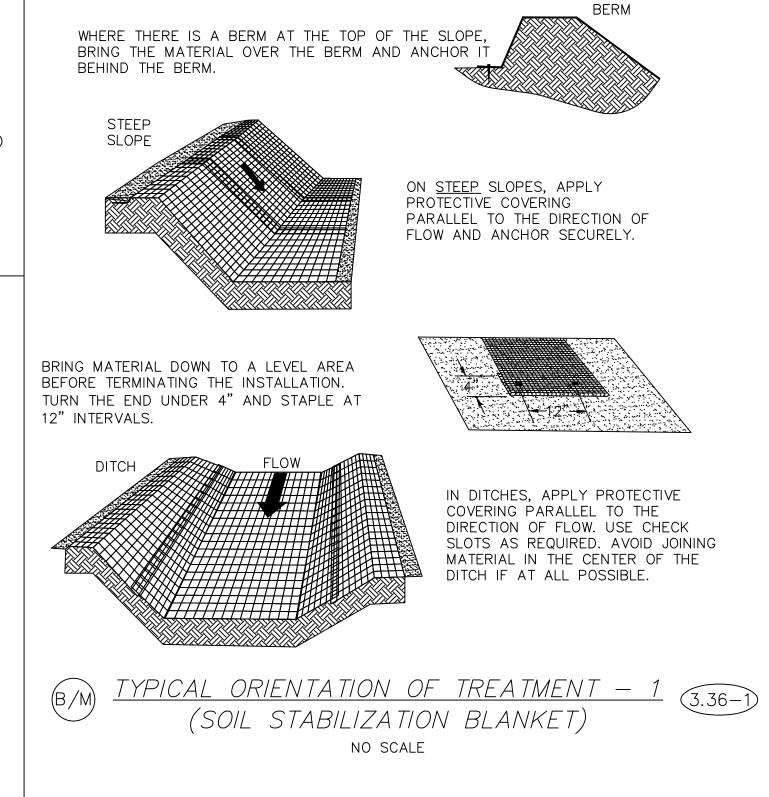
Δ Σ

 \vdash 0 ک

JOB NO. 37723

SHEET NO. C2.1





- COMPACTED SOIL

ON SHALLOW SLOPES, STRIPS OF NETTING PROTECTIVE

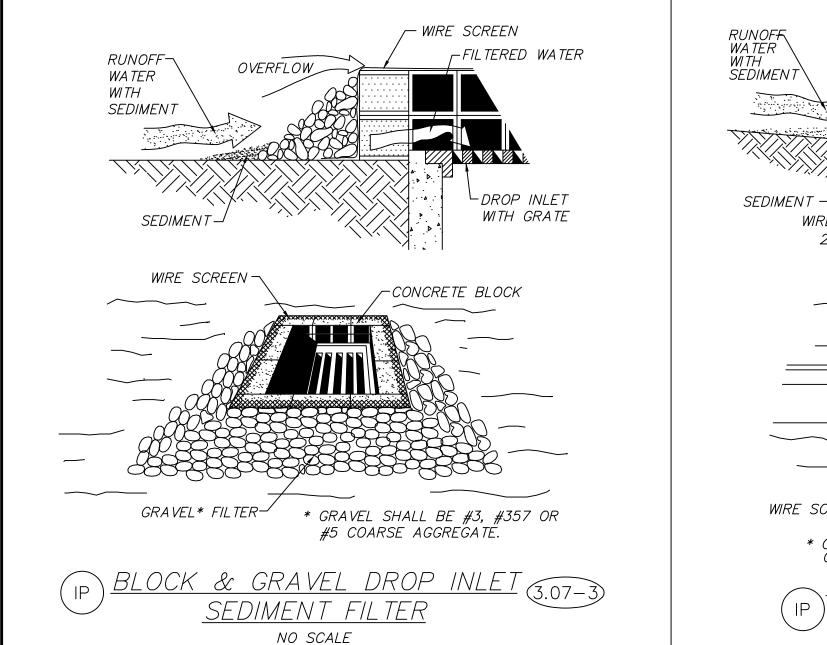
COVERINGS MAY BE APPLIED

ACROSS THE SLOPE.

FILTER FABRIC EMBEDDED 3" INTO GROUND

TEMPORARY DIVERSION DIKE

NO SCALE



ENDWALL

CULVERT

TOE OF FILL

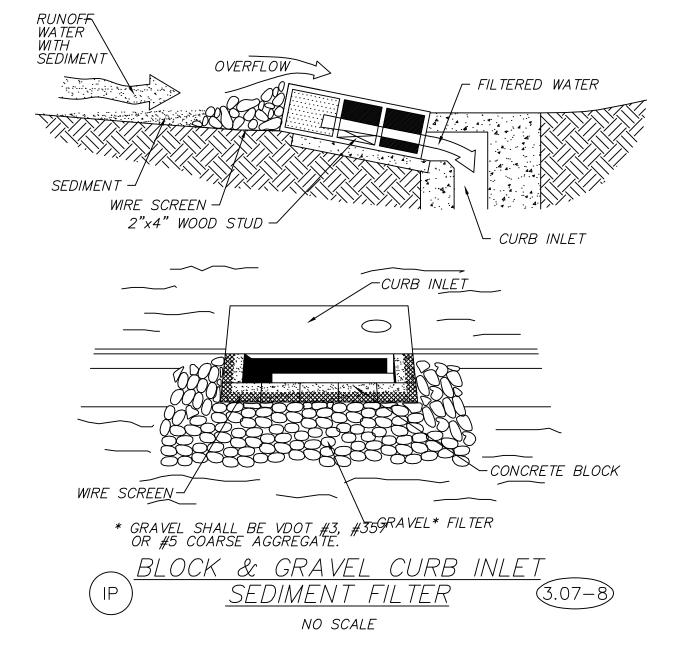
(3.08-1)

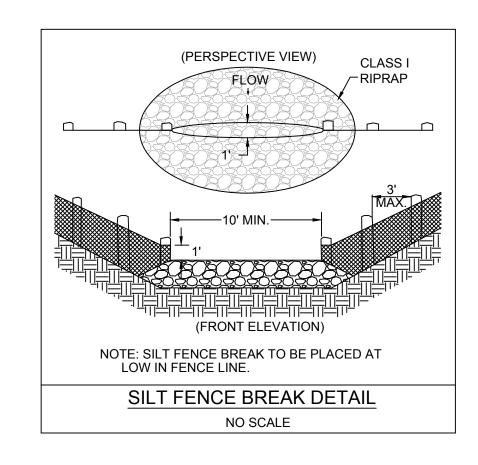
TOE OF FILL -

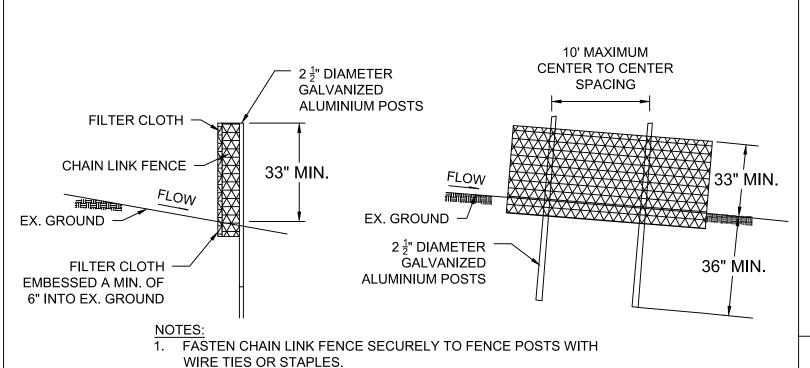
SILT FENCE -

SILT FENCE

CULVERT INLET PROTECTION



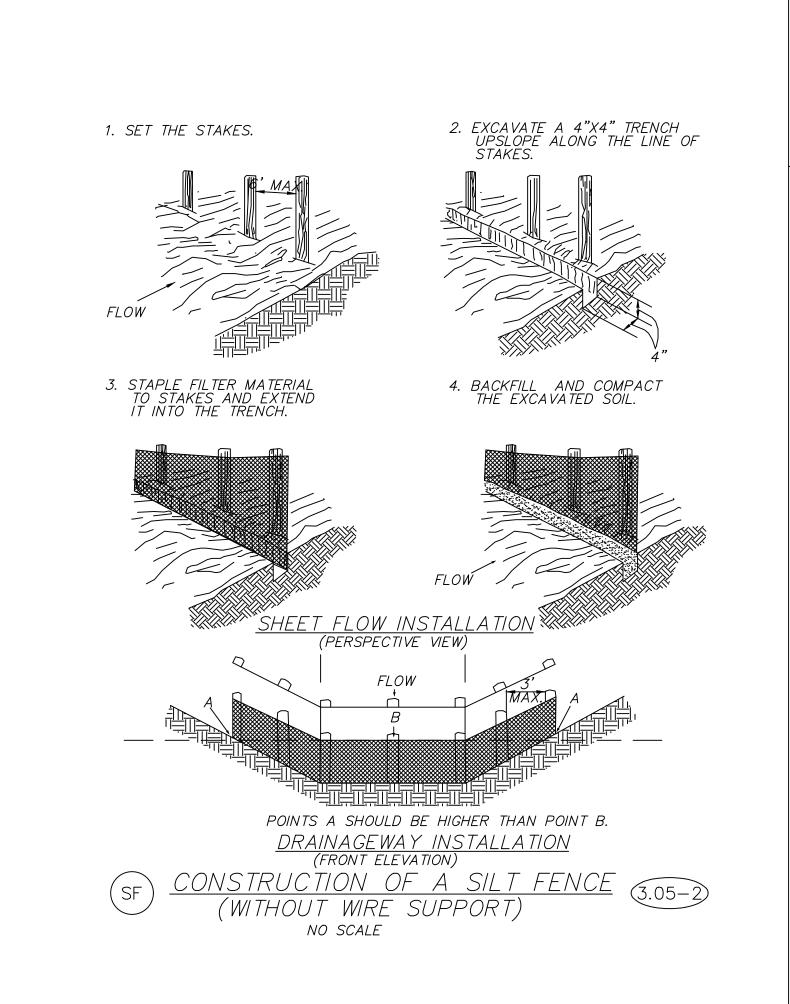


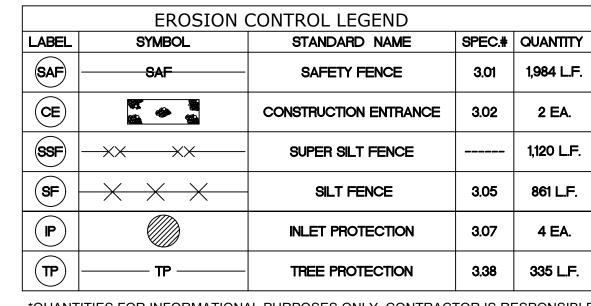


TIES SPACED A MINIMUM OF 24" ALONG POST. 3. EMBED FILTER CLOTH A MINIMUM OF 6" INTO EXISTING 4. OVERLAP AND FOLD ADJOINING SECTIONS OF FILTER CLOTH.

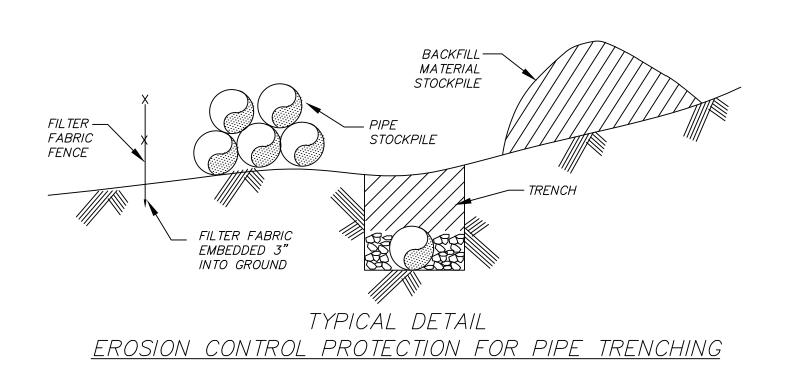
2. FASTEN FILTER CLOTH SECURELY TO CHAIN LINK FENCE WITH

SUPER SILT FENCE DETAIL NO SCALE

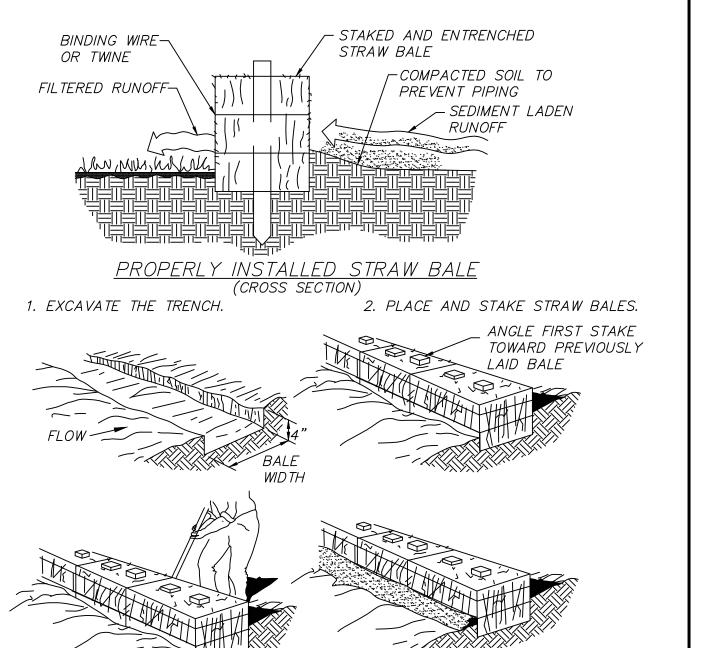




*QUANTITIES FOR INFORMATIONAL PURPOSES ONLY. CONTRACTOR IS RESPONSIBLE FOR PERFORMING THEIR OWN QUANTITY TAKE OFF.

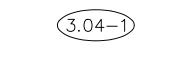


NO SCALE NOTE: AN ACCEPTABLE ALTERNATE TO FILTER FABRIC FENCE IS A LINE OF STAKED STRAW BALES



3. WEDGE LOOSE STRAW BETWEEN BALES. 4. BACKFILL AND COMPACT THE EXCAVATED SOIL. CONSTRUCTION OF STRAW BALE BARRIER

NO SCALE



RYAN J. RITTERSKAMP Lic. No. 040235 09/27/16

DATE 09/27/16 DRAWN BY B. SHAMLIN **DESIGNED BY** . RITTERSKAMP

CHECKED BY RITTERSKAMI SCALE N/A

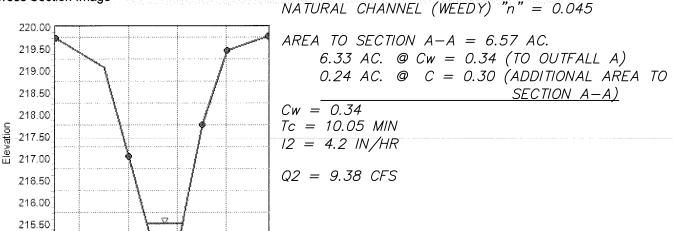
JOB NO. 37723 SHEET NO.

C2.2

Normal Depth Solve For Input Data

Channel Slope Normal Depth Discharge

Cross Section Image



0.02000 ft/ft

0.58 ft

9.38 ft³/s

Cross Section for Section C-C

Project Description

Manning Formula Friction Method Normal Depth Solve For

215.00

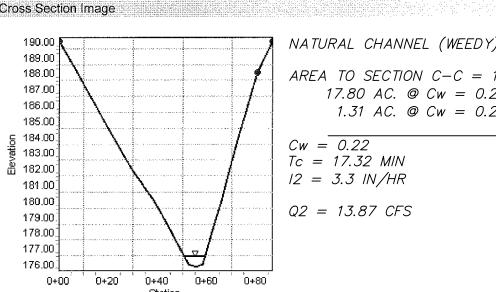
0+00 0+10

Input Data

0.03500 ft/ft Channel Slope 0.65 ft Normal Depth 13.87 ft³/s Discharge

0+20 0+30

Cross Section Image



NATURAL CHANNEL (WEEDY) "n" = 0.045 AREA TO SECTION C-C = 19.11 AC. 17.80 AC. @ Cw = 0.22 (TO OUTFALL C) 1.31 AC. @ Cw = 0.25 (ADDITIONAL AREA TO SECTION C-C)

VESCP MINIMUM STANDARDS

9VAC25-840-40. Minimum standards.

A VESCP must be consistent with the following criteria, techniques and

1. Permanent or temporary soil stabilization shall be applied to denuded areas within seven days after final grade is reached on any portion of the site. Temporary soil stabilization shall be applied within seven days to denuded areas that may not be at final grade but will remain dormant for longer than 14 days. Permanent stabilization shall be applied to areas that are to be left dormant for more than one year.

2. During construction of the project, soil stock piles and borrow areas shall be stabilized or protected with sediment trapping measures. The applicant is responsible for the temporary protection and permanent stabilization of all soil stockpiles on site as well as borrow areas and soil intentionally transported from the project site.

3. A permanent vegetative cover shall be established on denuded areas not otherwise permanently stabilized. Permanent vegetation shall not be considered established until a ground cover is achieved

that is uniform, mature enough to survive and will inhibit erosion. 4. Sediment basins and traps, perimeter dikes, sediment barriers and other measures intended to trap sediment shall be constructed as a

first step in any land-disturbing activity and shall be made functional

5. Stabilization measures shall be applied to earthen structures such

before upslope land disturbance takes place.

as dams, dikes and diversions immediately after installation.

6. Sediment traps and sediment basins shall be designed and constructed based upon the total drainage area to be served by the

a. The minimum storage capacity of a sediment trap shall be 134 cubic yards per acre of drainage area and the trap shall only control drainage

b. Surface runoff from disturbed areas that is comprised of flow from drainage areas greater than or equal to three acres shall be controlled by a sediment basin. The minimum storage capacity of a sediment basin shall be 134 cubic yards per acre of drainage area. The outfall basin during a 25-year storm of 24-hour duration. Runoff coefficients used in runoff calculations shall correspond to a bare earth condition or those conditions expected to exist while the sediment basin is utilized.

7. Cut and fill slopes shall be designed and constructed in a manner that will minimize erosion. Slopes that are found to be eroding excessively within one year of permanent stabilization shall be provided with additional slope stabilizing measures until the problem is

8. Concentrated runoff shall not flow down cut or fill slopes unless contained within an adequate temporary or permanent channel, flume

9. Whenever water seeps from a slope face, adequate drainage or other protection shall be provided.

10. All storm sewer inlets that are made operable during construction. shall be protected so that sediment-laden water cannot enter the conveyance system without first being filtered or otherwise treated to

11. Before newly constructed stormwater conveyance channels or pipes are made operational, adequate outlet protection and any

required temporary or permanent channel lining shall be installed in both the conveyance channel and receiving channel.

12. When work in a live watercourse is performed, precautions shall be aken to minimize encroachment, control sediment transport and stabilize the work area to the greatest extent possible during construction. Nonerodible material shall be used for the construction of causeways and cofferdams. Earthen fill may be used for these tructures if armored by nonerodible cover materials

13. When a live watercourse must be crossed by construction vehicles more than twice in any six-month period, a temporary vehicular stream crossing constructed of nonerodible material shall be provided.

14. All applicable federal, state and local requirements pertaining to working in or crossing live watercourses shall be met.

15. The bed and banks of a watercourse shall be stabilized immediately after work in the watercourse is completed.

16. Underground utility lines shall be installed in accordance with the following standards in addition to other applicable criteria:

a. No more than 500 linear feet of trench may be opened at one time.

b. Excavated material shall be placed on the uphill side of trenches.

c. Effluent from dewatering operations shall be filtered or passed

through an approved sediment trapping device, or both, and discharged in a manner that does not adversely affect flowing streams

d. Material used for backfilling trenches shall be properly compacted in order to minimize erosion and promote stabilization

e. Restabilization shall be accomplished in accordance with this

f. Applicable safety requirements shall be complied with

17. Where construction vehicle access routes intersect paved or public roads, provisions shall be made to minimize the transport of sediment by vehicular tracking onto the paved surface. Where sediment is transported onto a paved or public road surface, the road surface shall be cleaned thoroughly at the end of each day. Sediment shall be emoved from the roads by shoveling or sweeping and transported to a sediment control disposal area. Street washing shall be allowed only after sediment is removed in this manner. This provision shall apply to individual development lots as well as to larger land-disturbing

18. All temporary erosion and sediment control measures shall be removed within 30 days after final site stabilization or after the temporary measures are no longer needed, unless otherwise authorized by the VESCP authority. Trapped sediment and the disturbed soil areas resulting from the disposition of temporary measures shall be permanently stabilized to prevent further erosion

19. Properties and waterways downstream from development sites shall be protected from sediment deposition, erosion and damage due o increases in volume, velocity and peak flow rate of stormwater runoff for the stated frequency storm of 24-hour duration in accordance with the following standards and criteria. Stream restoration and relocation projects that incorporate natural channel design concepts are not man made channels and shall be exempt from any flow rate capacity and velocity requirements for natural or man-made channels

h. All on-site channels must be verified to be adequate

a. Concentrated stormwater runoff leaving a development site shall be discharged directly into an adequate natural or man-made receiving channel, pipe or storm sewer system. For those sites where runoff is discharged into a pine or pine system, downstream stability analyses at the outfall of the pipe or pipe system shall be performed.

Cross Section for Section B-B

0.02000 ft/ft

Cw = 0.46

 $T_{C} = 11.91 \, MIN$

 $12 = 4.0 \, IN/HR$

 $Q2 = 32.73 \ CFS$

1.04 ft

32.73 ft³/s

NATURAL CHANNEL (WEEDY) "n" = 0.045

7.13 AC. @ Cw = 0.61 (TO OUTFALL B)

 $4.09 \ AC. \ @ \ C = 0.30 \ (ADDITIONAL \ AREA \ TO$

SECTION B-B)

AREA TO SECTION B-B = 17.79 AC.

Manning Formula

Normal Depth

0+00 0+05 0+10 0+15 0+20 0+25

Project Description

Cross Section Image

180.50

180.00

179.50

179.00

178.00

177.50

Friction Method

Solve For

Input Data

Channel Slope

Normal Depth

Discharge

(1) The applicant shall demonstrate that the total drainage area to the

point of analysis within the channel is one hundred times greater than the contributing drainage area of the project in question; or

(2) (a) Natural channels shall be analyzed by the use of a two-year storm to verify that stormwater will not overtop channel banks nor cause erosion of channel bed or banks.

(b) All previously constructed man-made channels shall be analyzed by the use of a ten-year storm to verify that stormwater will not overtop its banks and by the use of a two-year storm to demonstrate that stormwater will not cause erosion of channel bed or banks; and

(c) Pipes and storm sewer systems shall be analyzed by the use of a ten-year storm to verify that stormwater will be contained within the

c. If existing natural receiving channels or previously constructed manmade channels or pipes are not adequate, the applicant shall:

(1) Improve the channels to a condition where a ten-year storm will not overtop the banks and a two-year storm will not cause erosion to the channel, the bed, or the banks; or

(2) Improve the pipe or pipe system to a condition where the ten-year storm is contained within the appurtenances;

(3) Develop a site design that will not cause the pre-development peak runoff rate from a two-year storm to increase when runoff outfalls into a natural channel or will not cause the pre-development peak runoff rate from a ten-year storm to increase when runoff outfalls into a man-made

(4) Provide a combination of channel improvement, stormwater detention or other measures which is satisfactory to the VESCP

d. The applicant shall provide evidence of permission to make the

e. All hydrologic analyses shall be based on the existing watershed characteristics and the ultimate development condition of the subject

f. If the applicant chooses an option that includes stormwater detention, he shall obtain approval from the VESCP of a plan for maintenance of the detention facilities. The plan shall set forth the maintenance requirements of the facility and the person responsible for performing the maintenance.

g. Outfall from a detention facility shall be discharged to a receiving channel, and energy dissipators shall be placed at the outfall of all detention facilities as necessary to provide a stabilized transition from the facility to the receiving channel.

i. Increased volumes of sheet flows that may cause erosion or sedimentation on adjacent property shall be diverted to a stable outlet, adequate channel, pipe or pipe system, or to a detention facility.

i. In applying these stormwater management criteria, individual lots or parcels in a residential, commercial or industrial development shall not b. Adequacy of all channels and pipes shall be verified in the following be considered to be separate development projects. Instead, the development, as a whole, shall be considered to be a single development project. Hydrologic parameters that reflect the ultimate development condition shall be used in all engineering calculations.

> k. All measures used to protect properties and waterways shall be employed in a manner which minimizes impacts on the physical, chemical and biological integrity of rivers, streams and other waters of

> I. Any plan approved prior to July 1, 2014, that provides for stormwater

management that addresses any flow rate capacity and velocity requirements for natural or man-made channels shall satisfy the flow rate capacity and velocity requirements for natural or man-made channels if the practices are designed to (i) detain the water quality volume and to release it over 48 hours; (ii) detain and release over a 24-hour period the expected rainfall resulting from the one year, 24hour storm; and (iii) reduce the allowable peak flow rate resulting from the 1.5, 2, and 10-year, 24-hour storms to a level that is less than or equal to the peak flow rate from the site assuming it was in a good forested condition, achieved through multiplication of the forested peal flow rate by a reduction factor that is equal to the runoff volume from the site when it was in a good forested condition divided by the runoff volume from the site in its proposed condition, and shall be exempt from any flow rate capacity and velocity requirements for natural or man-made channels as defined in any regulations promulgated pursuant to § 62.1-44.15:54 or 62.1-44.15:65 of the Act.

m. For plans approved on and after July 1, 2014, the flow rate capacity and velocity requirements of § 62.1-44.15:52 A of the Act and this subsection shall be satisfied by compliance with water quantity requirements in the Stormwater Management Act (§ 62.1-44.15:24 et seg, of the Code of Virginia) and attendant regulations, unless such land-disturbing activities are in accordance with <u>9VAC25-870-48</u> of the Virginia Stormwater Management Program (VSMP) Regulations.

n. Compliance with the water quantity minimum standards set out in 9VAC25-870-66 of the Virginia Stormwater Management Program VSMP) Regulations shall be deemed to satisfy the requirements of subdivision 19 of this subsection

Statutory Authority

§ 62.1-44.15:52 of the Code of Virginia.

Historical Notes

Former 4VAC50-30-40, derived from VR625-02-00 § 4; eff September 13, 1990; amended, Virginia Register Volume 11, Issue 11, eff. March 22, 1995; Volume 29, Issue 4, eff. November 21, 2012; amended and renumbered, Virginia Register Volume 30, Issue 2, eff. October 23,

MS-19 ANALYSIS:

IN ACCORDANCE WITH MINIMUM STANDARD 19 OF THE FROSION AND SEDIMENT CONTROL REGULATIONS, ADEQUACY OF RECEIVING CHANNELS OF PIPES MUST BE VERIFIED BY ADDRESSING ONE OF THE FOLLOWING ADEQUACY SITUATIONS:

THE DRAINAGE AREA FROM THE PROJECT AT THE DISCHARGE POINT IS LESS THAN OR EQUAL TO ONE PERCENT (1%) OF THE TOTAL DRAINAGE AREA AT THE DISCHARGE POINT (PROJECT DRAINAGE AREA AND TOTAL DRAINAGE AREA ARE REQUIRED),

NATURAL CHANNELS MUST BE ANALYZED TO DEMONSTRATE THAT (1) THE TWO-YEAR STORM WILL NOT OVERTOP THE CHANNEL BANKS AND (2) THE TWO—YEAR STORM WILL NOT CAUSE EROSION OF THE CHANNEL BED OR BANKS (Q-CAPACITY, Q-2 AND V-2 ARE REQUIRED), (EXCEPT Q-CAPACITY AND Q-2 ARE NOT APPLICABLE IF THE CHANNEL IS IN THE 100-YEAR FLOODPLAIN, RPA, OR WETLANDS),

OR

MAN-MADE CHANNELS MUST BE ANALYZED TO DEMONSTRATE THAT (1) THE TEN-YEAR STORM WILL NOT OVERTOP THE CHANNEL BANKS AND (2) THE TWO—YEAR STORM WILL NOT CAUSE EROSION OF THE CHANNEL BED OR BANKS, OR (3) THE 10-YEAR STORM WILL NOT CAUSE EROSION OF THE CHANNEL BED OR BANKS FOR IMPROVEMENTS WITHIN COUNTY DRAINAGE EASEMENTS (Q-CAPACITY, Q-10, AND V-2/V-10 ARE REQUIRED),

PIPES AND STORM SEWER SYSTEMS MUST BE ANALYZED TO DEMONSTRATE THAT THE 10-YEAR STORM WILL BE CONTAINED WITHIN THE SYSTEM (Q-CAPACITY, Q-10, AND HYDRAULIC GRADE LINE CALCULATIONS ARE REQUIRED),

RUNOFF IS DISCHARGED THROUGH AN ENERGY DISSIPATOR WHERE FLAT POORLY DRAINED TOPOGRAPHY EXISTS WITHIN THE LIMITS OF THE 100-YEAR FLOODPLAIN, RPA, OR WETLAND AND WHEN ACCEPTABLE VELOCITIES ARE DEMONSTRATED PER ITEM B, (A FORMAL WRITTEN VARIANCE REQUEST MUST BE SUBMITTED TO AND APPROVED BY CITY OF RICHMOND ENVIRONMENTAL ENGINEERING),

OR

THE PRE-DEVELOPMENT RUNOFF RATE IS MAINTAINED FOR THE Q-2(NATURAL) OR Q-10(MAN-MADE) STORM EVENT DURING THE SAME POST-DEVELOPED STORM EVENT DISCHARGING INTO A NATURAL OR MAN-MADE CONVEYANCE SYSTEM,

A COMBINATION OF THE ABOVE METHODS TO INCLUDE DETENTION AND DRAINAGE IMPROVEMENTS, OR OTHER MEASURES ACCEPTABLE TO THE CITY OF RICHMOND DEPARTMENT OF ENVIRONMENTAL ENGINEERING TO PREVENT DOWNSTREAM EROSION (A FORMAL WRITTEN VARIANCE REQUEST MUST BE SUBMITTED TO AND APPROVED BY CITY OF RICHMOND ENVIRONMENTAL ENGINEERING).

MS-19 SUMMARY								
DISCHARGE POINT	ADEQUACY SITUATION	PROJECT DRAINAGE AREA (AC)	TOTAL DRAINAGE AREA (AC)	Q-CAPACITY	Q-2 Q-2/Q-10	V-2 V-2/V-10	CROSS SECTION, PROFILE, DA MAP AND CALCULATIONS SHOWN ON SHEET	
Α	В	1.71	4.25	936	6.07	2.01	C7.5, C11.0, C11.2	
В	В	4.41	15.23	963	28.02	4.66	C7.5, C11.0, C11.2	
С	В	1.56	19.11	14,204	13.87	3.61	C7.5,C11.0,C11.2,C11.3	
D-D	С	0.19	0.19	4.29	0.8/1.0	1.14/1.25	C7.6, C11.1	
E-E	С	0.24	0.24	4.29	1.1/1.5	1.28/1.40	C7.6, C11.1	
F-F	С	0.09	0.09	4.18	0.4/0.6	0.92/1.02	C7.6, C11.1	
G-G	С	0.12	0.12	4.18	0.6/0.8	1.01/1.11	C7.6, C11.1	

GENERAL EROSION AND SEDIMENT CONTROL NOTES:

(VESC Control Handbook Table 6-1)

ES-1: Unless otherwise indicated, all vegetative and structural erosion and sediment control practices will be constructed and maintained according to minimum standards and specifications of the Virginia Erosion and Sediment Control Handbook and the Virginia Erosion and Sediment Control Regulations 9VAC25-840.

ES-2: The plan approving authority must be notified one week prior to the pre-construction conference, one week prior to the commencement of land disturbing activity, and one week prior to the final inspection.

ES-3: All erosion and sediment control measures are to be placed prior to or as the first step in clearing.

ES-4: A copy of the approved erosion and sediment control plan shall be maintained on the site at all times.

ES-5: Prior to commencing land disturbing activities in areas other than indicated on these plans (including, but not limited to, off-site borrow or waste areas), the contractor shall submit a supplementary erosion control plan to the owner for review and approval by the plan approving authority.

ES-6: The contractor is responsible for installation of any additional erosion control measures necessary to prevent erosion and sedimentation as determined by the plan approving authority.

ES-7: All disturbed areas are to drain to approved sediment control measures at all times during land disturbing activities and during site development until final stabilization is achieved.

ES-8: During dewatering operations, water will be pumped into an approved filtering

ES-9: The contractor shall inspect all erosion control measures periodically and after each runoff-producing rainfall event. Any necessary repairs or cleanup to maintain the effectiveness of the erosion control devices shall be made immediately.

Richmond Standard E+S Notes

6.86

1.82

0.41

0.96

6.96

2.13

1.62

1 20

11.91

6.70

17.32

10.63

84.00

1.30

78.00

11.60

140.00

Total Tc=

0.25

Total Tc=

1. Permanent or temporary soil stabilization shall be applied to denuded areas within seven days after final grade is reached on any portion of the site. Temporary soil stabilization shall be applied within seven days to denuded areas that may not be at final grade but will remain, dormant (undisturbed) for longer than 14 days. Permanent stabilization shall be applied to areas that are to be left dormant for more than one

2. Excess excavation disposed of off the site shall be disposed of in accordance with the Virginia Erosion and Sediment Control Handbook.

3. Erosion and Sediment Controls shall be installed in accordance with Virginia Erosion and Sediment Control Handbook and shall be placed prior to or as the first step of the land disturbing activity.

4. Erosion and Sediment Controls shall be maintained so that the sediment carrying runoff from the site will not enter storm drainage facilities.

5. Erosion and Sediment Controls shall be maintained until the disturbed area is

6. Properties adjoining the site shall be kept clean of mud or silt carried from the site by vehicular traffic or runoff. 7. The disposal of waste materials removed from erosion and sediment control facilities

and the disposal of these facilities shall be in accordance with the Virginia Erosion and Sediment Control Handbook. 8. Stabilization measures shall be applied to earthen structures such as dams, dikes and diversions immediately after installation. During construction of the project, soil stockpiles shall be stabilized or protected with sediment trapping measures. The applicant is responsible for the temporary protection and permanent stabilization of all

soil stockpiles on site as well as soil intentionally transported from the project site. 9. During construction of the project, soil stockpiles shall be stabilized or protected with sediment trapping measures. The applicant is responsible for the temporary protection and permanent stabilization of all soil stockpiles on site as well as soil intentionally transported from the project site.

RYAN J. RITTERSKAMP Lic. No. 040235 09/27/16

STONAL STONAL

CORPORATE OFFICE
arkway, Suite 300 | Richm
0 FAX 804.560.1016 ww

09/27/16

DRAWN BY B. SHAMLIN **DESIGNED BY** . RITTERSKAMI CHECKED BY

RITTERSKAMI SCALE

N/A

JOB NO. 37723

SHEET NO. C2.3

465.00 torm Sewer Flow Time (from Manning's Equation, Refer to Storm Sewer Calculations) Channel Flow, Tc=0.00948H^{-0.38}L^{1.1} TIME OF CONCENTRATION TO SECTION "B-B' Ground Character Overland Flow, $T_c = 0.225L^{0.42}S^{-0.19}C^{-1.0}$ FOREST 50.00 0.1200 hannel Flow, Tc=0.00948H^{-0.38}L^{1.13} 522.00 torm Sewer Flow Time (from Manning's Equation, Refer to Storm Sewer Calculations) Channel Flow, T_c=0.00948H^{-0.38}L^{1..} 165.00 TIME OF CONCENTRATION TO SECTION "C-C" Ground Character Overland Flow, $T_c = 0.225L^{0.42}S^{-0.19}C^{-1.0}$ 0.0600 100.00

Ground Character

FOREST

Overland Flow, $T_c = 0.225L^{0.42}S^{-0.19}C^{-1.0}$

Channel Flow, Tc=0.00948H^{-0.38}L^{1.1}

Channel Flow, Tc=0.00948H^{-0.38}L^{1.}

TIME OF CONCENTRATION TO SECTION "A-A"

0.1300

Page **2** of **2**

	TIME OF CONCENTRATION TO EXISTING CLUVERT UNDER CHIPPENHAM PARKWAY									
ID	Ground Character	Length	Slope	С	Н	Tc				
Ш	Glound Character	(ft)	(ft/ft)	C	(ft)	(min)				
Overland Flow, $T_c = 0.225L^{0.42}S^{-0.19}C^{-1.0}$										
1	FOREST	50.00	0.1200	0.25		6.96				
Channel Flow, Tc=0.00948H ^{-0.38} L ^{1.13}										
2		522.00			78.00	2.13				
Storm Sewe	Storm Sewer Flow Time (from Manning's Equation, Refer to Storm Sewer Calculations)									
3						1.74				
Channel Flo	Channel Flow, Tc=0.00948H ^{-0.38} L ^{1.13}									
4		165.00			11.90	1.19				
				Tota	l Te=	12.02				

AREA TO EXISTING CULVERT UNDER CHIPPENHAM = 16.50 AC. 4.01 AC. @ Cw = 0.34 (TO OUTFALL A) 7.13 AC. \bigcirc Cw = 0.61 (TO OUTFALL B) 5.36 AC. @ C = 0.30 (ADDITIONAL AREA TO CULVERT) Cw = 0.44Tc = 12.02 MIN $12 = 3.9 \, IN/HR$ 110 = 5.3 IN/HR $125 = 6.0 \, \text{IN/HR}$

Q2 = 28.31 CFSQ10 = 38.48 CFSQ25 = 43.56 CFS

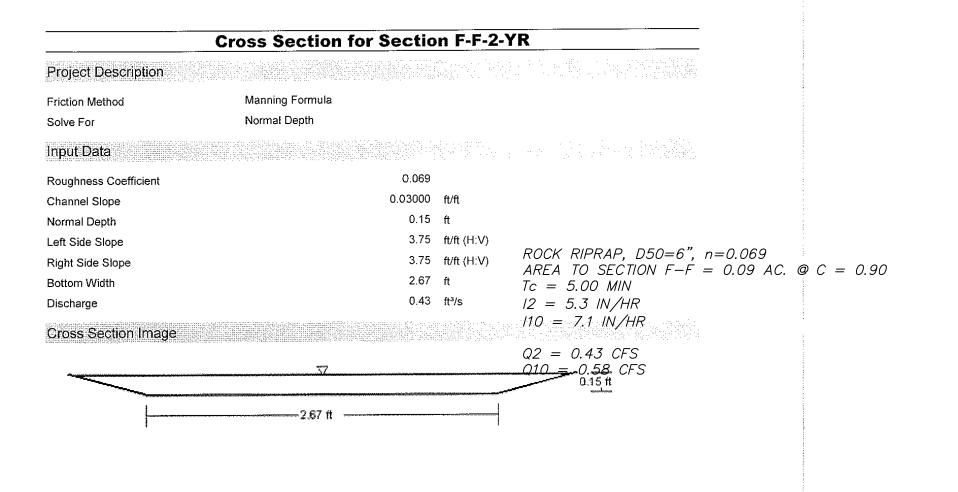
STONY P	OINT PAR	CEL "F"	1														CUL	VERT DES	IGN FORM LI	D-269
						COUNTY	Richmone	d (city)				SHEET		OF			DESIGNER:	ACW	DATE:	1/15/20
EX CULV	ERT		~~~~~~~				VA					UNITS		ENGLISE	·I]	REVIEWER:		DATE:	
				Į.											· · · · · · · · · · · · · · · · · · ·		<u> </u>			
	OGICAL DA	ATA															•			
INPUT			_				Roadway	y Width	55	ft	RO	ADWAY EI	LEVATION	202.00	ft		Road Length	1000	Suface Type	PAVED
0						overvina este, tito e travescore	:>											Freeboard=	21.78	fl .
ration	5			Shì	dr. Elev. Lt. =	203,30	ft		¬ ,								Sh	ldr. Elev. Rt.=	201.60	fi
VS					ELHWd=	179.82	ft 🔻	-	- ỷ /							·	· -			
	FLOW (cfs)			Elst=	176.64	ft		Hwi									→ H			
Design	38.48			Eli=	176.64	ft	_		\				··				₹	TW DEPTH	1.35	ft
Check	28.31			L=	306	ft		+ -	-		Original	tream Bed	_				TW	TW VEL,	5.28	ñ
Max.	43.56			S≃	2.17%			_					<u> </u>	<u> </u>		_	cı	nannel Inv.El	170,00	ft
SCRIPTION	: :			SKEW=	= 0	C										Elo=	170,00	ft		
Single/Mult	iple Conforn	ning			************											-		=		
ription:				TOTAL	FLOW PER				H:	EADWATI	ER CAL	CULATION	NS				CONTROL	<u> </u>	MINIMUM	
cadwall				FLOW	BARREL		INLET CO	NTROL				O	UTLET CON	TROL			HEADWATER	OUTLET	SHOULDER	COMMEN"
Conforming	/ Broken Bac	k Culverts		Q	Q/N	HWi/D	HWi	FALL	ELHWi	TW	dc	(dc+D)/2	ho	ke	н	ELHWo	ELEV	VEL	ELEV.	
SHAPE		N N	Mannings	cfs	cfs		ft	ft	ft	fì	ft	ft	fit		ft	n.	n	fps	ſt.	
		1	•	38.48	38.5	1.06	3.18	0.00	179.82	1.35	າດາ			0.50	1.84		179.82			
					 		 		1											
				-	-				1								 	1		
				13.50	1 13.0	7.70	3.17	0.00	100.15	1.11	2.13	2.50	2.50	0.50			100,75	13,70		
Culvert	<u> </u>		TAILWA	TER DATA:	<u></u>	L		<u> </u>	TAILWAT	ER RESU	LTS:			DOADV	VAV DAT	<u> </u>	<u></u>	ROADWA	YOVERTOR	PING:
	ex em ()]				1		Flow	Valority	Shear force	i i		Α		1	Overtopping	Overtoppin
Liev.	SKEW							1			depth	•		 				-	Discharge	Elevation
			Bottom Widt	th, ft		u=	0.045		cts								PAVED	cis		ft
			Side Slope	Lt: (H:1V)					Design							ın, tt	202	Design	_	0.00
	<u> </u>								Check		<u> </u>			Length of	Road, ft		1000	Check		0.00
<u> </u>	l.,l		1	ope, ft/ft	0.0450		1	1	Max.	171.41	1.41	5.45	3.97	 				Max,	0	0.00
			"n" =											<u> </u>	· . • · · · · · · · · · · · · · · · · ·					
FOOTNOTE	S:																			• •
FOR BOX C	ULVERTS							(4) ELh	i = HWi + El	i (INVERT	OF INL	ET CONTR	OL SECTIO	N)		(6) ho = TW	or (dc + D/2) (W	HICHEVER	IS GREATER)
V/D OR HWi	/D FROM DE	ESIGN CH	IARTS					(5) TW	BASED ON	DOWNSTE	REAM C	ONTROL O	R FLOW			(7) H = [1 +	ke + (29n ² L)/R ¹	$v^{2/2}$ $v^{2/2}$		
/I - (ELHWd	- ELsf); FAL	L IS ZERO	O FOR CULY	VERTS ON GR	ADE			DEPT	H IN CHAN	INEL						•				
				 		COMMENTS	S / DISCUS						·			CULVERT	BARREL SEL	ECTED		
		ŧ.	i	INLET											SIZE:					
																			J.:	
	23														ENTRAN	~ C .		CI DICIP		
HW IN OU	TLET CONT	'R∩I	sf	Streambed		1														
	EX CULV HYDROL INPUT 0 ration 'S Check Max. SCRIPTION Single/Multi ription: cadwall Conforming SHAPE Circular Culvert Elev. COTNOTE FOR BOX CU //D OR HWi II - (ELHWd EFINITION DESIGN H	CHIPPENHAM PAR EX CULVERT HYDROLOGICAL D/ INPUT 0 ration 5 //S FLOW (cfs) Design 38.48 Check 28.31 Max. 43.56 CRIPTION: Single/Multiple Conforming / Broken Bac SHAPE Size (in) Circular 36 Culvert Elev. SKEW COTNOTES: FOR DOX CULVERTS //D OR HWI/D FROM DI II - (ELHWd - ELsf); FAL EFINITIONS: DESIGN HEADWATER	CHIPPENHAM PARKWAY EX CULVERT HYDROLOGICAL DATA INPUT 0 ration 5 /S FLOW (cfs) Design 38.48 Check 28.31 Max. 43.56 SCRIPTION: Single/Multiple Conforming ription: radwall Conforming / Broken Back Culverts SHAPE Size (in) N Circular 36 1 Culvert Elev. SKEW Culvert Elev. SKEW COTNOTES: FOR BOX CULVERTS COTOTOTES: FOR BOX CULVERTS FOR BOX CULVERTS FOR HWI/D FROM DESIGN CHE FOR HWI/D FROM DESIGN CHE CULVERTS FOR HWI/D FROM DESIGN CHE FOR H	### CULVERT HYDROLOGICAL DATA	CHIPPENHAM PARKWAY	CHIPPENHAM PARKWAY	CHIPPENHAM PARKWAY EX CULVERT	CHIPPENHAM PARKWAY	COUNTY Richmond (eity)	CHIPPENHAM PARKWAY	CHIPPENHAM PARKWAY	COUNTY Richmond (city) Roadway Width 55 ft RO.	CHIPPENHAM PARKWAY	COUNTY Rickmond (Gity)	CHIPPENHAM PARKWAY	County C	COUNTY Red-mond (ally) SHEET	CRUPTENHAM PARKWAY	CHIPPENIAM PARKWAY	CHIPPEN

Cross Section for Section D-D-2-YR Project Description Manning Formula Friction Method Normal Depth Solve For Input Data 0.069 Roughness Coefficient ROCK RIPRAP, D50=6", n=0.069 0.03160 ft/ft Channel Slope AREA TO SECTION D-D = 0.19 AC. @ Cw = 0.770.20 ft Normal Depth Tc = 5.00 MIN3.75 ft/ft (H:V) I2 = 5.3 IN/HRLeft Side Slope 110 = 7.1 IN/HR3.75 ft/ft (H:V) Right Side Slope 2.67 ft Q2 = 0.78 CFSBottom Width Q10 = 1.04 CFS0.78 ft³/s Discharge Cross Section Image Cross Section for Section D-D-10-YR Project Description Manning Formula Friction Method Normal Depth Solve For Input Data 0.069 Roughness Coefficient 0.03160 ft/ft Channel Slope 0.23 ft Normal Depth 3.75 ft/ft (H:V) Left Side Slope 3.75 ft/ft (H:V) Right Side Slope 2.67 ft Bottom Width

1.04 ft³/s

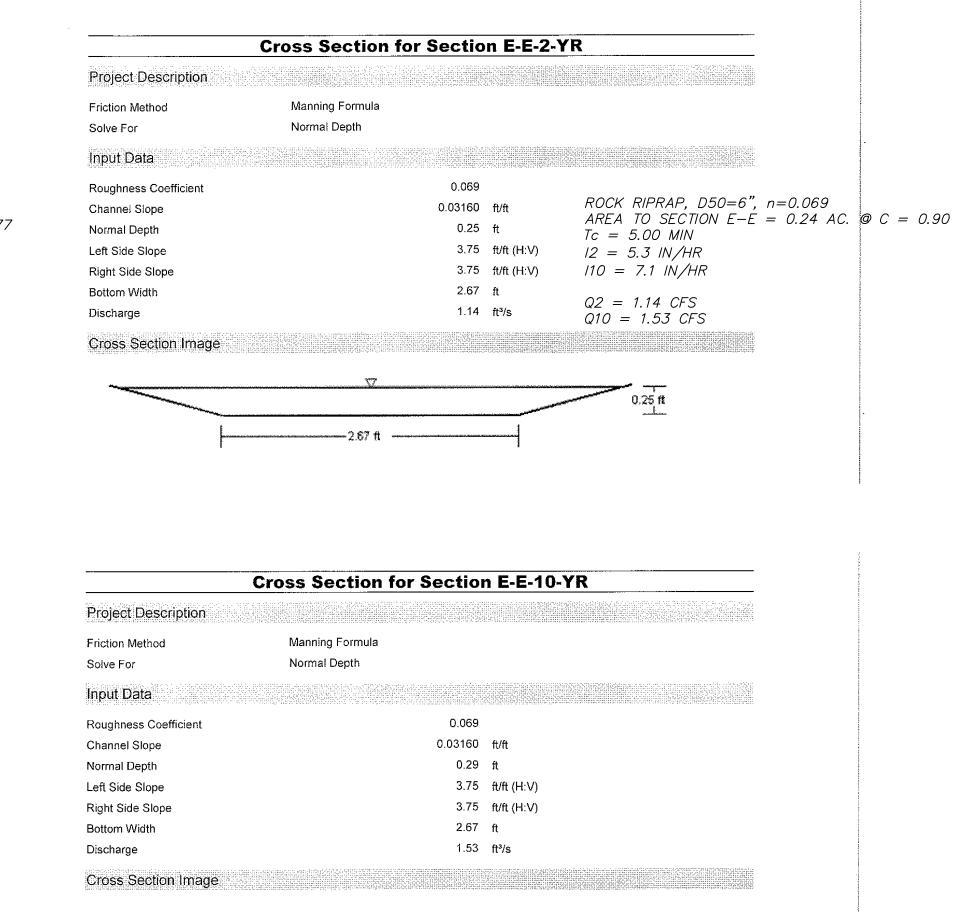
Discharge

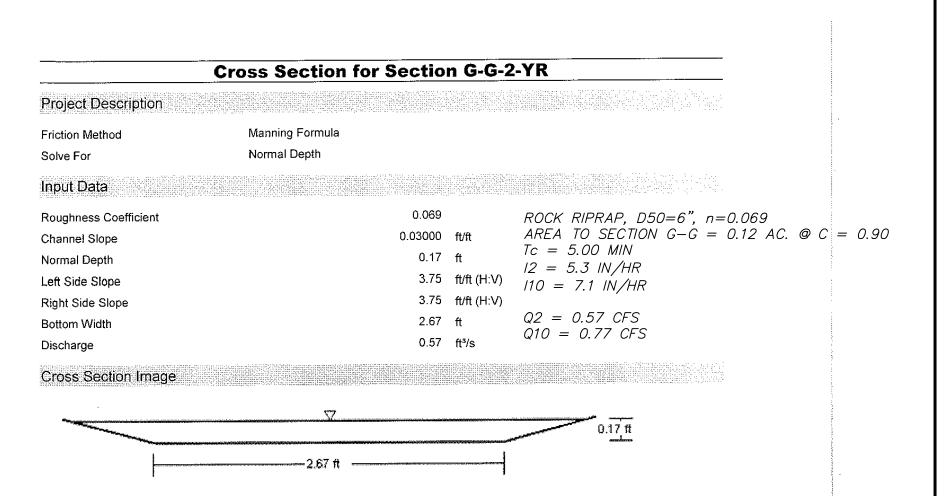
Cross Section Image

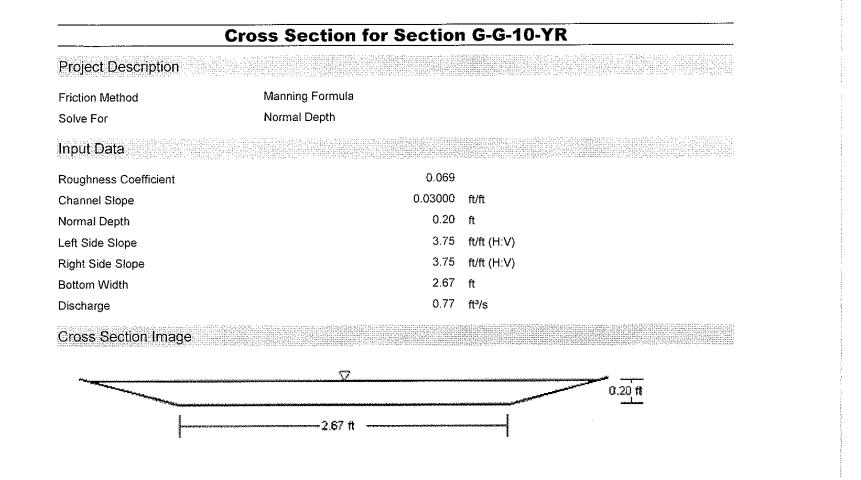


Friction Method	Manning Formula			
Solve For	Normal Depth			
Input Data				100 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1
Roughness Coefficient		0.069		
Channel Slope		0.03000	ft/ft	
Normal Depth		0.17	ft	
Left Side Slope		3.75	ft/ft (H:V)	
Right Side Slope		3.75	ft/ft (H:V)	
Sottom Width		2.67	ft	
Discharge		0.58	ft³/s	
Cross Section Image				
			stitut (d. f. i.	2010101010000
4				
			0.17 ft	

Cross Section for Section F-F-10-YR







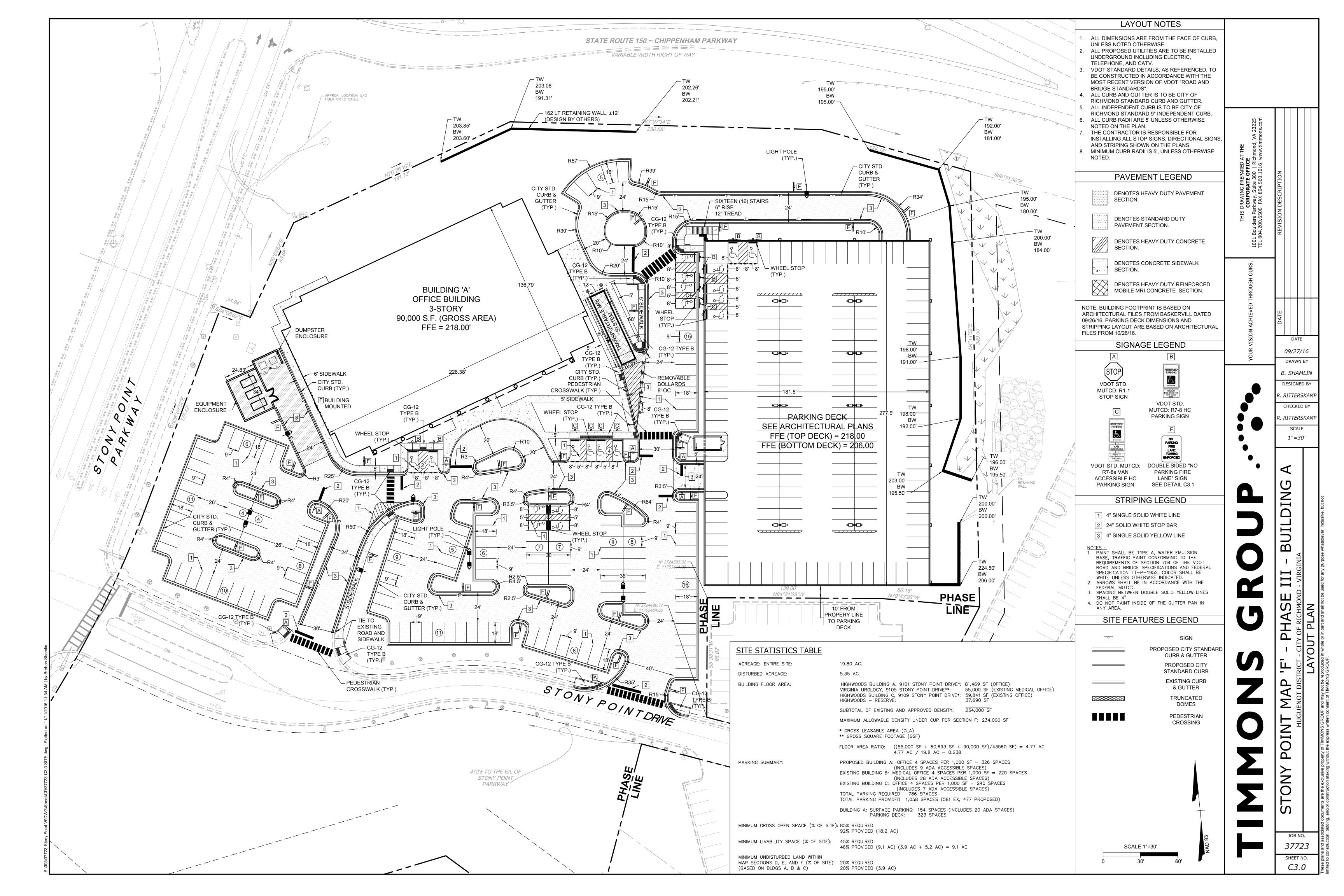


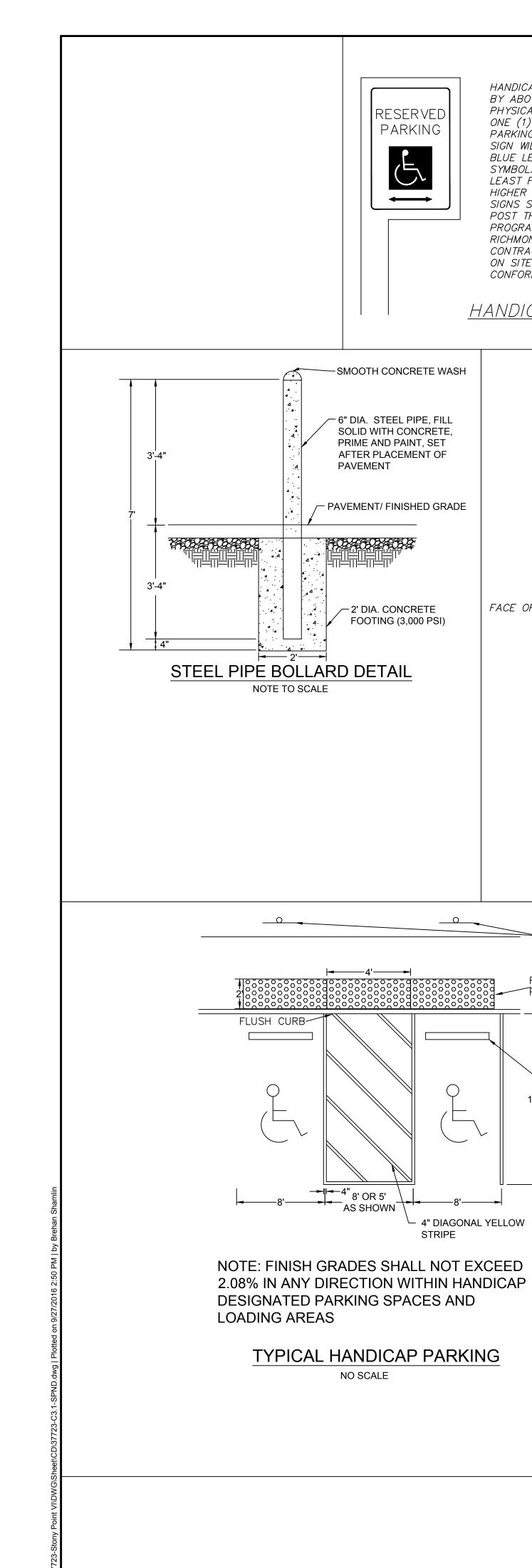
DATE 09/27/16 DRAWN BY

B. SHAMLIN DESIGNED BY . RITTERSKAMP CHECKED BY . RITTERSKAMI

SCALE

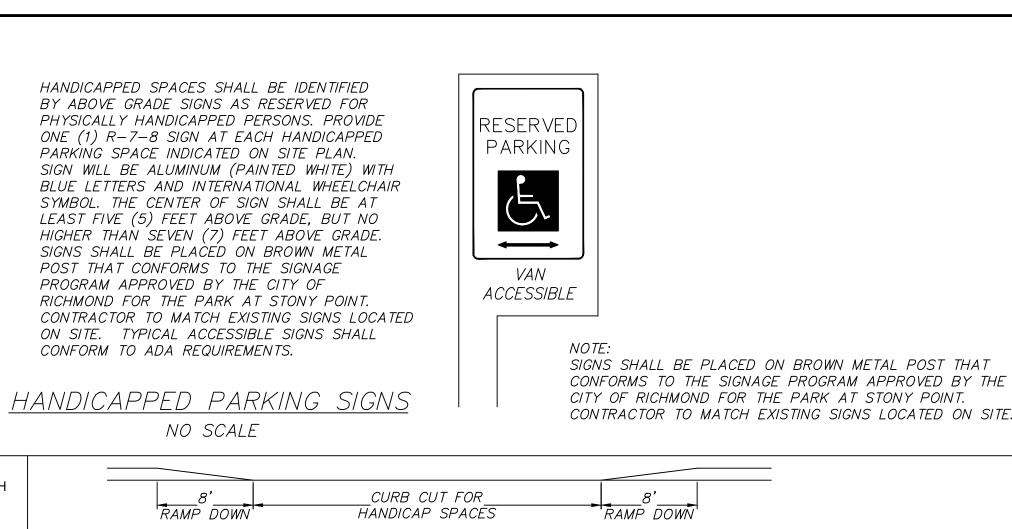
JOB NO. 37723 SHEET NO. C2.4

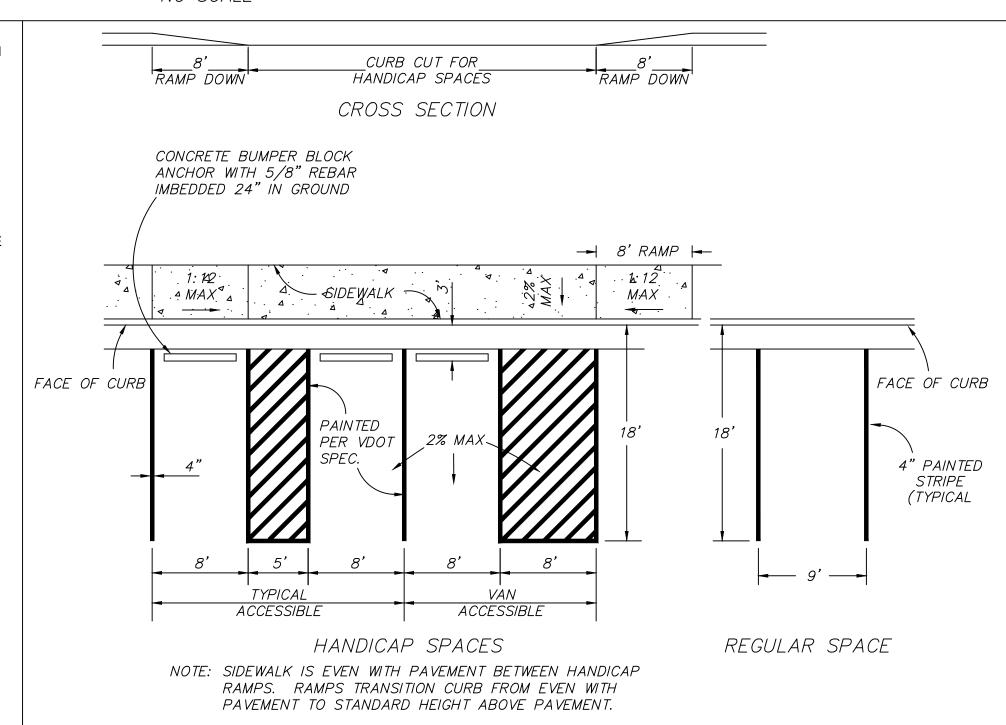




RESERVED

PARKING





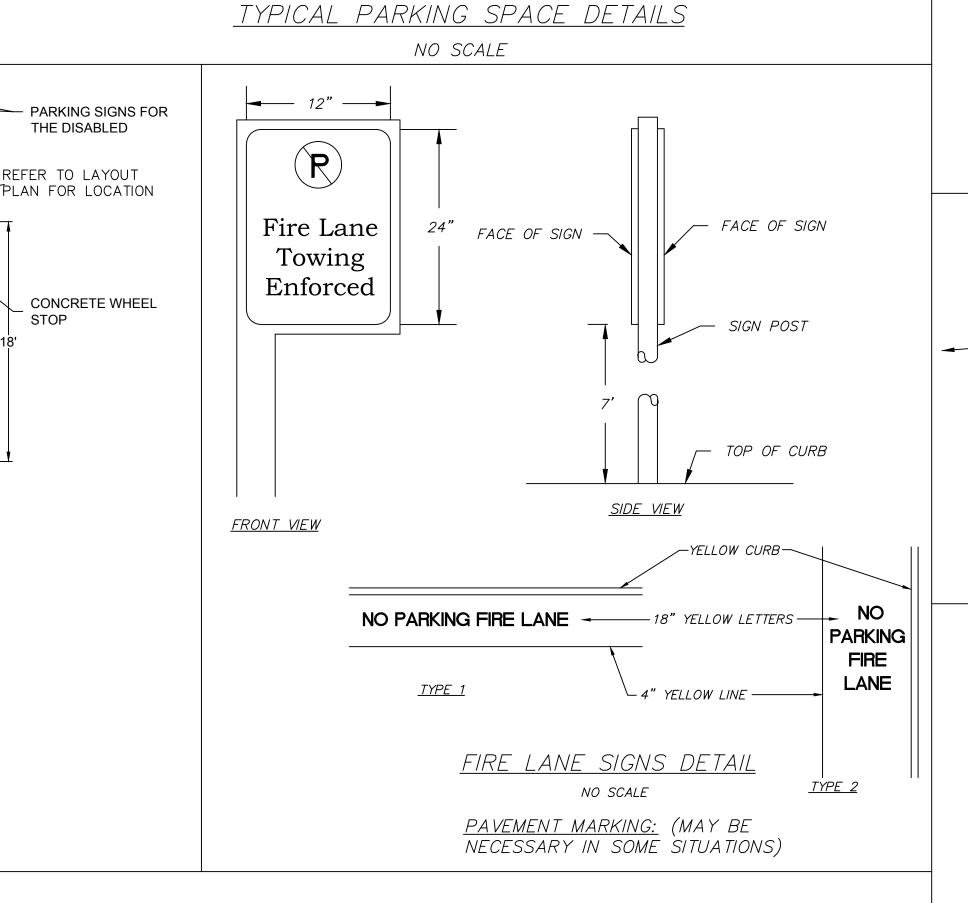
THE DISABLED

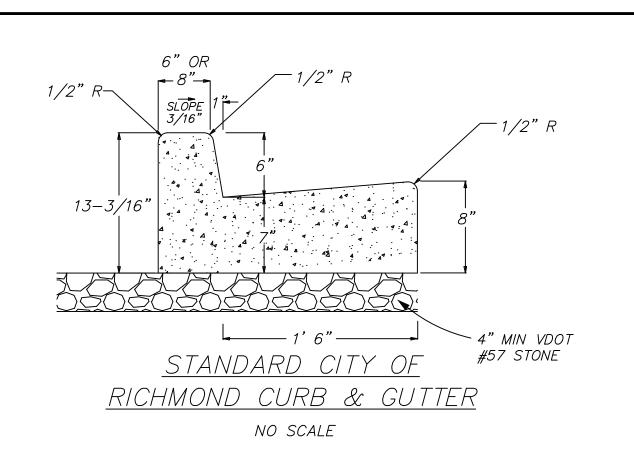
REFER TO LAYOUT

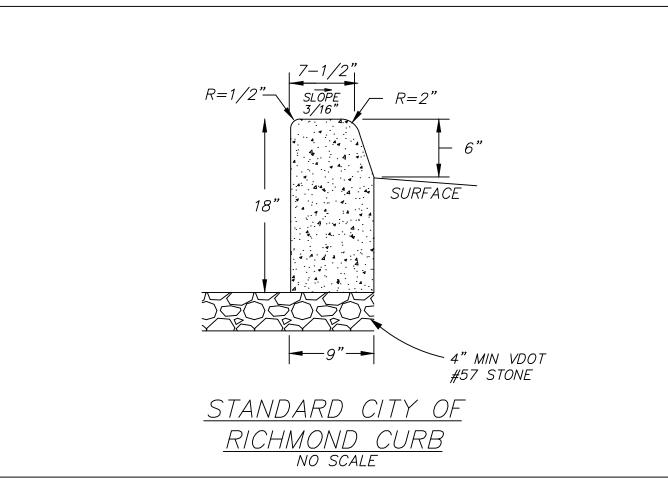
STOP

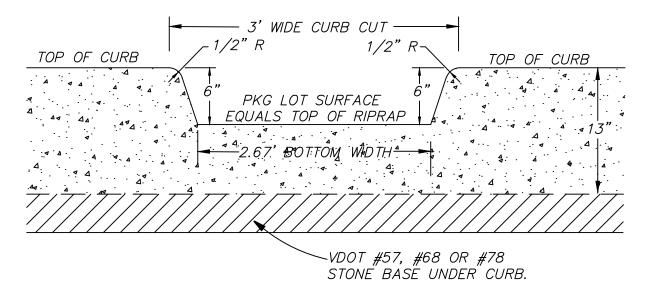
- 4" DIAGONAL YELLOW

STRIPE



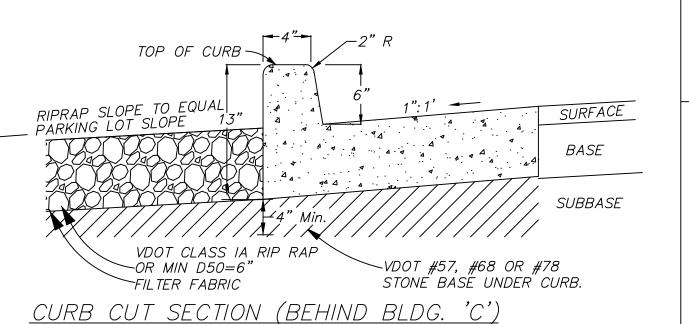


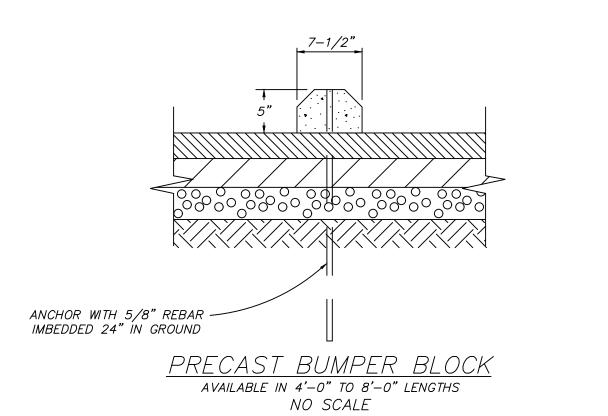




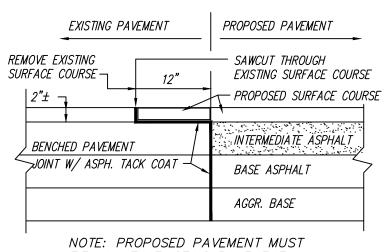
CURB CUT PROFILE (BEHIND BLDG. 'C'

NO SCALE

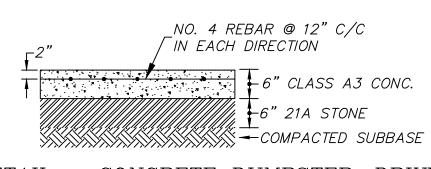




NO SCALE

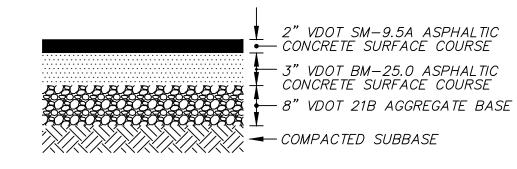


TIE IN WITH EXISTING PAVEMENT PAVEMENT JOINT DETAIL NOT TO SCALE



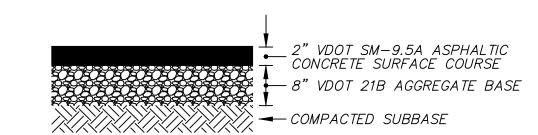
DETAIL ~ CONCRETE DUMPSTER, DRIVEWAY ELECT. TRANSFORMER & GENERATOR PADS

NOT TO SCALE



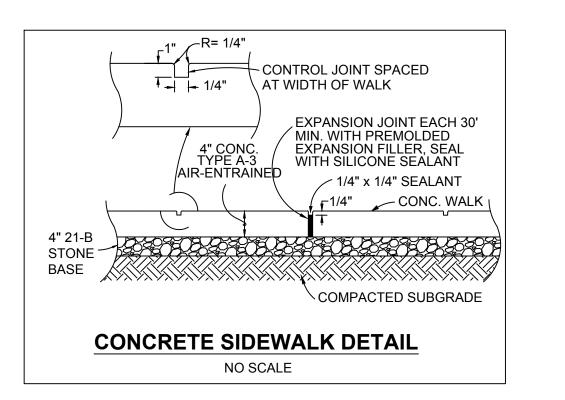
HEAVY DUTY PAVEMENT DETAIL NO SCALE

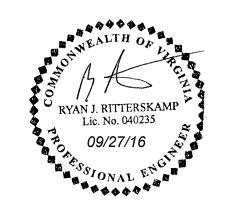
NOTE: THE PAVEMENT DETAIL, AS SHOWN IS THE RECOMMENDATION FROM A GEOTECHNICAL ENGINEERING STUDY OF STONY POINT SITE VI BY SCHNABEL ENGINEERING ASSOCIATES, INC. DATED JANUARY 28, 2016



STANDARD DUTY PAVEMENT DETAIL NO SCALE

NOTE: THE PAVEMENT DETAIL, AS SHOWN IS THE RECOMMENDATION FROM A GEOTECHNICAL ENGINEERING STUDY OF STONY POINT SITE VI BY SCHNABEL ENGINEERING ASSOCIATES, INC. DATED JANUARY 28, 2016





09/27/16 DRAWN BY B. SHAMLIN **DESIGNED BY** . RITTERSKAMI

CHECKED BY SCALE

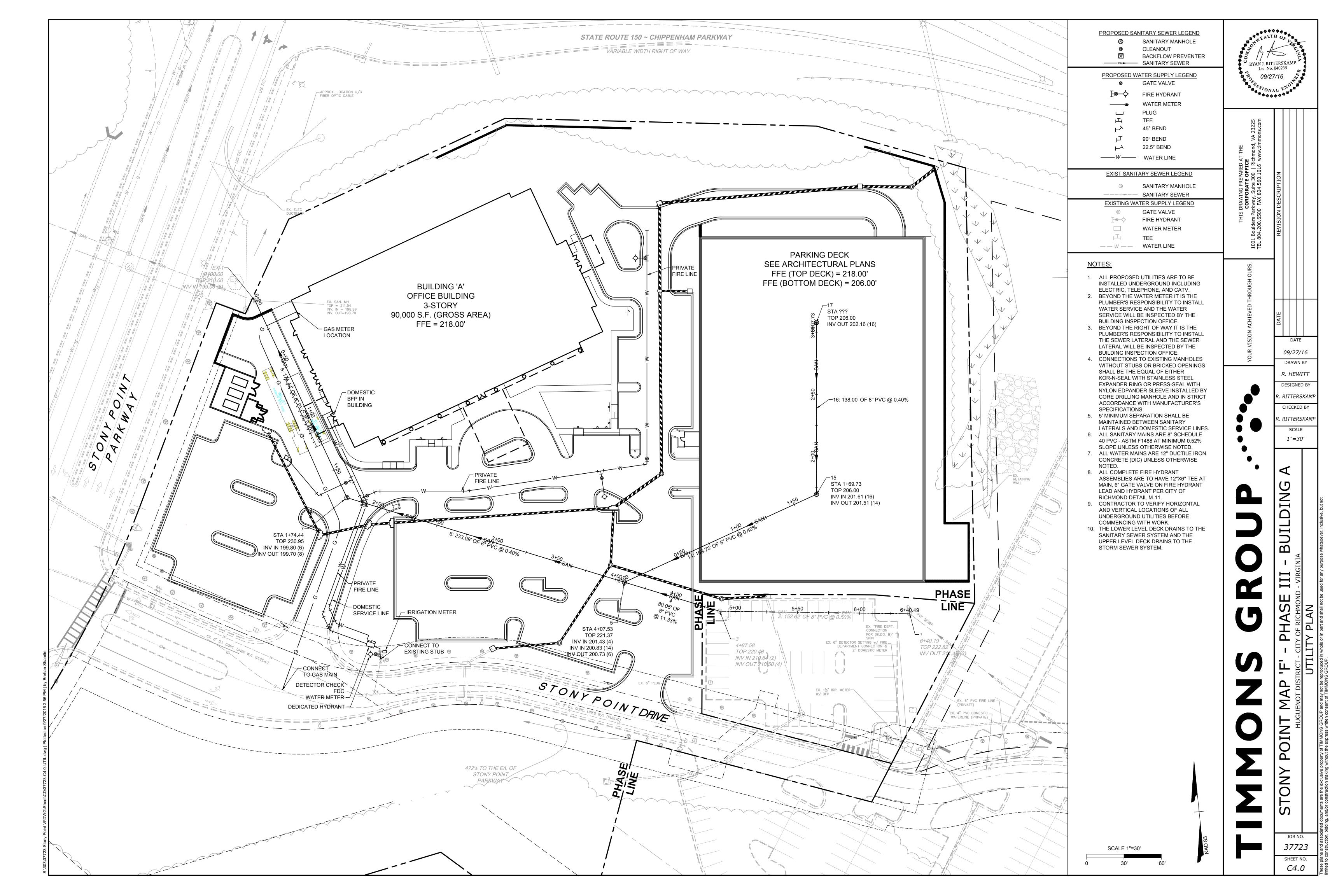
RITTERSKAMI

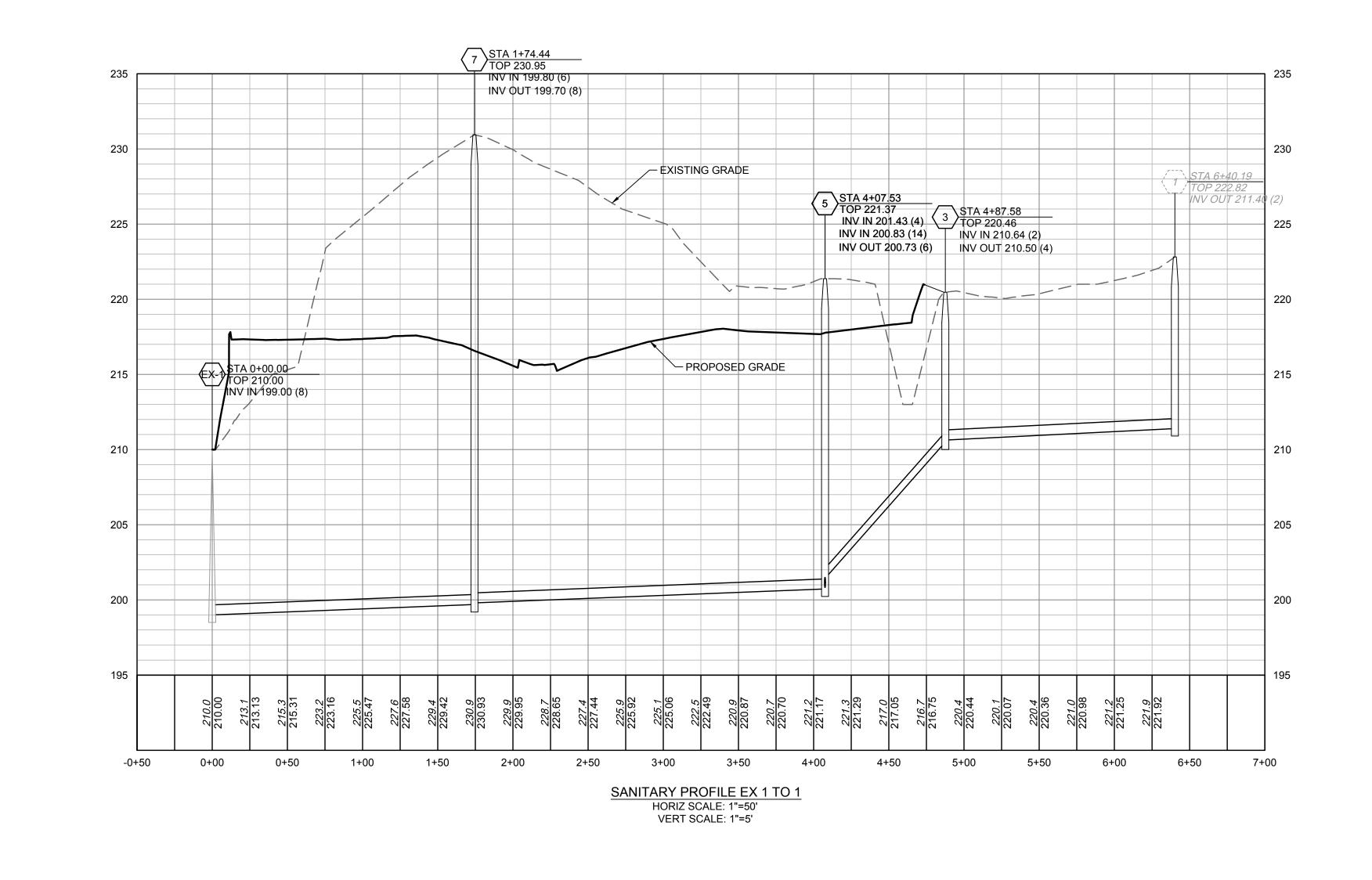
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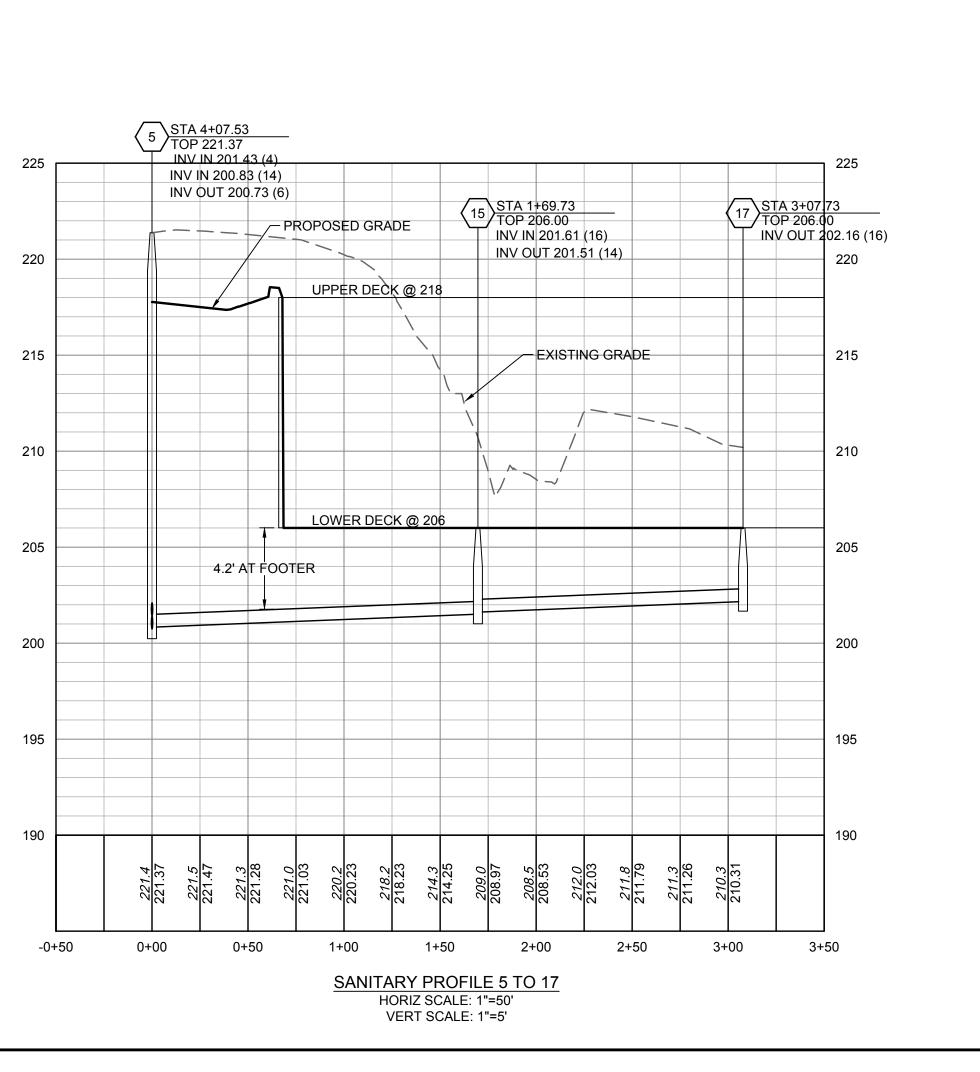
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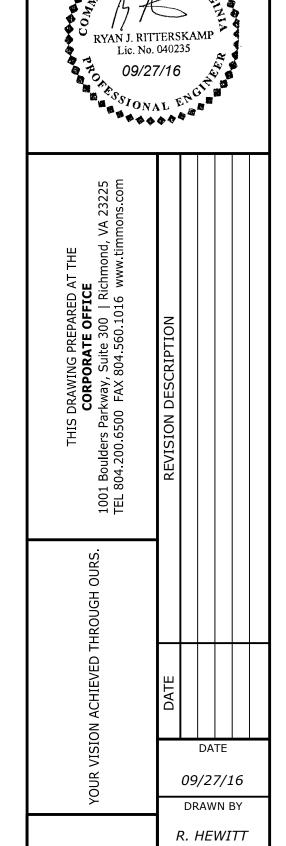
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JOB NO. 37723 SHEET NO. C3.1







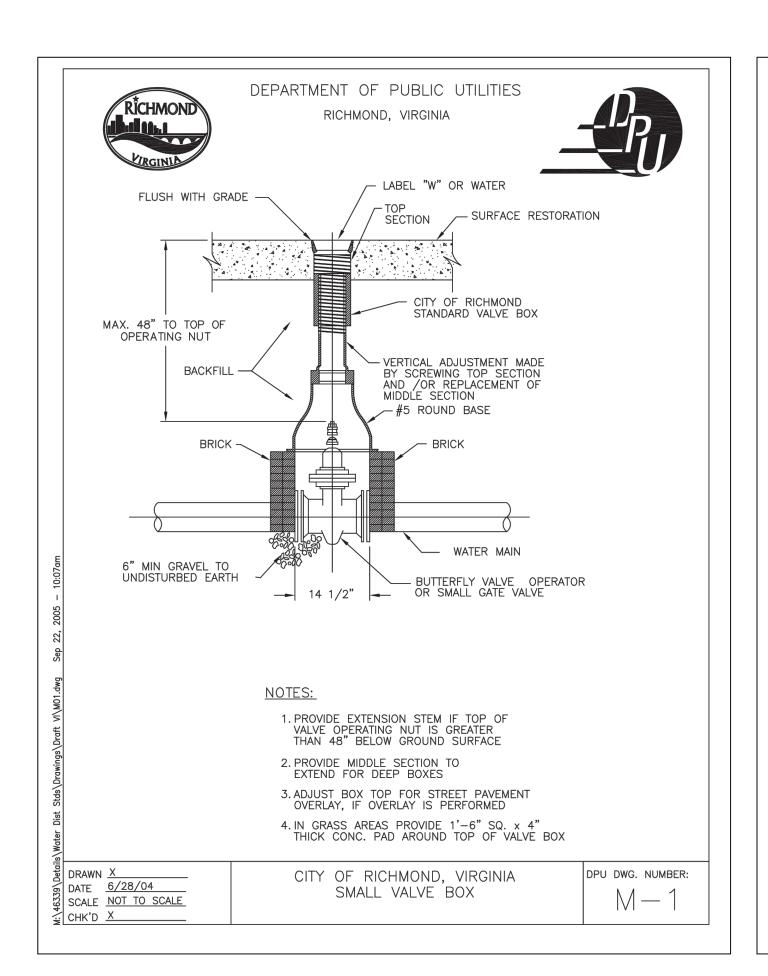


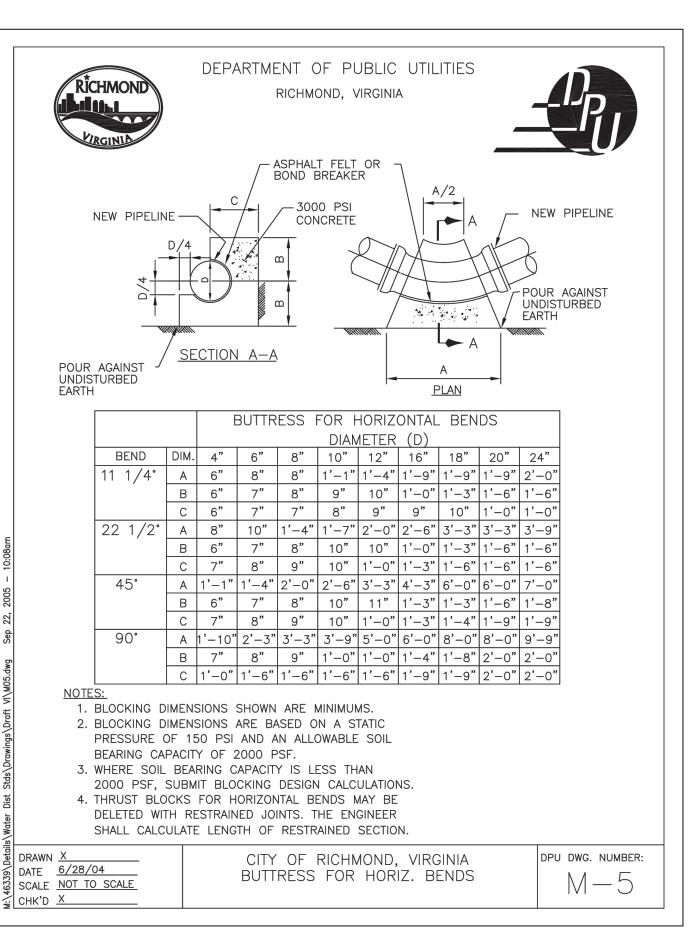
DESIGNED BY R. RITTERSKAMP

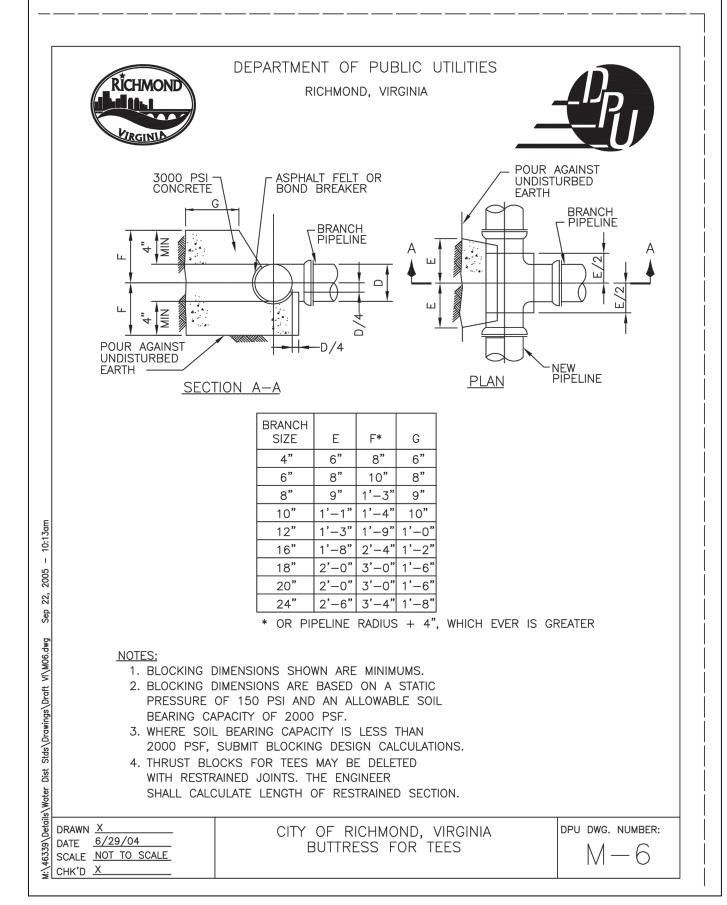
CHECKED BY . RITTERSKAMP SCALE H: 1"=50' V: 1'' = 5'

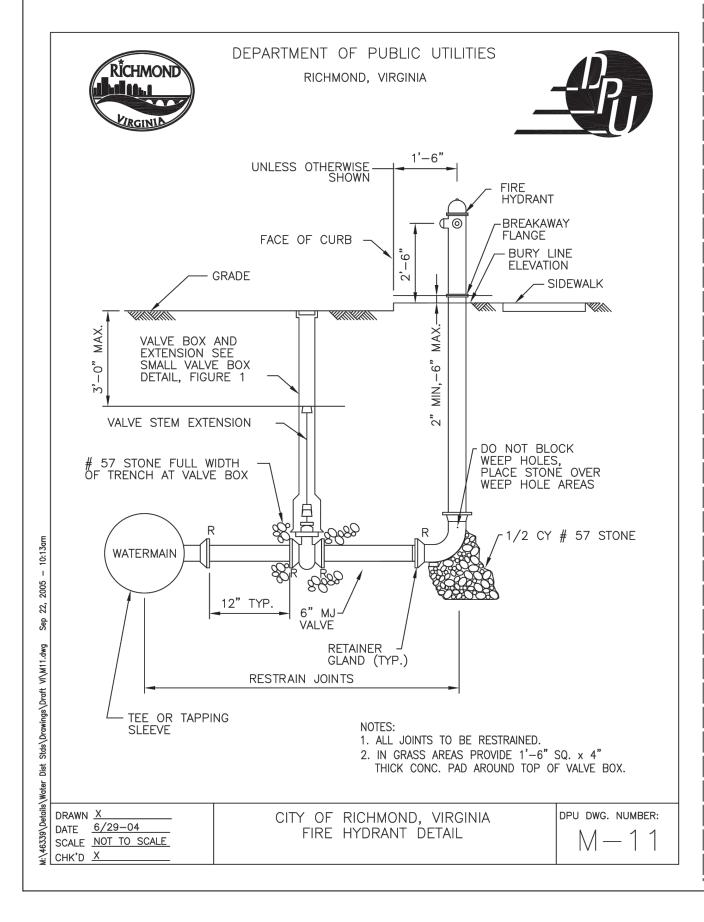
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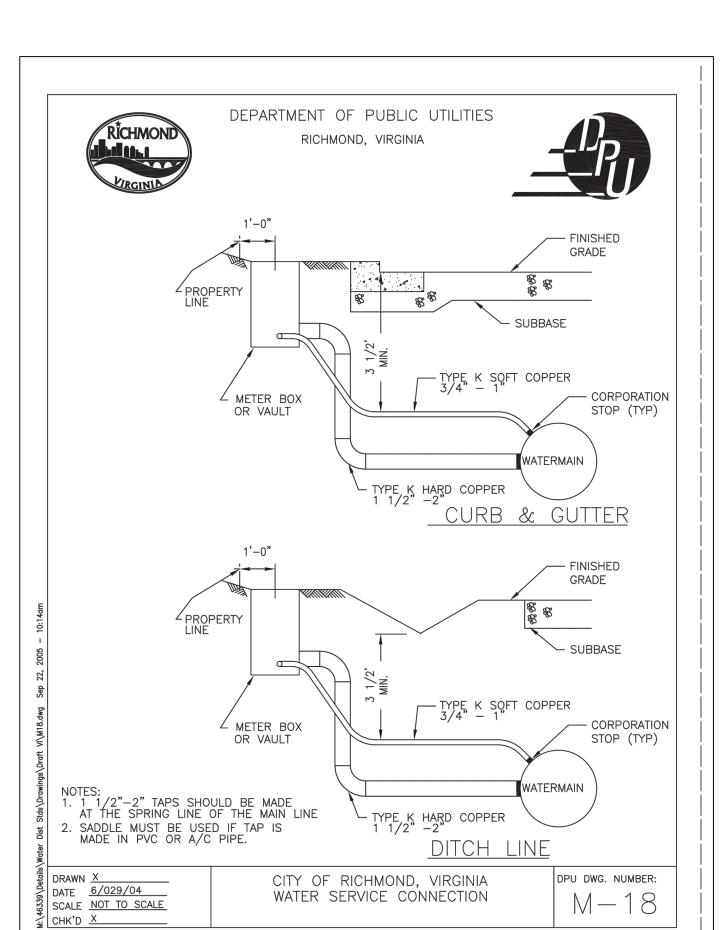
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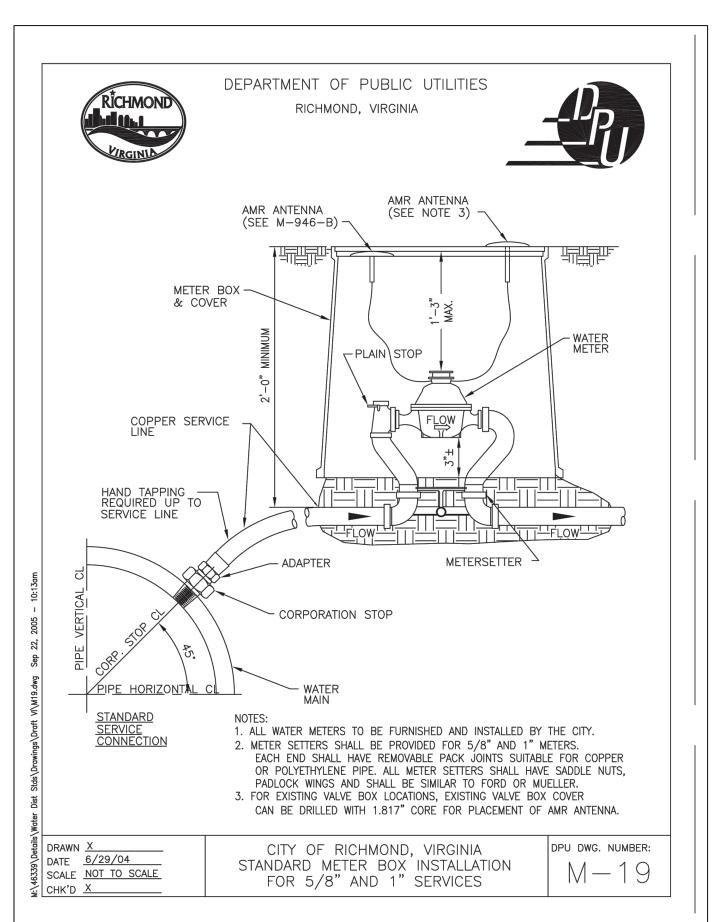


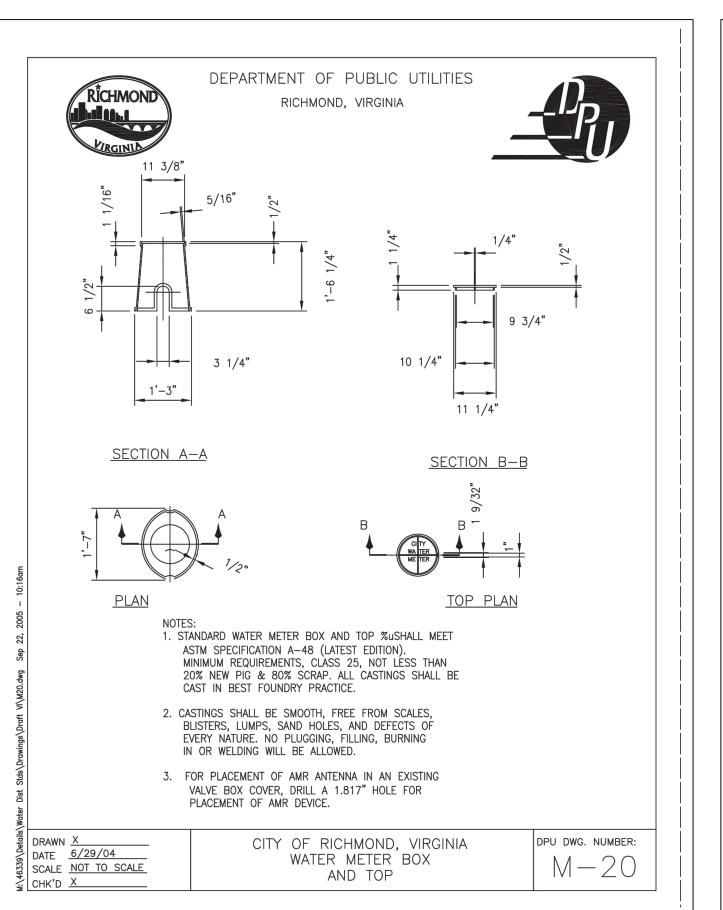


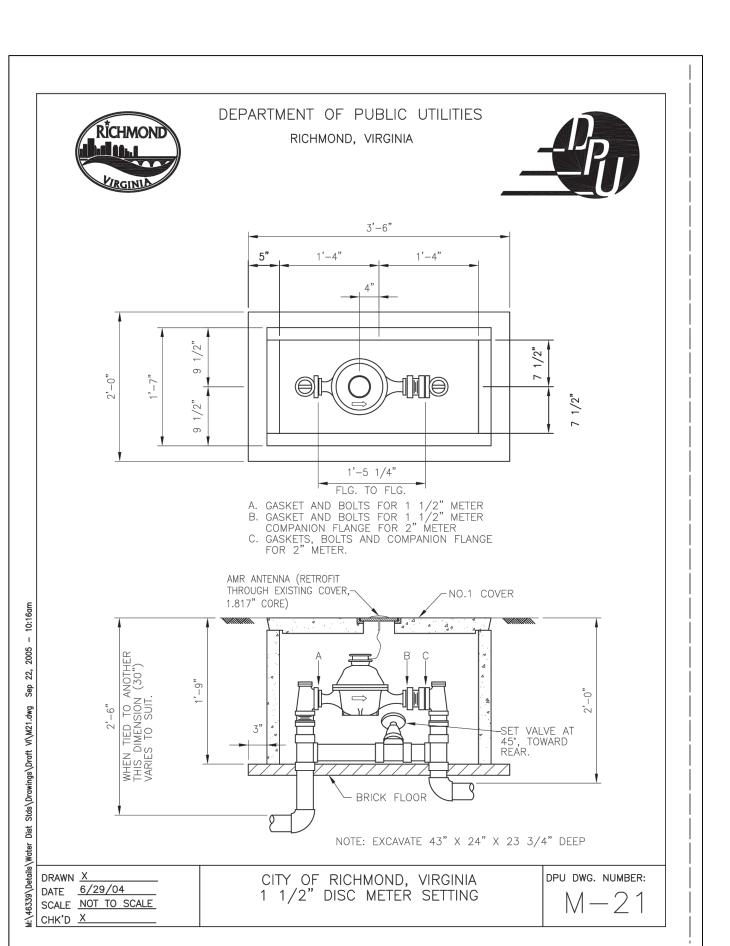


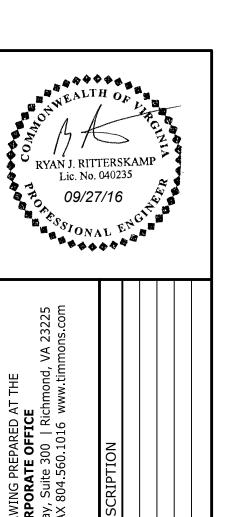












DATE

09/27/16

DRAWN BY

B. SHAMLIN

DESIGNED BY

. RITTERSKAMP

CHECKED BY

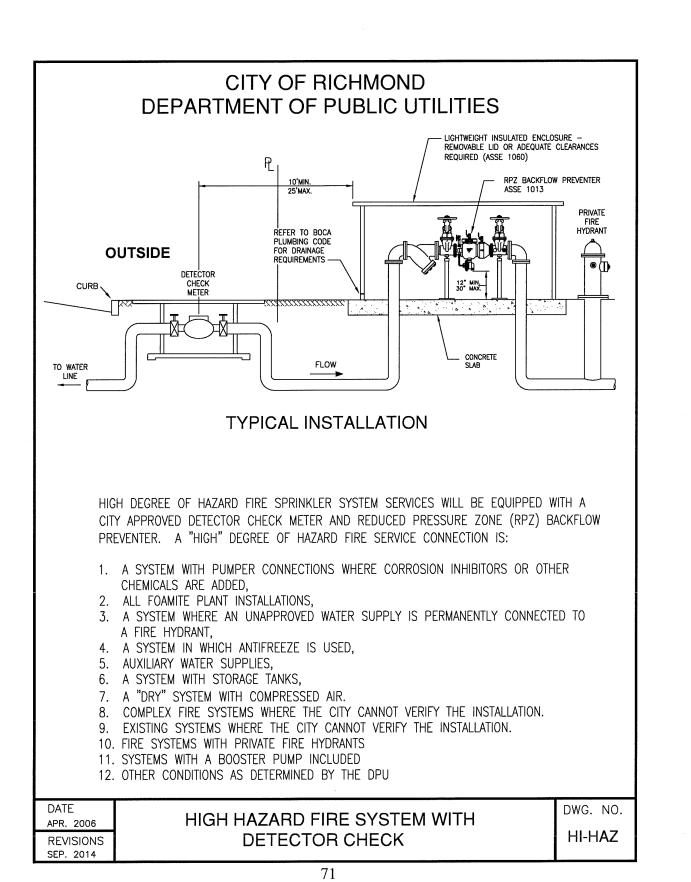
RITTERSKAMI

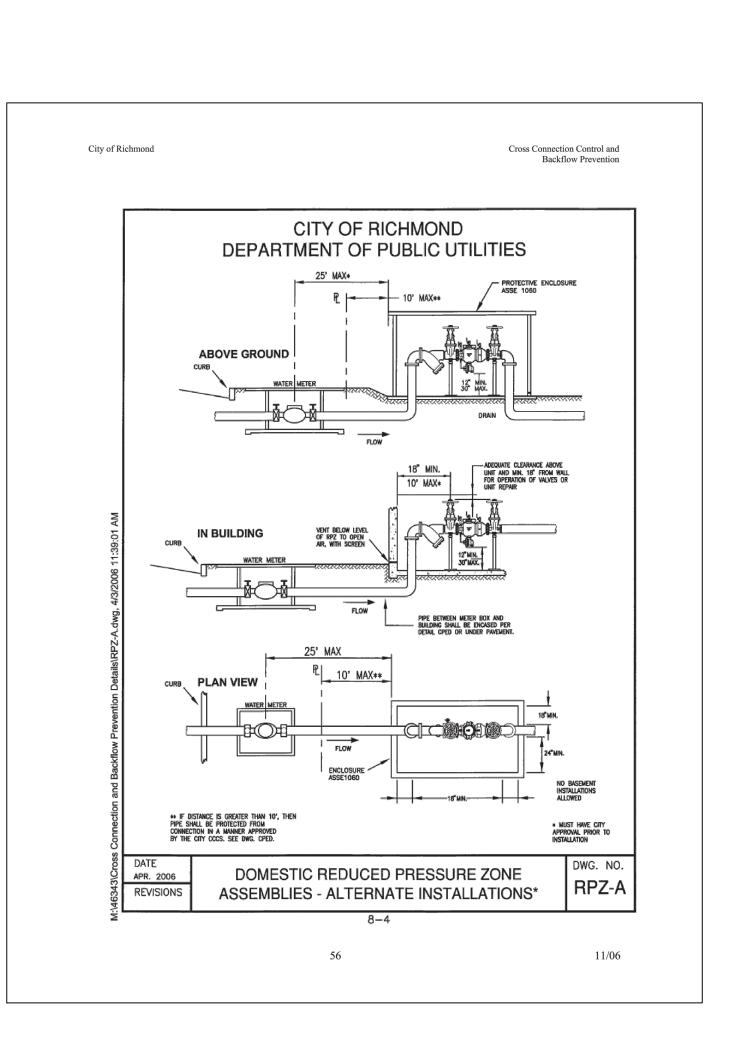
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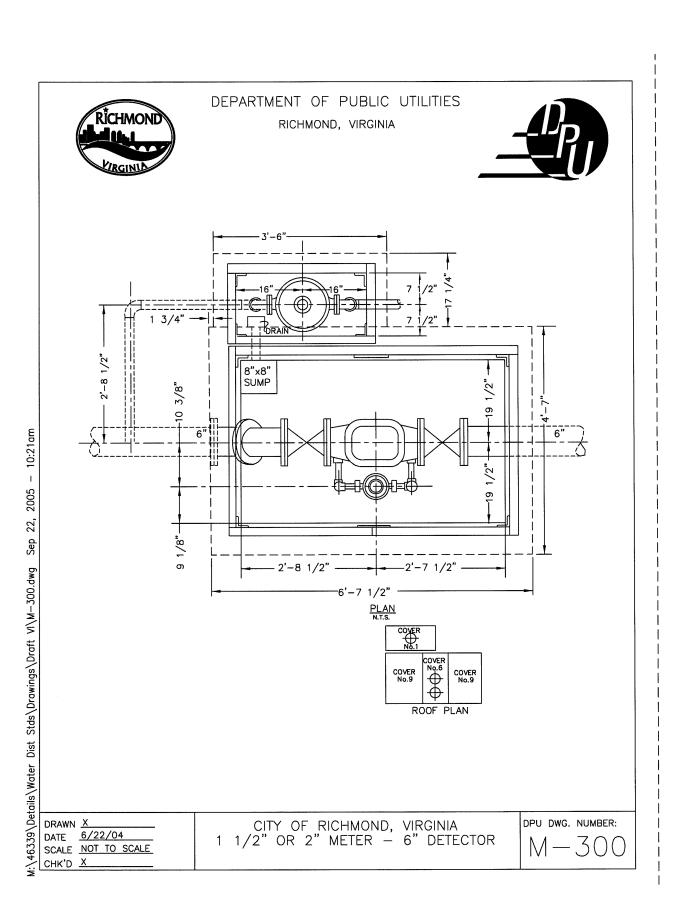
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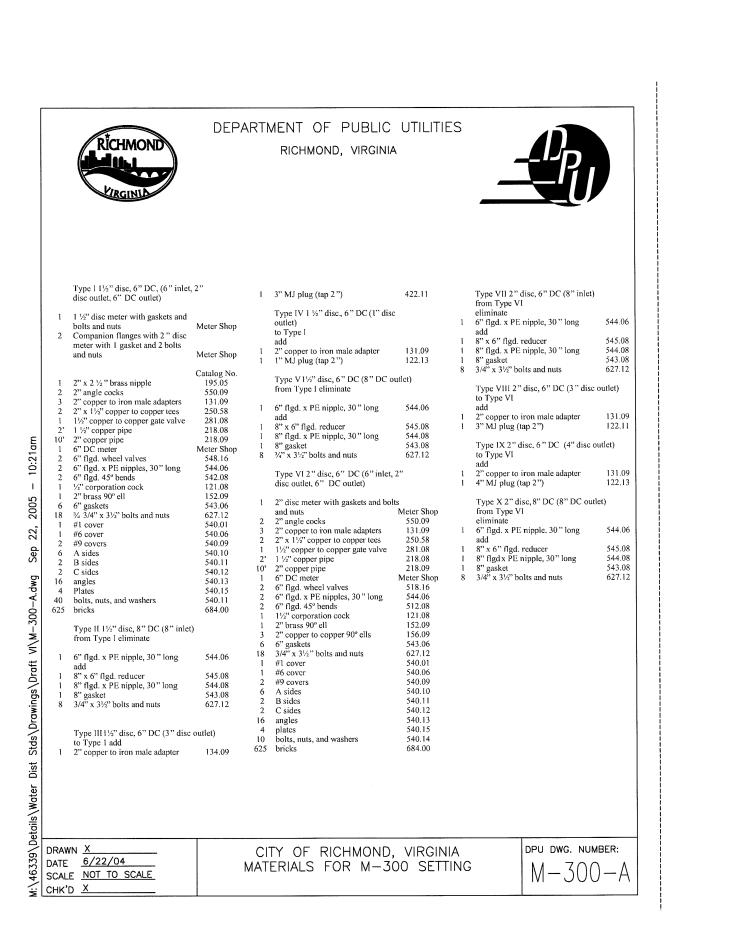
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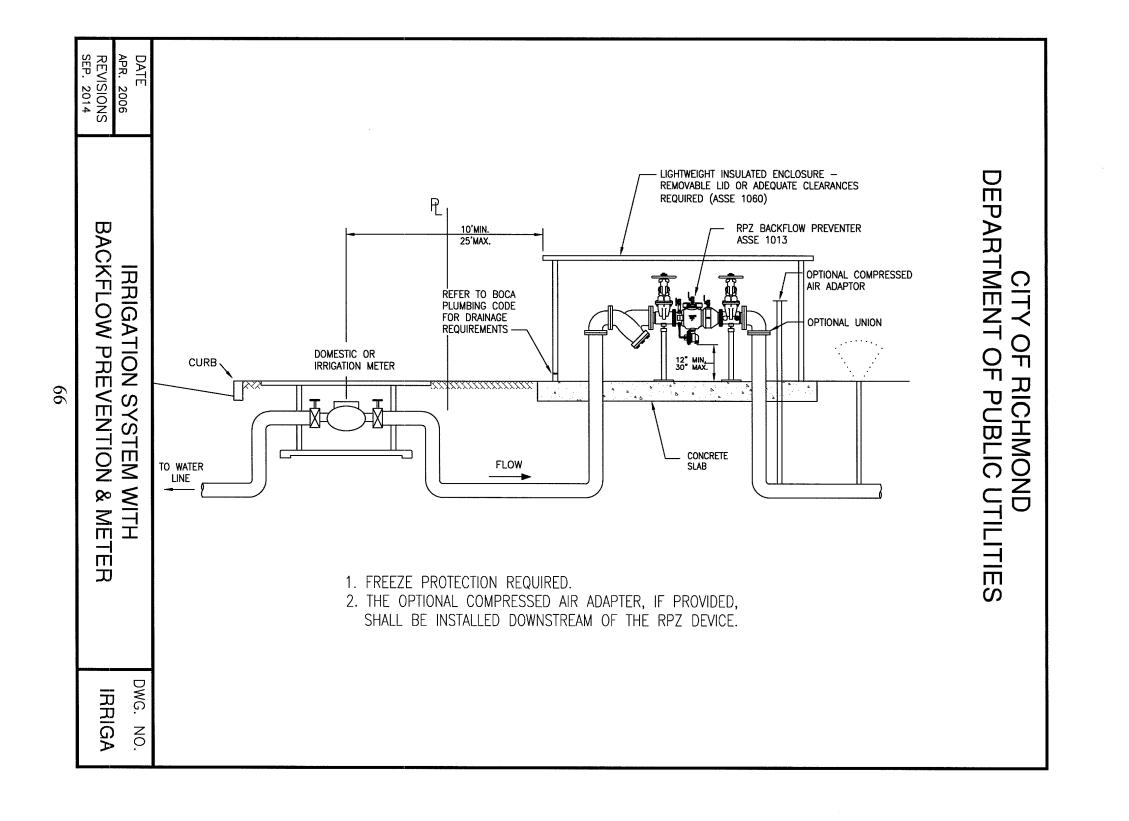
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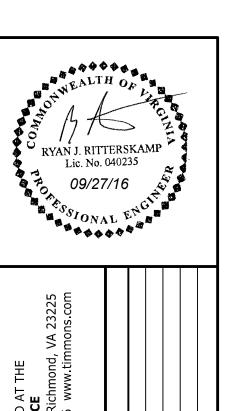












DATE

09/27/16 DRAWN BY

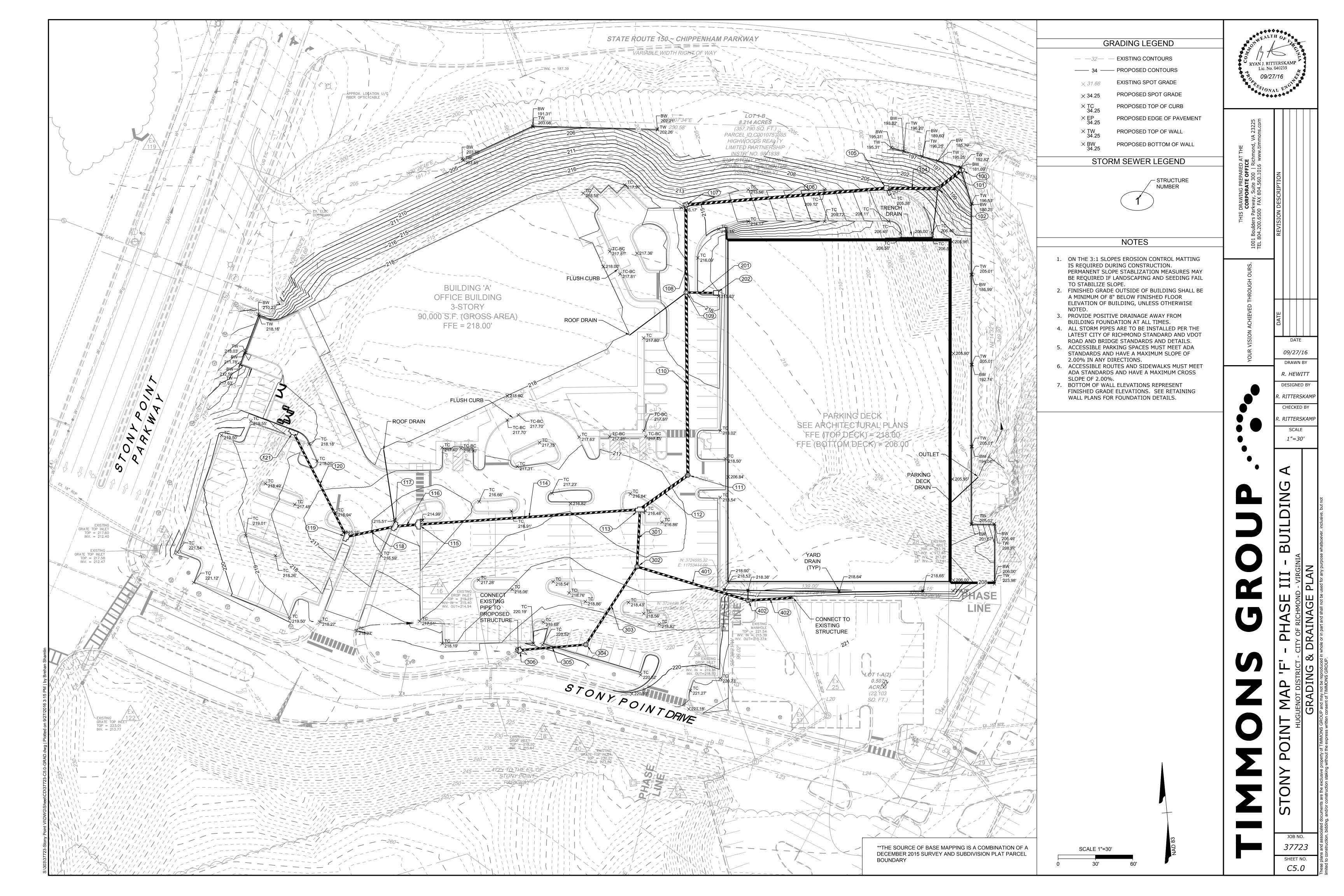
B. SHAMLIN

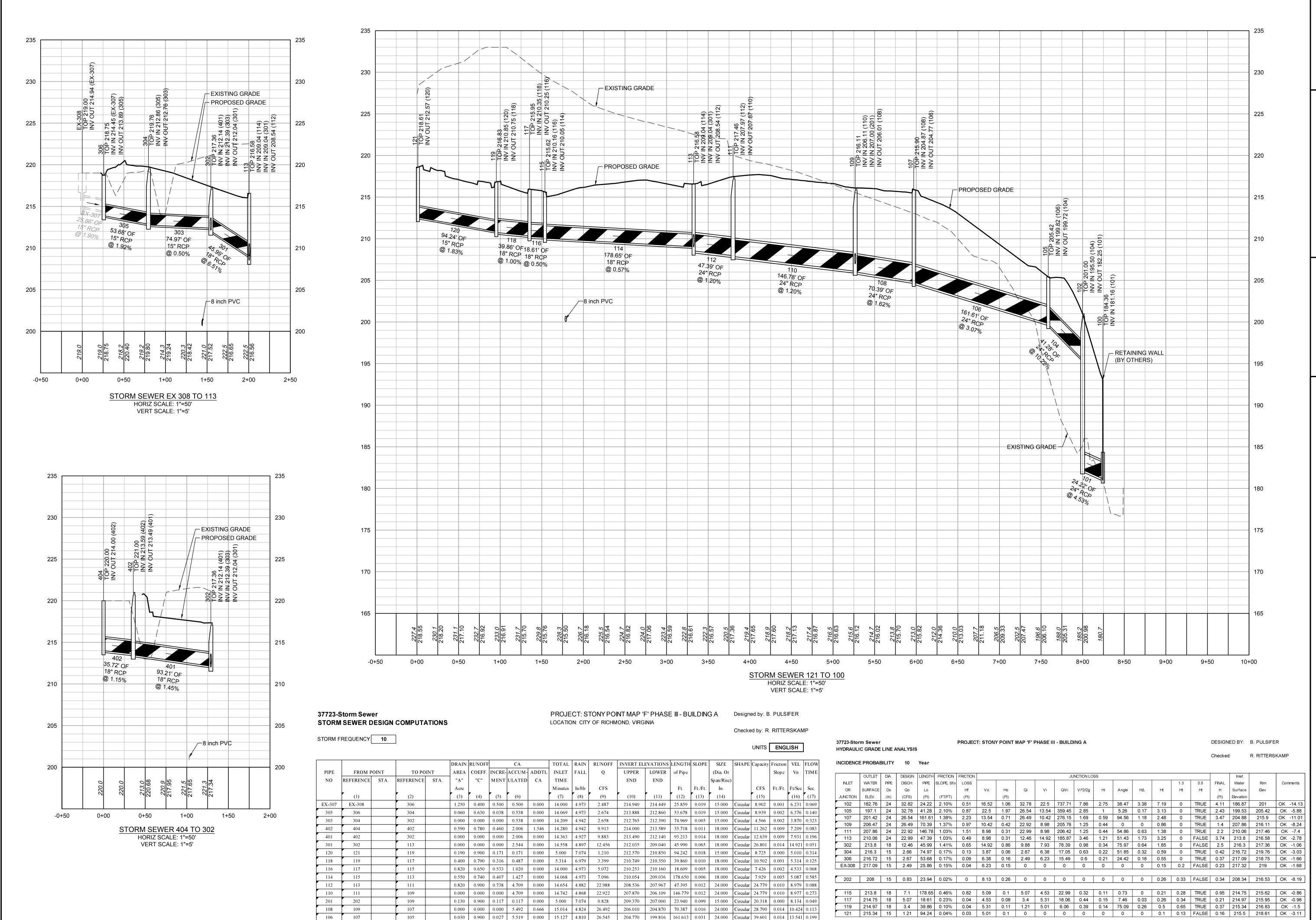
DESIGNED BY

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JOB NO. 37723 SHEET NO.

C4.3





0.310 | 0.800 | 0.248 | 6.856 | 1.089 | 15.326 | 4.782 | 32.784 | 199.720 | 195.500 | 41.282 | 0.102 | 24.000 | Circular | 72.317 | 0.021 | 22.502 | 0.031

0.020 | 0.350 | 0.007 | 6.863 | 0.000 | 15.356 | 4.782 | 32.817 | 182.253 | 181.156 | 24.217 | 0.045 | 24.000 | Circular | 48.142 | 0.021 | 16.523 | 0.024

105

102

104

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DATE

09/27/16

DRAWN BY

R. HEWITT

DESIGNED BY

. RITTERSKAMI

CHECKED BY

RITTERSKAM

SCALE

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POINT

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JOB NO.

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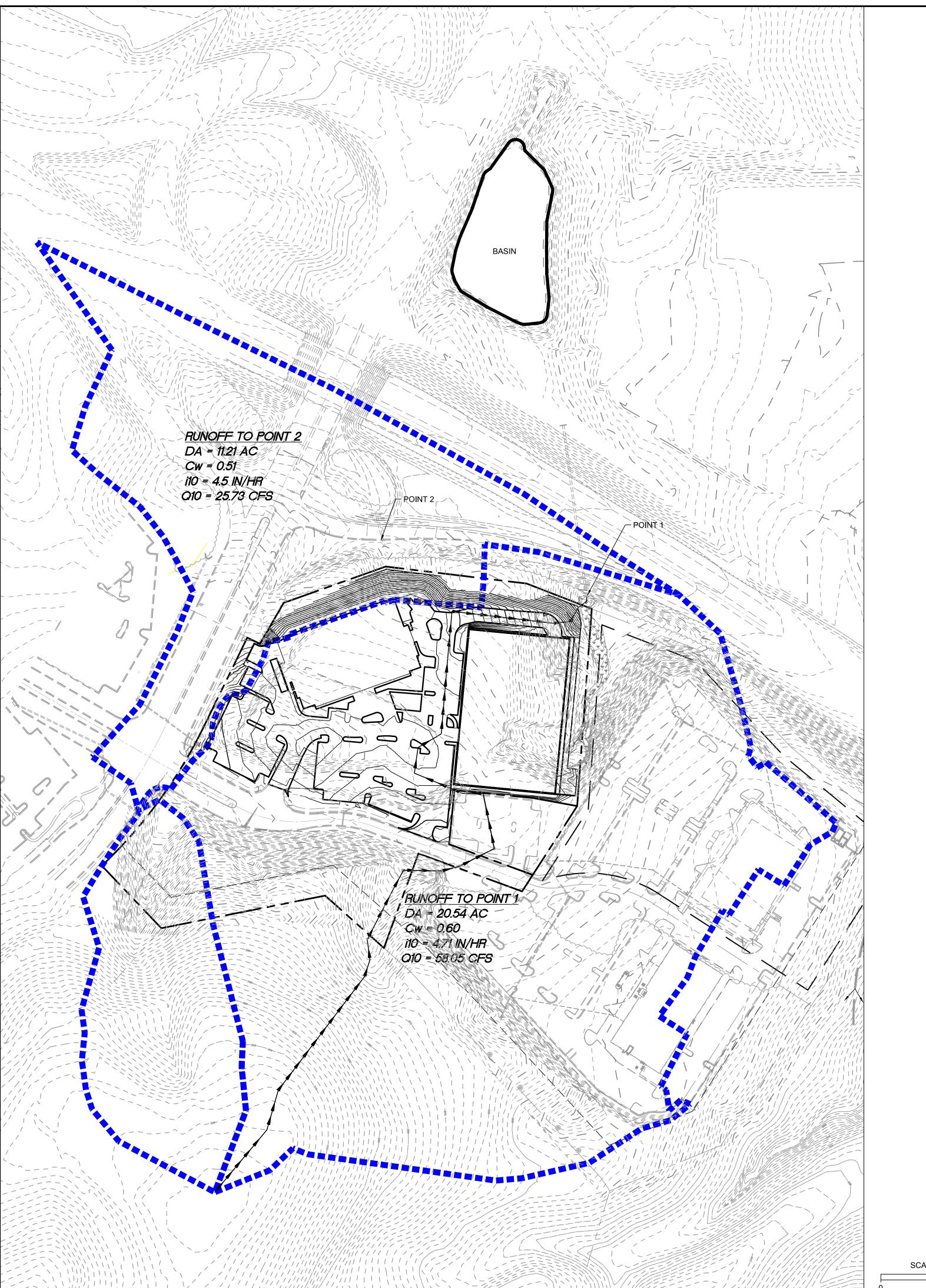
SHEET NO.

C5.1

402 216.3 18 9.88 93.21 0.89% 0.83 7.93 0.24 9.91 7.21 71.47 0.81 0.28 20.43 0.2 0.73 0 FALSE 1.55 217.85 221 OK -3.15

402 217.85 18 9.91 35.72 0.89% 0.32 7.21 0.2 0 0 0 0 0 0 0 0.2 0.26 FALSE 0.58 218.43 220 OK -1.57

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1001 Boulders Parkway Suite 300 Richmond, VA 23225

P 804.200.6500 F 804.560.1016 www.timmons.com

9/27/2016 JN 37723

EXISTING SWM/BMP RETENTION BASIN (4 times WQV with 50% Removal Efficency) - STONY POINT PARCEL E - STORMWATER QUALITY LEDGER

	IMPERVIOUS COVER WITHIN CBPA RMA **SEE NOTE 1**	IMPERVIOUS COVER WITHIN VSMP AREA *SEE NOTE 2**	POLLUTANT REMOVAL REQUIREMENT	IMPERVIOUS COVER DRAINING TO RETENTION BASIN	PERVIOUS COVER DRAINING TO RETENTION BASIN	TP REMOVAL CREDIT FROM IMPERVIOUS COVER	TP REMOVAL CREDIT FROM PERVIOUS COVER	
	(ACRES)	(ACRES)	(LBS PER YEAR)	(ACRES)	(ACRES)	(LBS PER YEAR)	(LBS PER YEAR)	
MAP PARCEL E								
MCV Oncology Expansion	0	N/A	0	1.00	1.96	1.08	0.11	
Richmond Medical Commons	2.54	N/A	4.3	4.64	0.97	5.03	0.06	
Stony Point III and IV	0.62	N/A	1.1	9.08	7.16	9.83	0.41	
MAP PARCEL F								
Phase I (Bldg B)	0	N/A	0	2.29	1.12	2.48	0.06	
Phase II (Bldg C)	N/A	3.86	5.9	3.18	1.19	3.44	0.07	
Phase III (Bldg A)	N/A	3.92	6.2	3.92	1.05	4.25	0.06	
		=	47.5			26.4	0.0	

TP Removal Removal Credit =

1) City of Richmond's CBPA Ordinance applies to projects constructed prior to January 2005. 2) VA DEQ's VSMP Part IIC Technical Criteria applies to projects constructed after January 2005. 3) In order to utilize the entire potential TP removal credit of 26.88 lbs. per year, the actual water quality volume within the existing retention basin was field verified and increased as necessary. (See Stony Point Parcel 'F' - Phase II plans for reference)

PERFORMANCE-BASED WATER QUALITY CALCULATIONS APPENDIX 5D	PERFORMANCE-BASED WATER QUALITY CALCULATIONS APPENDIX 5D
Worksheet 2 : Situation 2 Page 1 of 4	Worksheet 2: Situation 2 Page 2 of 4
Summary of Situation 2 criteria: from calculation procedure STEP 1 thru STEP 3 , Worksheet 1:	$\underline{STEP 5}$ Determine the relative post-development pollutant load (L _{post}).
Applicable area (A)* = $_5.6$ acres $I_{post} = (total \ post-development \ impervious \ cover \div A) \times 100 = \{70} \%$ $I_{watershed} = \{\%} or [I_{watershed} = 16\%]$ $I_{existing} = (total \ existing \ impervious \ cover \div A*) \times 100 = \{70} \%$	$\begin{split} \mathbf{L_{post}} &= [0.05 + (0.009 \times I_{post})] \times A \times 2.28 \text{(Equation 5-21)} \\ \text{where:} \qquad & L_{post} &= \text{ relative post-development total phosphorous load (pounds per year)} \\ & I_{post} &= \text{ post-development percent impervious cover (percent expressed in whole numbers)} \\ & A &= \text{applicable area (acres)} \end{split}$
I _{existing} 0 % # I _{watershed} 16 %; and	$\mathbf{L}_{\text{post}} = [0.05 + (0.009 \times \underline{70})] \times \underline{5.6} \times 2.28$
I_{post} 70 % $> I_{watershed}$ 16 %	=8.7 pounds per year
$\underline{\text{STEP 4}}$ Determine the relative pre-development pollutant load (L_{pre}).	STEP 6 Determine the relative pollutant removal requirement (RR).
$\begin{aligned} \mathbf{L}_{\text{pre(watershed)}} &= [0.05 + (0.009 \times I_{\text{watershed}})] \times \mathbf{A} \times 2.28 & \text{(Equation 5-16)} \\ \text{where: } \mathbf{L}_{\text{pre(watershed)}} &= \text{relative pre-development total phosphorous load (pounds per year)} \\ I_{\text{watershed}} &= \text{average land cover condition for specific watershed or locality } \underbrace{\mathbf{or}}_{\text{the Chesapeake Bay default value of 16% (percent expressed in whole numbers)}} \end{aligned}$	$\mathbf{RR} = \mathbf{L}_{post} \; ! \; \mathbf{L}_{pre(watershed)}$ $\mathbf{RR} = \underline{8.7} \; ! \; \underline{2.5}$ $= \underline{6.2} pounds \; per \; year$ STEP 7 Identify best management practice (BMP) for the site.
A = applicable area (acres)	
$\mathbf{L}_{\text{pre(watershed)}} = [0.05 + (0.009 \times \underline{16})] \times \underline{5.6} \times 2.28$ $= \underline{2.5} \text{pounds per year}$	1. Determine the required pollutant removal efficiency for the site:
	$\mathbf{EFF} = (\underline{} \div \underline{}) \times 100$

Stony Point Parcel F Phase III Building A VSMP Stormwater Quality Compliance

September 27, 2016

Stony Point Parcel F is part of the overall Stony Point Community Unit Plan and the Parcel F Master Plan was originally approved around 2002. Parcel F Phase I was constructed soon thereafter and was developed under the City's CBPA Regulations that only required water quality treatment in areas designated as RMAs. Parcel F Phase III (building A) meets the Grandfathering criteria outlined in 9VAC25-870-48 and is subject to Part IIC of the VSMP technical criteria.

Under the Part II C water quality criteria, Phase II is required to limit the post-developed pollutant discharge to a level that does not exceed the pre-developed pollutant discharge based on an average land cover condition of 16% imperviousness. Phase II will achieve the Part II C pollutant removal requirements by utilizing the excess water quality volume contained in the existing stormwater retention pond located on Parcel E (Stony Point III). The vast majority of Parcel F Phase III drains directly into the Stony Point III retention pond via an existing culvert under Chippenham Parkway and the calculations show that there is enough water quality volume for this existing pond to function as a Retention Basin II (4 times WQV with 50% removal efficiency) per section 3.06 of the VA Stormwater Management Handbook.

Our calculations indicate that the existing Stony Point III SMW retention basin should be removing at least 26.88 lbs. of pollutant per year. Applicable water quality regulations require Parcel F Phases II and III and a portion of Parcel E to remove 17.5 lbs. per year so no additional on-site water quality treatment is required on Parcel F Phase III.

Summary of Calculations

- 1. Pond volumes from Existing BMP/SWM Retention Pond Plan by Timmons Group dated 2-11-2016 indicate the existing Stony Point III SWM retention basin contains 6 feet of water below the normal pool elevation and the permanent pool volume is approximately 146,270 cubic feet. In accordance with Appendix 5D worksheets of the VA Stormwater management Handbook, this permanent pool results in 36,567 cu. ft. of WQV that will treat the first 0.5 inch of stormwater runoff on 20.14 acres of impervious cover. Assuming a 50% pollutant removal efficiency, the existing retention basin is removing 22.57 lbs. of pollutant per year.
- 2. There are 3.16 acres of impervious cover within the RMA on Parcel E that currently drains into the existing Stony Point III SWM retention basin. The pollutant removal requirement (RR) for this 3.16 acres is to 5.4 lbs. per year. 3. There are 3.86 acres of impervious cover proposed on Parcel F Phase II (building C) with 3.28
- acres draining directly into the Stony Point III basin and 0.58 acres bypassing it. The RR for this 3.86 acres is 5.9 lbs. per year.
- 4. There are 3.92 acres of impervious cover proposed on Parcel F Phase III (building A) draining directly into the Stony Point III basin t. The RR for this 3.92 acres is 6.2 lbs. per year. 5. We estimate 20.0 acres of impervious cover currently drains into the existing Stony Point III SWM retention basin. If applicable, the RR for this 20.0 acres would be 26.5 lbs. per year which

would require a permanent pool (4xWQV) of 145,200 cu. ft.

09/27/16 DRAWN BY

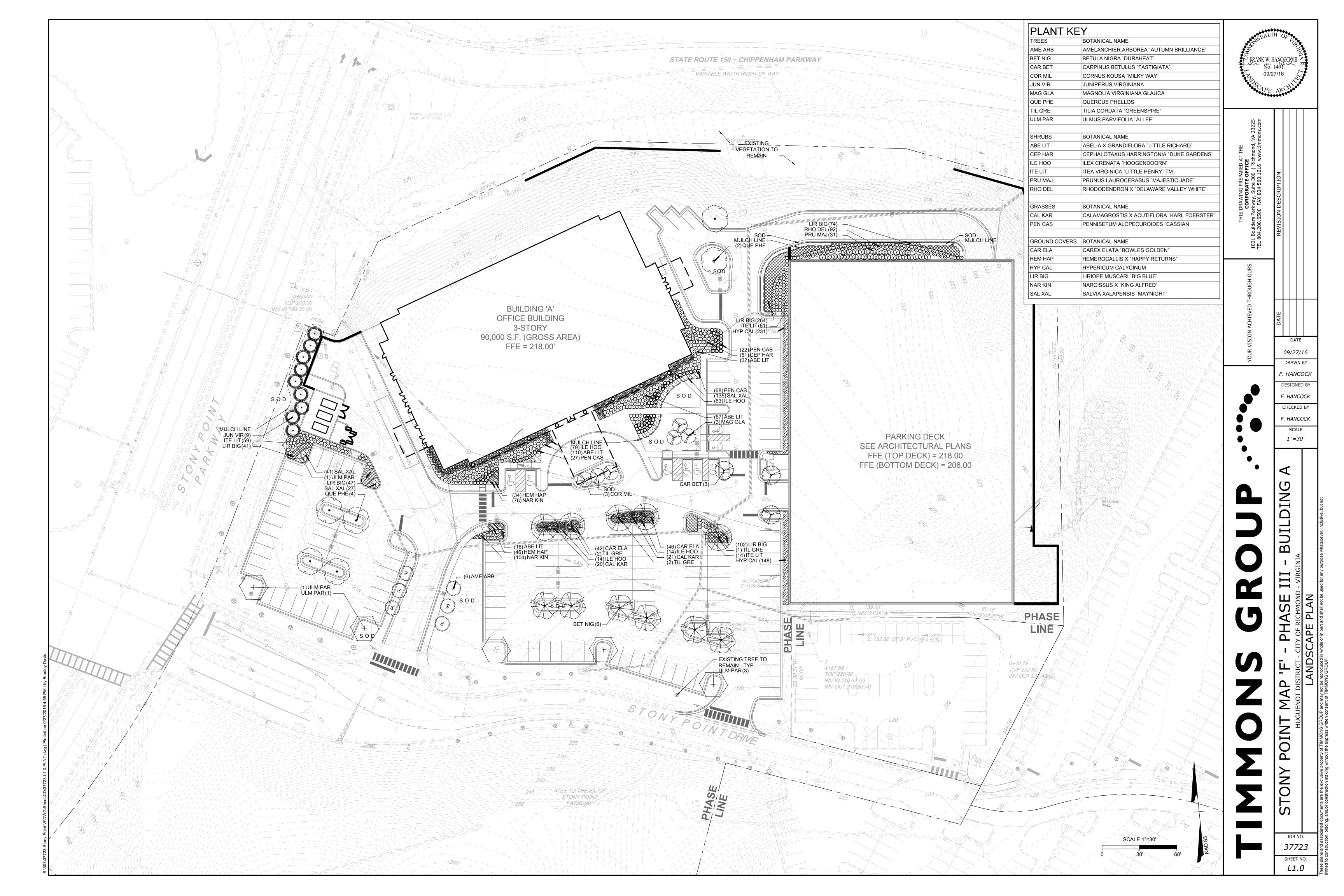
R. HEWITT DESIGNED BY R.RITTERSKAMP CHECKED BY

AS SHOWN

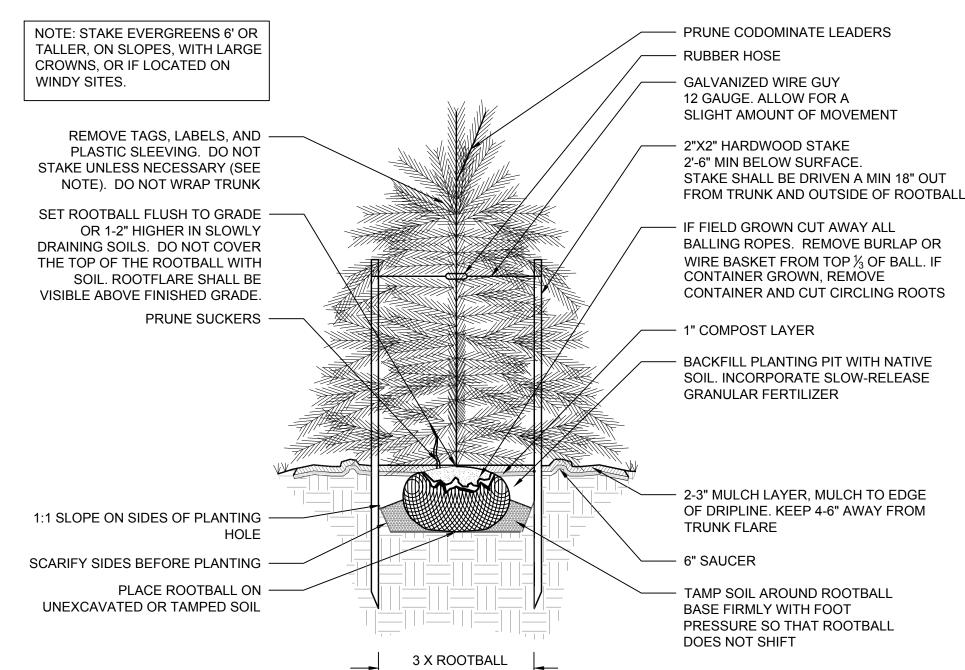
. RITTERSKAMP

37723 SHEET NO.

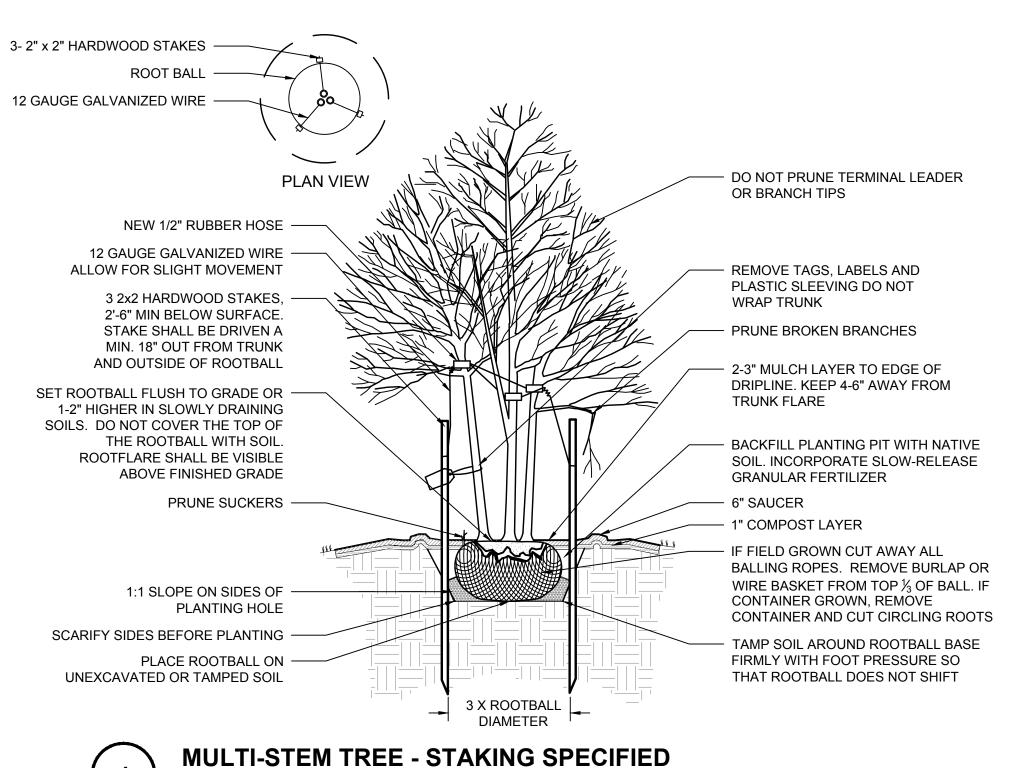
SCALE 1"=100'

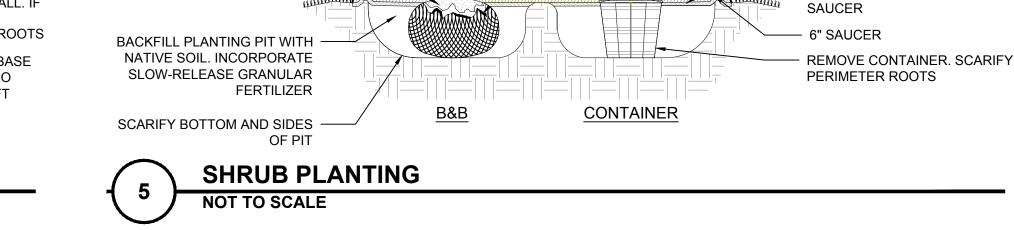


PLANT SCHEDULE QTY BOTANICAL NAME COMMON NAME | MINIMUM INSTALLED SIZE | ROOT REMARKS AME ARB AMELANCHIER ARBOREA 'AUTUMN BRILLIANCE' DOWNY SERVICEBERRY 3 STEMS ONLY, LIMBED UP TO 4 2.5" CAL BET NIG 6 BETULA NIGRA 'DURAHEAT' RIVER BIRCH 2.5" CAL 3-5 STEM ONLY) FRANK W. HANCOCK) No. 1491 CAR BET CARPINUS BETULUS 'FASTIGIATA' PYRAMIDAL EUROPEAN HORNBEAN | 2.5" CAL CORNUS KOUSA 'MILKY WAY' MILKY WAY KOUSA DOGWOOD 09/27/16 COR MIL 2.5" CAL JUN VIR JUNIPERUS VIRGINIANA EASTERN RED CEDAR 7`-8` HT. 7 - 8` HT MAG GLA MAGNOLIA VIRGINIANA GLAUCA SWEET BAY QUE PHE QUERCUS PHELLOS WILLOW OAK 2.5" CAL 5 TILIA CORDATA `GREENSPIRE TIL GRE GREENSPIRE LITTLELEAF LINDEN 2.5" CAL. R&R |6 | ULMUS PARVIFOLIA 'ALLEE' ULM PAR ALLEE LACEBARK ELM 2.5" CAL. COMMON NAME SHRUBS QTY | BOTANICAL NAME MIN. INSTALLED SIZE ROOT REMARKS 230 ABELIA X GRANDIFLORA LITTLE RICHARD ABE LIT LITTLE RICHARD ABELIA 24" HT/SPRD CONTAINER 36" O.C. CEP HAR 51 CEPHALOTAXUS HARRINGTONIA 'DUKE GARDENS' DUKE GARDENS PLUM YEW 24" HT/SPRD CONTAINER 36" O.C. **GENERAL NOTES** CONTAINER 36" O.C. ILE HOO | 170 | ILEX CRENATA `HOOGENDOORN` HOOGENDOORN JAPANESE HOLLY | 18" HT ITE LIT |134 | ITEA VIRGINICA `LITTLE HENRY` TM VIRGINIA SWEETSPIRE 24" HT/SPRD CONTAINER | 36" O.C. PRE-CONSTRUCTION EDGE OF BUILDING, PRU MAJ 31 PRUNUS LAUROCERASUS 'MAJESTIC JADE CONTAINER 5 O.C. MAJESTIC JADE LAUREL 36" HT WALK OR STRUCTURE CONTRACTOR IS RESPONSIBLE FOR CONTACTING "MISS UTILITY" AT RHO DEL 92 RHODODENDRON X DELAWARE VALLEY WHITE AZALEA 24" HT/SPRD CONTAINER 36" O.C. VOIDS 1.800.552.7001 FOR LOCATION OF ALL UTILITY LINES.TREES SHALL BE LOCATED A MINIMUM OF 5 FEET FROM SEWER/WATER CONNECTIONS. - MULCH LINE NOTIFY LANDSCAPE ARCHITECT OF CONFLICTS. GRASSES QTY | BOTANICAL NAME COMMON NAME MIN. INSTALLED SIZE ROOT REMARKS PLANT UNDER DRIP LINE OF VERIFY ALL PLANT MATERIAL QUANTITIES ON THE PLAN PRIOR TO LIMBED UP SHRUBS & TREES CAL KAR |41 | CALAMAGROSTIS X ACUTIFLORA `KARL FOERSTER` | FEATHER REED GRASS CONTAINER | 18" OC. BIDDING, PLANT LIST TOTALS ARE FOR CONVENIENCE ONLY AND SHALL BE VERIFIED PRIOR TO BIDDING. PEN CAS 106 | PENNISETUM ALOPECUROIDES `CASSIAN FOUNTAIN GRASS 24" HT/SPRD CONTAINER 36" O.C. PROVIDE PLANT MATERIALS OF QUANTITY, SIZE, GENUS, SPECIES, AND VARIETY INDICATED ON PLANS. ALL PLANT MATERIALS AND INSTALLATION SHALL COMPLY WITH RECOMMENDATIONS AND GROUND COVERS | QTY | BOTANICAL NAME COMMON NAME | MINIMUM INSTALLED SIZE | ROOT SPACING TRIANGULAR INFILL SPACING REQUIREMENTS OF ANSI Z60.1 "AMERICAN STANDARD FOR NURSERY CAR ELA 88 CAREX ELATA 'BOWLES GOLDEN' **BOWLES GOLDEN SEDGE** 6" POT CONTAINER | 18" O.C. STOCK". IF SPECIFIED PLANT MATERIAL IS NOT OBTAINABLE, SUBMIT HAPPY RETURNS DAYLILY PLANT PERIMETER HEM HAP 80 | HEMEROCALLIS X 'HAPPY RETURNS' |6" POT CONTAINER | 18" O.C. PROOF OF NON AVAILABILITY TO THE ARCHITECTS, TOGETHER WITH AT INDICATED SPACING HYP CAL 379 HYPERICUM CALYCINUM AARONS BEARD 1 GAL CONTAINER 18" O.C PROPOSAL FOR USE OF EQUIVALENT MATERIAL PROVIDE AND INSTALL ALL PLANTS AS IN ACCORDANCE WITH DETAILS 528 LIRIOPE MUSCARI 'BIG BLUE LIR BIG BIG BLUE LILYTURF 1 GAL CONTAINER 18" O.C. AND CONTRACT SPECIFICATIONS NAR KIN 180 NARCISSUS X KING ALFRED KING ALFRED DAFFODIL 3 PER SF 3 BULBS BULBS CONSTRUCTION/INSTALLATION SAL XAL 238 | SALVIA XALAPENSIS `MAYNIGHT SALVIA MAYNIGHT 1 GAL CONTAINER 18" O.C. • LANDSCAPE ARCHITECT RESERVES THE RIGHT TO REJECT ANY PLANTS AND MATERIALS THAT ARE IN AN UNHEALTHY OR UNSIGHTLY PLANT UP TO EDGE OF SHRUBS CONDITION, AS WELL AS PLANTS AND MATERIALS THAT DO NOT UNLESS OTHERWISE NOTED CONFORM TO ANSI Z60.1 "AMERICAN STANDARD FOR NURSERY STOCK" LABEL AT LEAST ONE TREE AND ONE SHRUB OF EACH VARIETY AND CALIPER WITH A SECURELY ATTACHED, WATERPROOF TAG BEARING THE DESIGNATION OF BOTANICAL AND COMMON NAME. **PLAN VIEW** INSTALL LANDSCAPE PLANTINGS AT ENTRANCES/EXITS AND PARKING AREAS ACCORDING TO PLANS SO THAT MATERIALS WILL NOT PRUNE CODOMINATE LEADERS INTERFERE WITH SIGHT DISTANCES. CONTRACTOR IS RESPONSIBLE FOR WATERING ALL PLANT MATERIAL REMOVE BROKEN, BADLY DURING INSTALLATION AND UNTIL FINAL INSPECTION AND ACCEPTANCE BY OWNER. CONTRACTOR SHALL NOTIFY OWNER OF CONDITIONS DEFORMED, RUBBING, NARROW CROTCH ANGLES, WATER WHICH AFFECTS THE GUARANTEE. PLAN VIEW SPROUTS, OR CROSS-BRANCHES. 12 GAUGE GALVANIZED WIRE INSPECTIONS/GUARANTEE 3-2"x2" HARDWOOD STAKES • UPON COMPLETION OF LANDSCAPE INSTALLATION, THE LANDSCAPE CONTRACTOR SHALL NOTIFY THE GENERAL CONTRACTOR WHO WILL 2-3" MULCH LAYER VERIFY COMPLETENESS, INCLUDING THE REPLACEMENT OF ALL DEAD GALVANIZED WIRE GUY PLANT MATERIAL. CONTRACTOR IS RESPONSIBLE FOR SCHEDULING A 12 GAUGE. ALLOW FOR A HEIGHT VARIES 1" COMPOST LAYER SLIGHT AMOUNT OF MOVEMENT FINAL INSPECTION BY THE LANDSCAPE ARCHITECT. WITH PLANTS ALL EXTERIOR PLANT MATERIALS SHALL BE GUARANTEED FOR ONE AMENDED SOIL MIX 8' 2"x2" HARDWOOD STAKE. FULL YEAR AFTER DATE OF FINAL INSPECTION AGAINST DEFECTS 2'-6" MIN BELOW SURFACE. INCLUDING DEATH AND UNSATISFACTORY GROWTH. DEFECTS UNDISTURBED EARTH STAKE SHALL BE DRIVEN A MIN RESULTING FROM NEGLECT BY THE OWNER, ABUSE OR DAMAGE BY 18" OUT FROM TRUNK AND OTHERS, OR UNUSUAL PHENOMENA OR INCIDENTS WHICH ARE BEYOND OUTSIDE OF ROOTBALL REMOVE TAGS, LABELS, AND THE CONTRACTORS CONTROL ARE NOT THE RESPONSIBILITY OF THE PLASTIC SLEEVING. DO NOT STAKE CONTRACTOR 2-3" MULCH LAYER TO EDGE OF DRIPLIN UNLESS SPECIFIED (SEE • PLANT MATERIAL QUANTITIES AND SIZES WILL BE INSPECTED FOR KEEP 4-6" AWAY FROM TRUNKFLARE NOTE) DO NOT WRAP TRUNK COMPLIANCE WITH APPROVED PLANS BY A SITE PLAN REVIEW AGENT OF THE PLANNING DEPARTMENT PRIOR TO THE RELEASE OF THE SET ROOTBALL FLUSH TO GRADE OR CERTIFICATE OF OCCUPANCY. HIGHER IN SLOWLY DRAINING SOILS. D • REMOVE ALL GUY WIRES AND STAKES 12 MONTHS AFTER INSTALLATION. NOT COVER THE TOP OF THE ROOTBAI PRUNE SUCKERS — **MULTI-STEM TREE - STAKING SPECIFIED** WITH SOIL. ROOTFLARE SHALL BE VISIB IF FIELD GROWN, CUT AWAY ALL — ABOVE FINISHED GRADE BALLING ROPES. REMOVE - 6" SAUCER BURLAP OR WIRE BASKET FROM TOP 1/3 OF BALL. IF CONTAINER BACKFILL PLANTING PIT WITH NATIVE GROWN, REMOVE CONTAINER SOIL. INCORPORATE SLOW-RELEASE AND CUT CIRCLING ROOTS GRANULAR FERTILIZER 3- 2" x 2" HARDWOOD STAKES -1:1 SLOPE OF SIDES OF — 1" COMPOST LAYER PLANTING HOLE PRUNE CODOMINATE LEADERS NOTE: STAKE EVERGREENS 6' OR PLACE ROOTBALL ON 12 GAUGE GALVANIZED WIRE -TALLER, ON SLOPES, WITH LARGE SCARIFY SIDES BEFORE PLANTING -RUBBER HOSE **UNEXCAVATED OR TAMPED SOIL** CROWNS, OR IF LOCATED ON TAMP SOIL AROUND ROOTBALL WINDY SITES. - GALVANIZED WIRE GUY NOTE: ONLY STAKE TREES WITH BASE FIRMLY WITH FOOT 12 GAUGE. ALLOW FOR A LARGE CROWNS, 2" CALIPER OR 3 X ROOTBALL PRESSURE SO THAT ROOTBALL SLIGHT AMOUNT OF MOVEMENT GREATER, IF LOCATED ON WINDY DO NOT PRUNE TERMINAL LEADER DIAMETER **PLAN VIEW** DOES NOT SHIFT REMOVE TAGS, LABELS, AND SITES, OR WHERE TAMPERING OR BRANCH TIPS 2"X2" HARDWOOD STAKE MAY OCCUR. PLASTIC SLEEVING. DO NOT 2'-6" MIN BELOW SURFACE STAKE UNLESS NECESSARY (SEE NEW 1/2" RUBBER HOSE -STAKE SHALL BE DRIVEN A MIN 18" OUT NOTE). DO NOT WRAP TRUNK FROM TRUNK AND OUTSIDE OF ROOTBALI 12 GAUGE GALVANIZED WIRE ALLOW FOR SLIGHT MOVEMENT REMOVE TAGS, LABELS AND SET ROOTBALL FLUSH TO GRADE - IF FIELD GROWN CUT AWAY ALL PLASTIC SLEEVING DO NOT OR 1-2" HIGHER IN SLOWLY **DECIDUOUS TREE - STAKING SPECIFIED** 3 2x2 HARDWOOD STAKES, BALLING ROPES. REMOVE BURLAP OR WRAP TRUNK DRAINING SOILS. DO NOT COVER 2'-6" MIN BELOW SURFACE. WIRE BASKET FROM TOP 1/3 OF BALL. IF THE TOP OF THE ROOTBALL WITH STAKE SHALL BE DRIVEN A - PRUNE BROKEN BRANCHES CONTAINER GROWN, REMOVE SOIL. ROOTFLARE SHALL BE MIN. 18" OUT FROM TRUNK CONTAINER AND CUT CIRCLING ROOTS VISIBLE ABOVE FINISHED GRADE.



CONIFEROUS TREE - STAKING SPECIFIED





INSTALL SHRUBS SO THAT THE -

TOP OF THE ROOTBALL IS AT

ORIGINALLY GROWN OR 1-2"

SOILS. DO NOT COVER THE TOP

REMOVE ALL STRING, WIRE, AND -

BURLAP FROM TOP 1/3 OF BALL

OF THE ROOTBALL WITH SOIL

ABOVE IN POOR DRAINING

THE SAME GRADE AS

09/27/16 DRAWN BY F. HANCOCK DESIGNED BY F. HANCOCK CHECKED BY F. HANCOCK SCALE JOB NO. 37723

> SHEET NO. L2.0

REMOVE ALL DEAD, BROKEN,

· 2-3" MULCH LAYER, KEEP AWAY

PROVIDE MULCH UP AND OVER

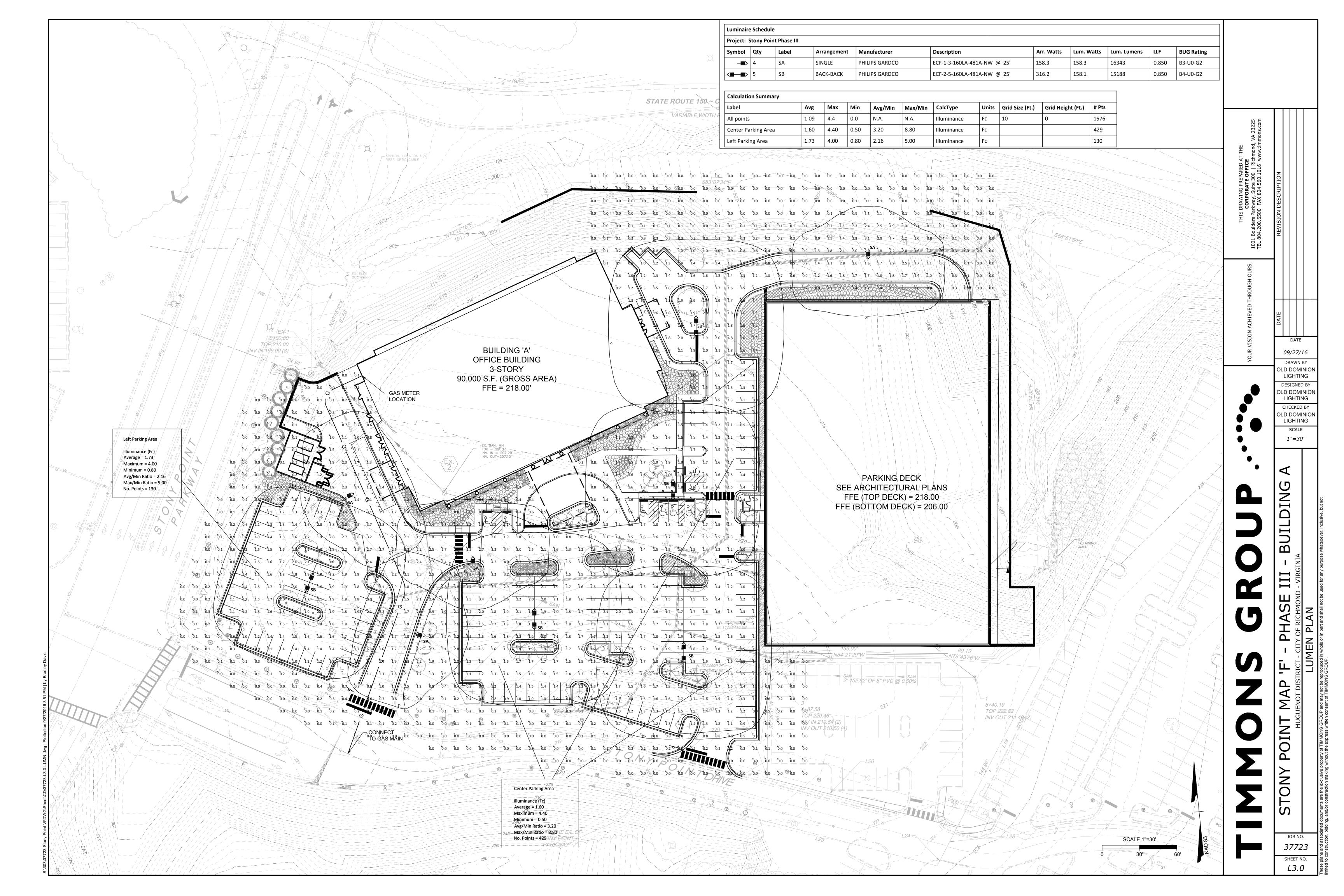
DISEASED, AND WEAK

PLANTING

FROM TRUNK

- 1" COMPOST LAYER

BRANCHES AT TIME OF



Specifications

The Philips Gardco EcoForm combines economy with performance in an LED area luminaire. Capable of delivering up to 20,000 lumens or more in a compact, low profile LED luminaire, EcoForm offers a new level of customer value. Integral control systems including motion response are available for further energy savings.

One piece die cast aluminum housing with integral arm and separate, self retained hinged, one piece die cast door frame.

IP Rating LED light engine rated IP66.

Vibration Resistance EcoForm with the Standard Arm carries a 3G vibration rating that conforms to standards set forth by ANSI C136.31. Testing includes vibration to 3G acceleration in three axes, all performed on the same luminaire.

Electrical

optics and is available with Type 2, 3, and 4 Driver efficiency (>90% standard). 120-480V available (restrictions apply). Open/short to 10% power. RoHS compliant. Surge protector standard. 10kA per ANSI/IEEE

LED Board and Array 32, 48, or 64 LEDs. Color temperatures:

3000K, 4000K, 5700K +/- 250K. Minimum arm kit. When specified with the retrofit CRI of 70, Aluminum metal clad board, arm (RAM) option, EcoForm seamlessly simplifies site conversions to LED by eliminating the need for additional pole. drilling on most existing poles. RAM will be

boxed separately.

Energy Saving Benefits

during unoccupied periods.

Each standard color luminaire receives a

applied, thermally cured, triglycidal

specs on optional or custom colors.

isocyanurate (TGIC) textured polyester

natural aluminum (NP). Consult factory for

Retrofit Arm Mount

LED Thermal Management The one piece housing design provides excellent thermal management critical to

RoHS compliant.

long LED system life **Optical Systems** Type 2, 3, 4, and 5 distributions available. Internal Shield option mounts to LED

distributions to control backlight circuit protection. Optional 0-10V dimming Standard luminaire arm mounts to 4" round fade and abrasion resistant, electrostatically warranty. The LED Drivers carry a 5 year poles. Square pole adapter included with every luminaire. Round Pole Adapter (RPA) required for 3-3.9" poles.

ECF-MR50, ECF-APD-MRO, ECF-MRI, EcoForm features an innovative retrofit ECF-APD-MRI luminaires may be specified for additional energy savings during

unoccupied periods. ETL/cETL listed to the UL 1598 standard,

suitable for Wet Locations. Suitable for use in ambients from -30° to 40°C (-22° to System efficacy up to 95 lms/W with 104°F). The quality systems of this facility significant energy savings over Pulse Start have been registered by UL to the ISO 9001 Metal Halide luminaires. Optional control series standards. options provide added energy savings

EcoForm luminaires feature a 5 year limited warranty. Philips Gardco LED luminaires with LED arrays feature a 5 year limited limited warranty. Motion sensors are covered by warranty for 5 years by the powdercoat finish. Standard colors include motion sensor manufacturer. bronze (BRP), black (BLP), white (WP), and

Prefix	Controls	Mounting	Optics	LED Wattage	Color Temp	Voltage	Finish	Options
ECF -	_		_	_	-	_		
ECF EcoForm	Standard luminaire (leave blank) DIM O-10V Dimming APD¹ Auto Profile Dimming APD-MRO² Auto Profile Dimming and Motion Response Override pole mounted motion sensor APD-MRI²³ APD with Motion Response Override luminaire sensor MRI²³ Motion Response at 50% low luminaire sensor MR5O² Motion Response at 50% low, pole mounted sensor LimeLight Wireless Controls LLC2¹⁵ #2 lens for 8-15′ mounted heights LLC3¹⁵ #3 lens for 15-25′ mounted heights	1 Standard 2 2@180 2@90 2@90 3 3@90 3@120 3@120 4 4@90 WS Wall mount including surface conduit rear entry permitted MA Mast Arm Fitter (requires 2-3/8" O.D. Mast Arm)	2 Type 2 3 Type 3 4 Type 4 5 Type 5	530 mA 55LA-3253¹ 75LA-4853 100LA-6453 700mA 70LA-3270 105LA-4870 135LA-6470 1050mA 105LA-321A¹ 160LA-481A 215LA-641A	(nominal) NW Neutral White 4,000 K 70 CRI (nominal) WW ⁴	120 120V 208 208V 240 240V 277 277V 347 347V 480 480V UNV 120-277V 50hz/60hz HVU 347-480V 50hz/60hz	BRP Bronze Paint BLP Black Paint WP White Paint NP Natural Paint OC Optional Color Specify optional color or RAL (ex OC-LGP or OC-RAL7024) SC Special color Specify, must supply color chip. Requires factory quote.	TL Tool-Less entry and driver removal hardware TB³ Terminal Block IS6 Internal Shield LF² Line Fusing LFC² Line Fusing for Canada PC 57.8 Receptacle with Photocell (includes PCR5) PCB 57.8 Photocell Button PCR5 517.3 Photocell Receptacle only with 2 dimming connections PCR7 51.23 Photocell Receptacle only with 2 dimming and 2 auxiliary connections RAM Retrofit Arm Mount kit PTF2º Pole Top Fitter for 2º/s²-3º Tenon PTF3º Pole Top Fitter for 3º-3/y² Tenon PTF4º Pole Top Fitter for 3º-3/y² Tenon PTF4º Round Pole Adapter for 3º-3.9º O.D RPA¹º Round Pole Adapter for 3°-3.9° O.D

- 1. Available in 120V–277V Voltages only (UNV. 120, 208, 240 & 277). 2. MR50 and APD-MR0 luminaires require
- separately. See page 9 of the ECF spec sheet 6. Not configurable with Type 5 (5) Optics. for Accessories. Available in 120V or 277V only. 7. Not configurable with 120-277V (UNV) Voltage. and auxiliary connections are not connected (for future use only). when used with Terminal Block (TB) Option. 8. Not configurable with 480V (480) Voltage.
- 4. Contact factory for lead times on warm white. 9. Not configurable with 3@120 (3@120) Mounting. LLC2/LLC3/LLC4 Wireless Controls are not No adaptor required for 4" round poles. configurable with PC/PCB/PCR5/PCR7 Options. RPAs provided with Black Paint standard.
- See page 7-8 of ECF spec sheet for more info.

 11. Works with 3-pin or 5-pin NEMA photocell/dimming device. 12. Works with 3-pin or 5-pin NEMA photocell/dimming device 13. If ordered with DIM, APD, MRI, MR50, APD-MRI, APD-MRO, dimming will not be connected to NEMA receptacle.

8 Philips Gardco

Site & Area

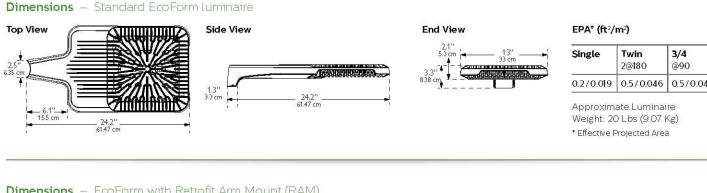
EcoForm Accessories (order separately)

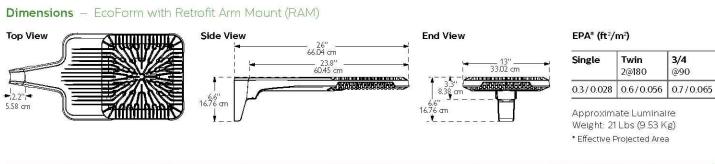
IR-100	MS-A-120V	MS-A-277V			
hand held programmer	120V Input Area Motion Sensor	277V Input Area Motion Sensor			
use with 'MRI' motion response en field programming is required. esired, only one is needed per job.	For MR50 (Motion Response) or APD-MRO (Automatic Profile Dimming with Motion Response Override)	For MR50 (Motion Response) or APD-MRO (Automatic Profile Dimming with Motion Response Override)			

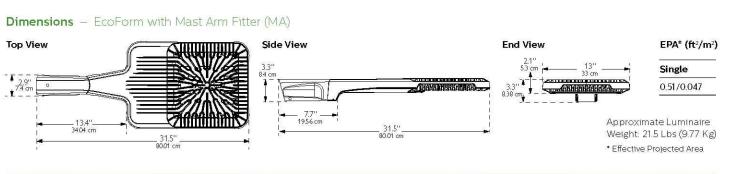
Note: Motion Sensors are ordered separately, with one (1) motion sensor required per pole location for MR50 or APD-MR0 luminaires. See Luminaire Configuration Information on page 5 of the ECF spec sheet for more details, Area motion sensor color is Arctic White. MRI and APD-MRI luminaires include an integral motion sensor.

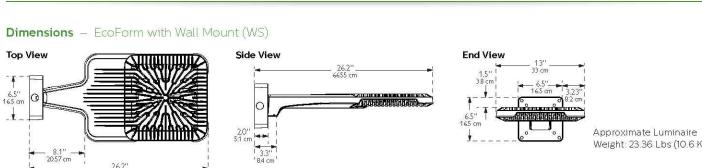
Prior to ordering, consult web based submittal data sheets for the most exclusions. Philips Gardco reserves the right to change materials or modify the design of its product without notification

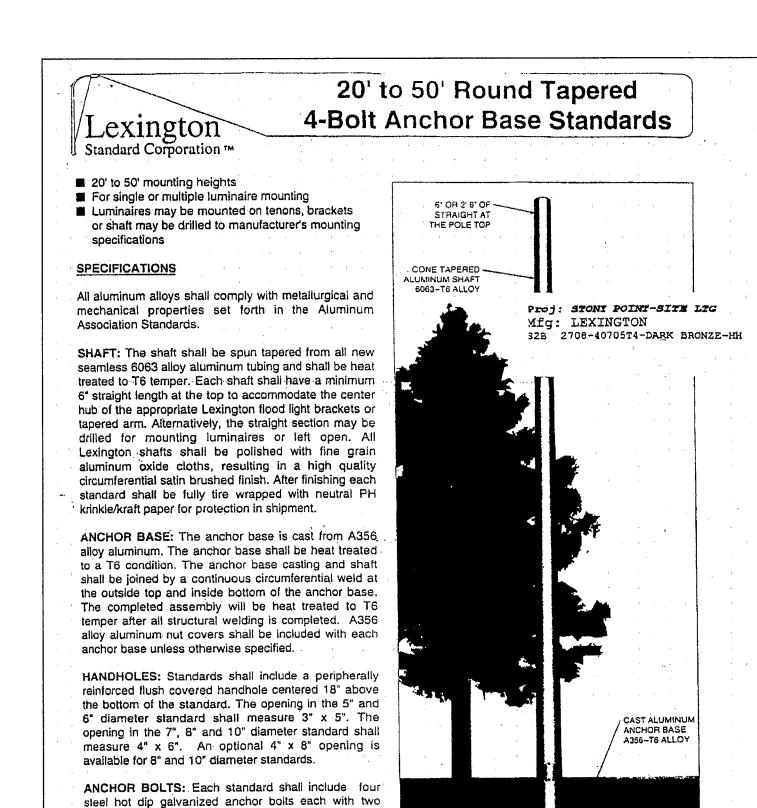
Specifications











nuts, two flat washers and one lock washer sized to

match the anchor bolts. A schedule of bolt sizes

SPECIAL FINISHES: Natural anodize, duranodic, or

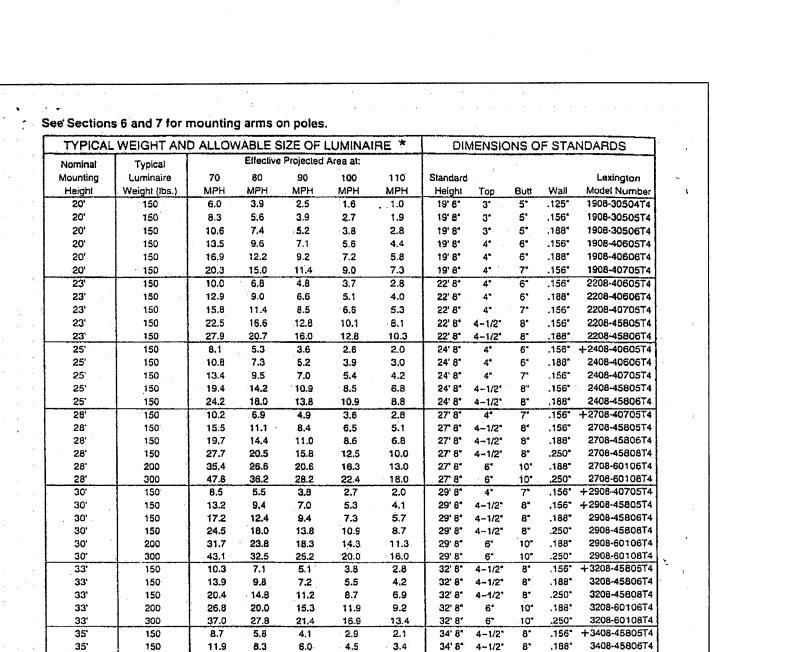
satin brushed finish. All aluminum parts and

accessories shall receive a finish similar to that

painted finishes may be specified as an addition to the

follows.

specified.



12.9

E.P.A. calculations allow for 1.3 gust factor.

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7.5

Islactory performance of lighting standards are dependent upon the standard being properly attached to a supporting foundation of adequate design. Lexington

12.0

© 1997 Lexington Standard Corporation (800) 899-7577 1-1

34' 8" 4-1/2" 8" .250" 3408-45808T4

38'8" 4-1/2" 8" .188" +3808-45806T4 38'8" 4-1/2" 8" .250" 3808-45808T4

(5

P0

37723 SHEET NO. L3.1

09/27/16

DRAWN BY

OLD DOMINION

LIGHTING

DESIGNED BY

OLD DOMINION

LIGHTING

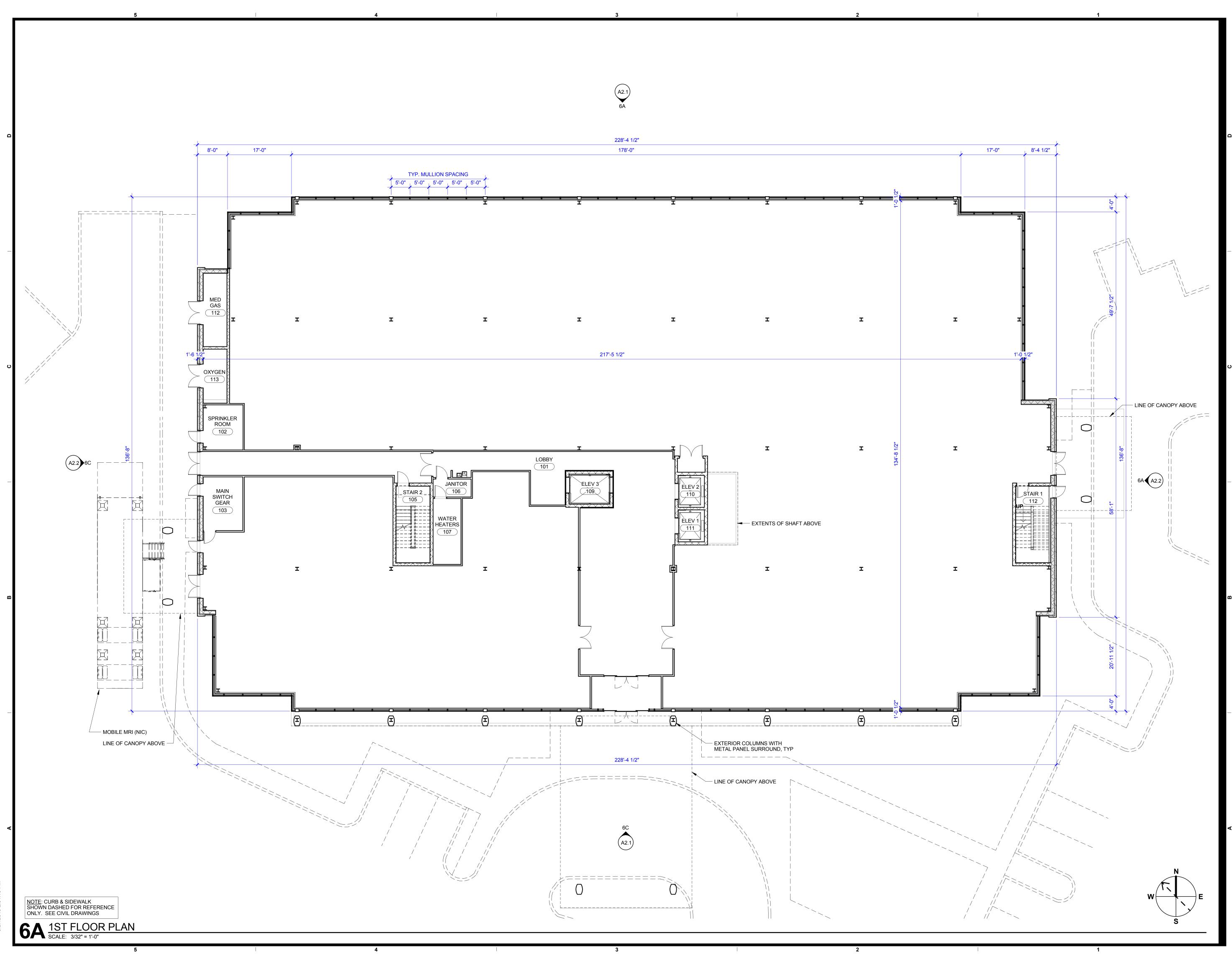
CHECKED BY

OLD DOMINION

LIGHTING

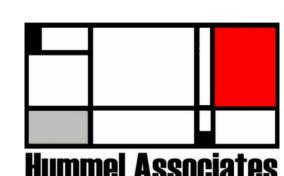
SCALE NOT TO SCALE

JOB NO.





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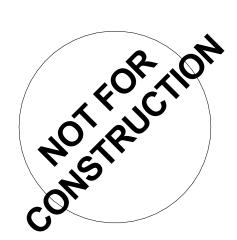
ALED _____



2140081 STONY POINT VI

9101 STONY POINT DRIVE RICHMOND VIRGINIA 23235

0 1' 2' 4' 10' SCALE: 3/32" = 1'-0"



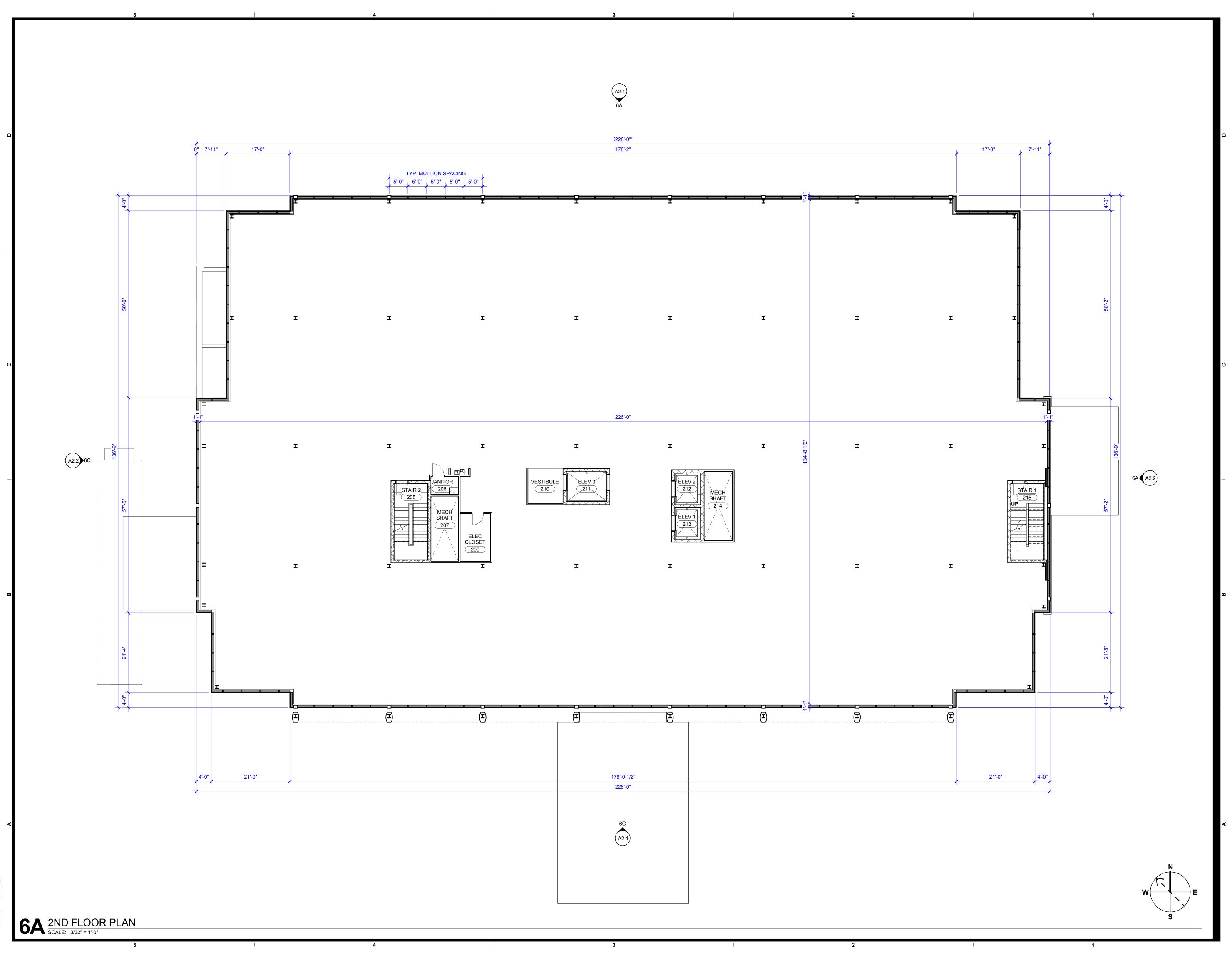
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1ST FLOOR - PLAN

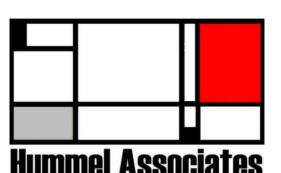
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2140081 STONY POINT VI

9101 STONY POINT DRIVE RICHMOND VIRGINIA 23235

0 1' 2' 4' 10' SCALE: 3/32" = 1'-0"



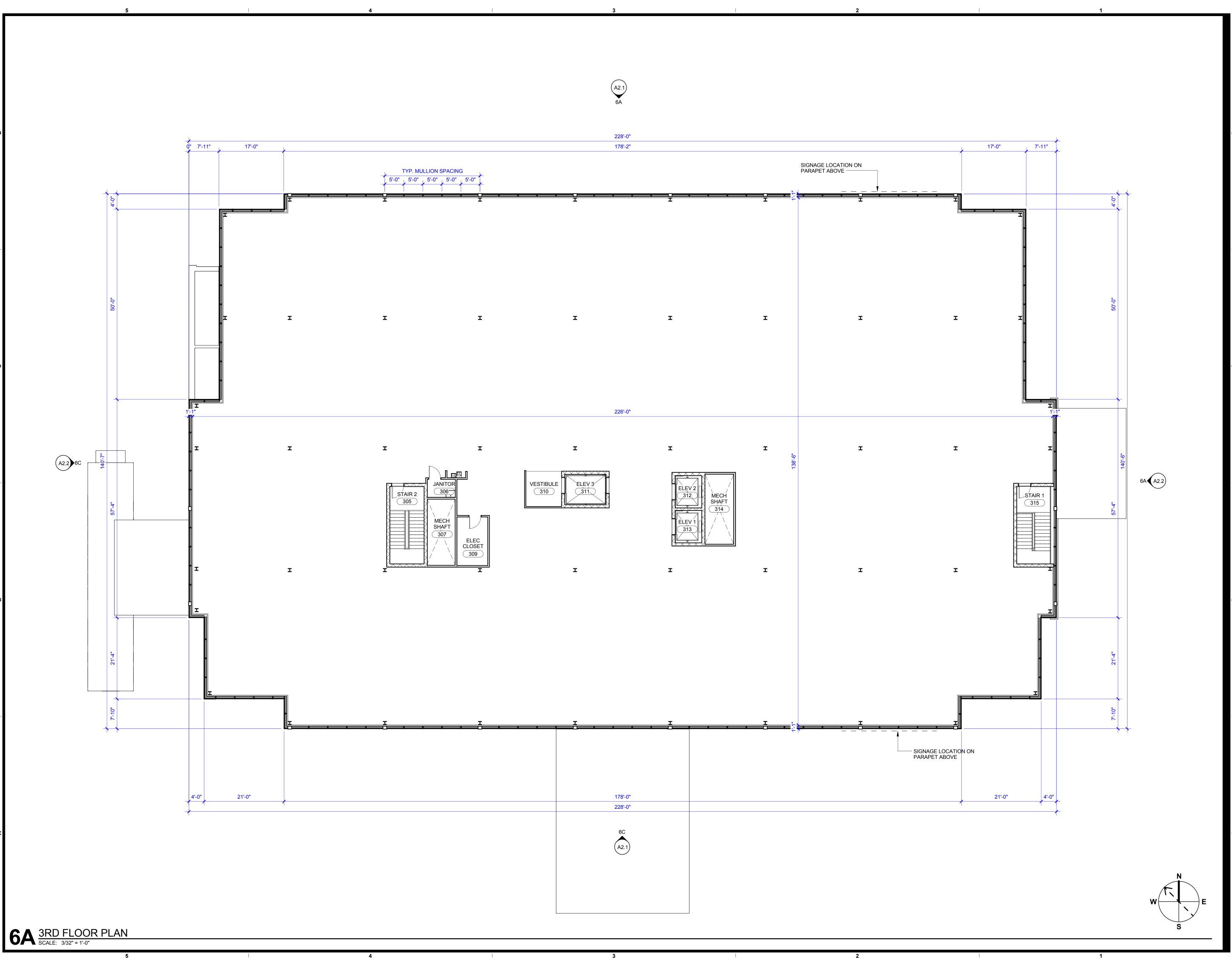
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2ND FLOOR - PLAN

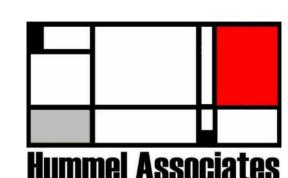
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9/27/2016 2:37:45 PN





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2140081 STONY POINT VI

9101 STONY POINT DRIVE RICHMOND VIRGINIA 23235

0 1' 2' 4' 10' 20' SCALE: 3/32" = 1'-0"



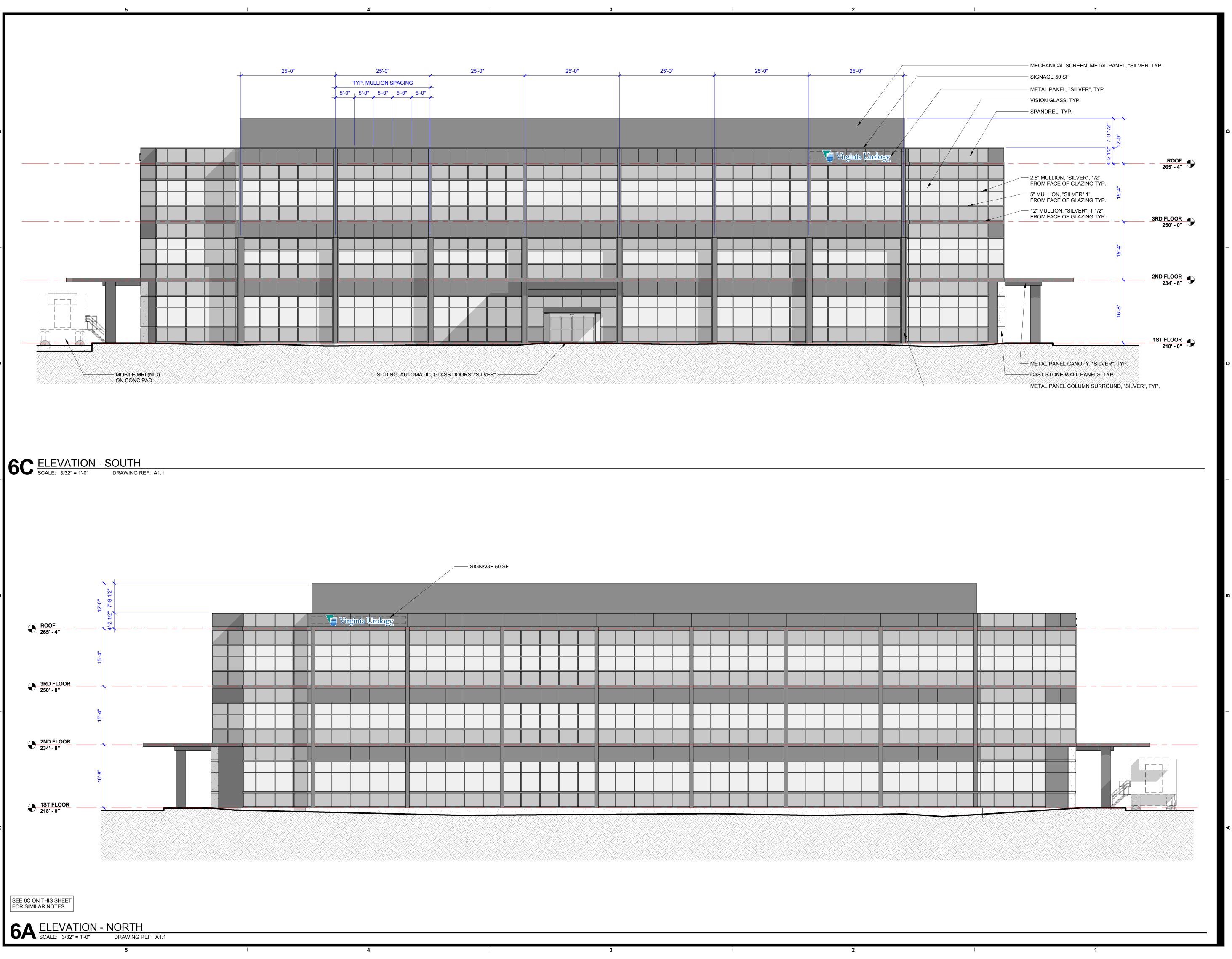
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3RD FLOOR - PLAN

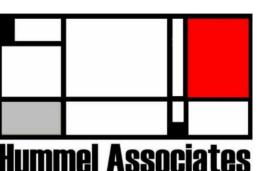
SKD FLOOK • PLAN

DRAWING NO.



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Highwood

2140081 STONY POINT VI

9101 STONY POINT DRIVE RICHMOND VIRGINIA 23235

0 1' 2' 4' 10' SCALE: 3/32" = 1'-0"

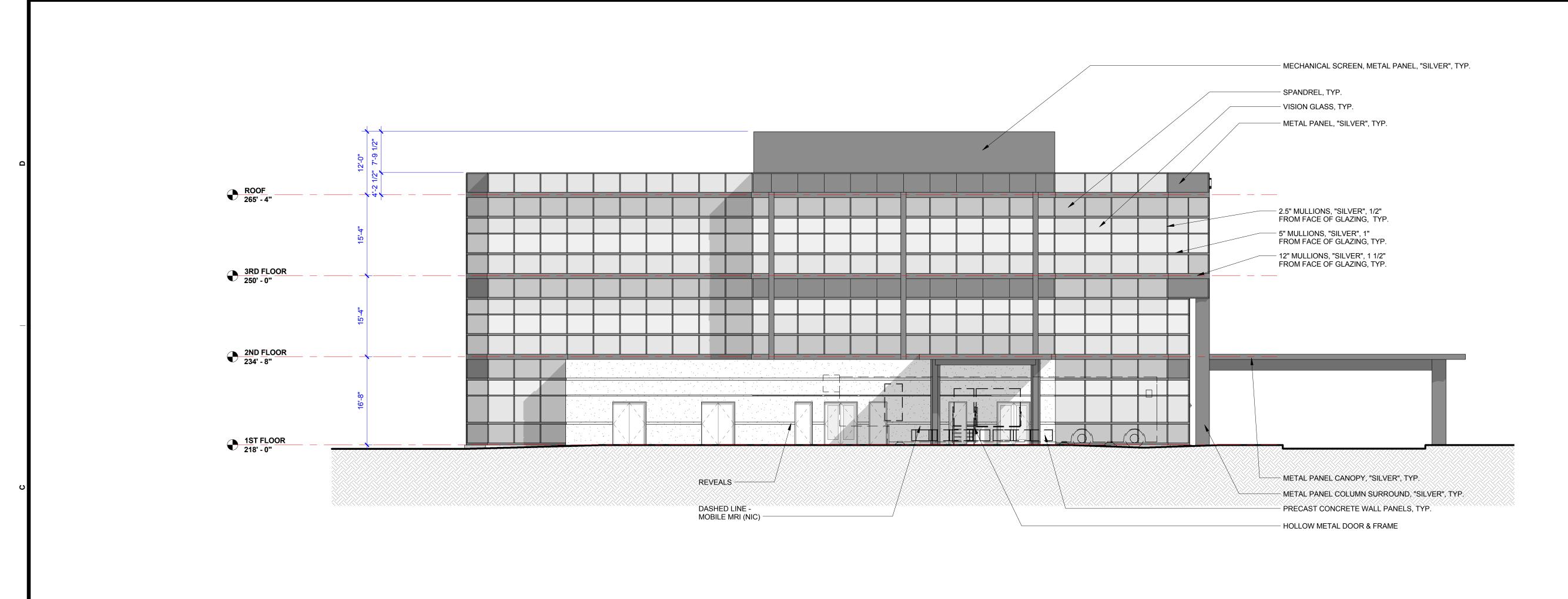


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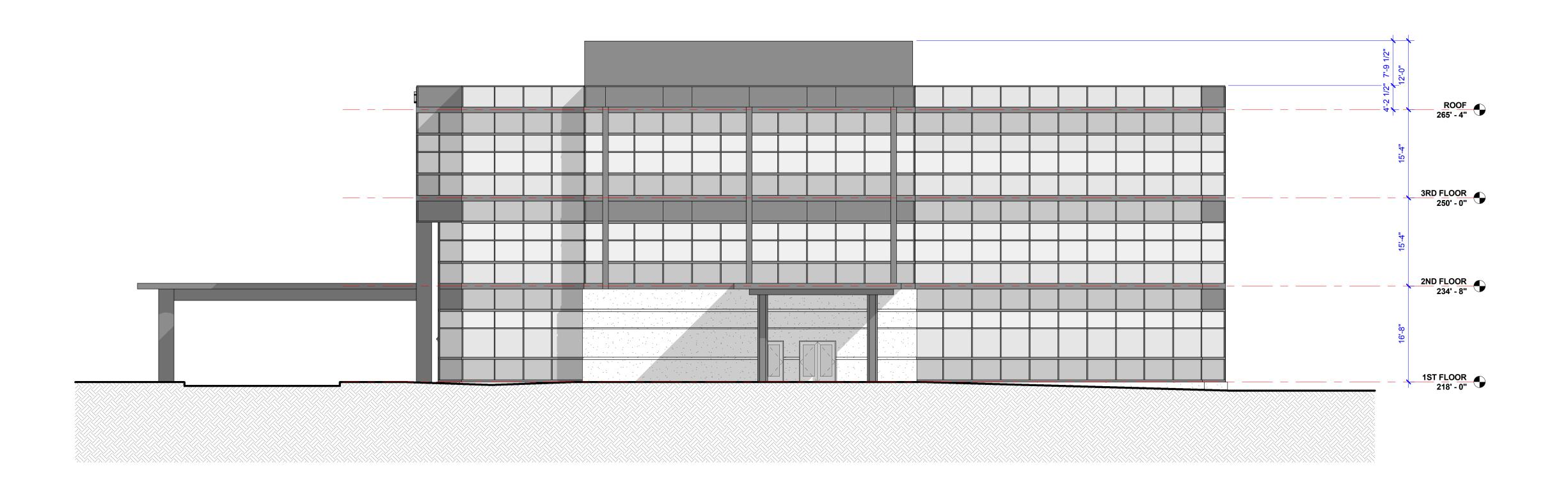
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EXTERIOR ELEVATIONS

DRAWING NO.



6C ELEVATION -WEST
SCALE: 3/32" = 1'-0" DRAWING R

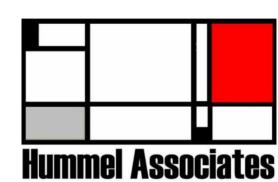


SEE 6C ON THIS SHEET FOR SIMILAR NOTES

6A ELEVATION - EAST
SCALE: 3/32" = 1'-0" DRAWING REF: A1

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WNER -



2140081 STONY POINT VI

9101 STONY POINT DRIVE RICHMOND VIRGINIA 23235

0 1' 2' 4' 10' SCALE: 3/32" = 1'-0"



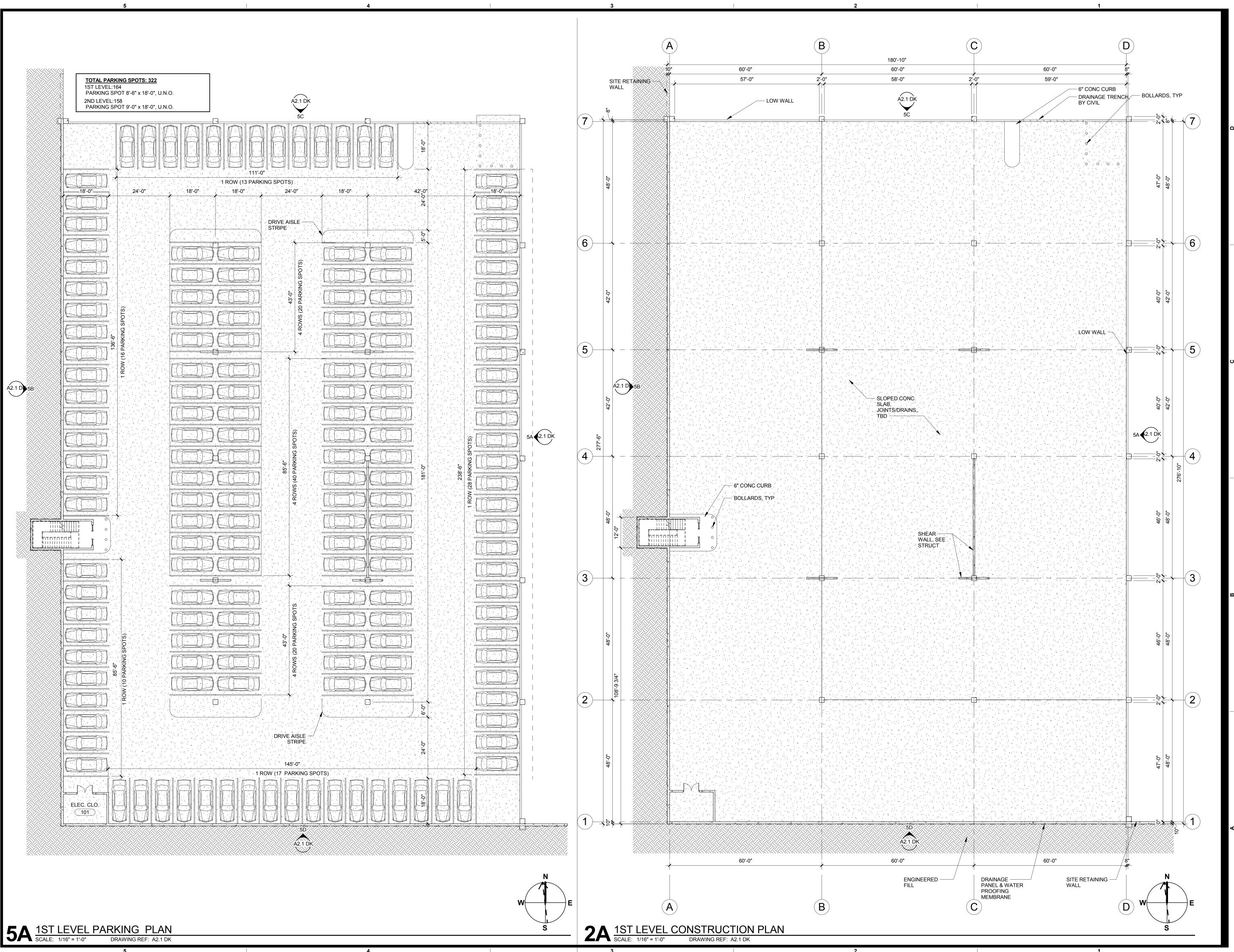
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EXTERIOR ELEVATIONS

DRAWING NO.

A2.2



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Highwoods

2.140081.B STONY POINT VI PARKING STRUCTURE

9101 STONY POINT DRIVE RICHMOND VIRGINIA 23235

0 2' 4' 8' 20' SCALE: 1/16" = 1'-0"



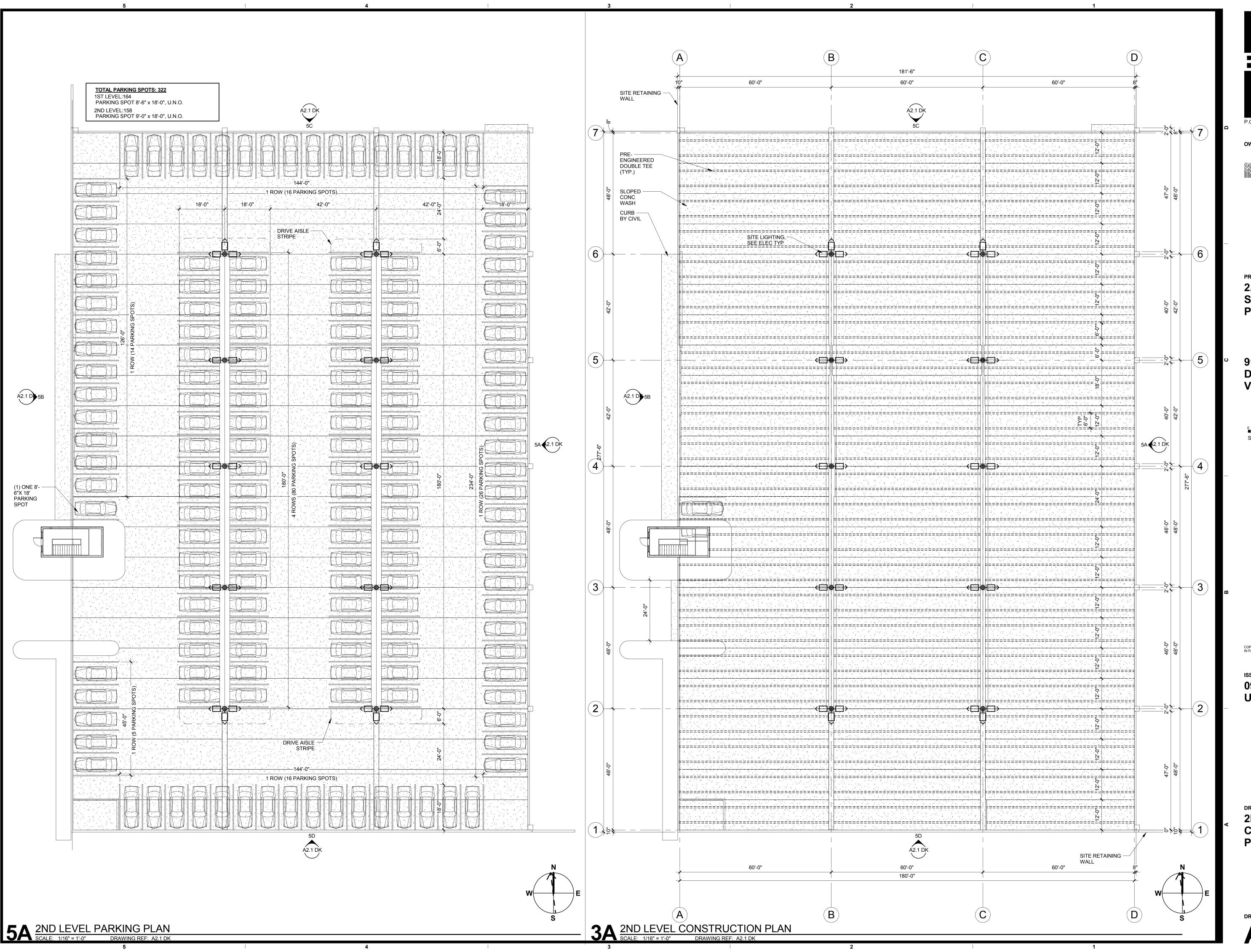
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1ST LEVEL
CONSTRUCTION &
PARKING PLAN

A1.1 DK

27/2016 3:06:08 PM



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Highwoods

2.140081.B STONY POINT VI PARKING STRUCTURE

9101 STONY POINT DRIVE RICHMOND VIRGINIA 23235

0 2' 4' 8' 20' SCALE: 1/16" = 1'-0"

> NOT RUCTION CONSTRUCTION

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2ND LEVEL
CONSTRUCTION &
PARKING PLAN

A12DK

